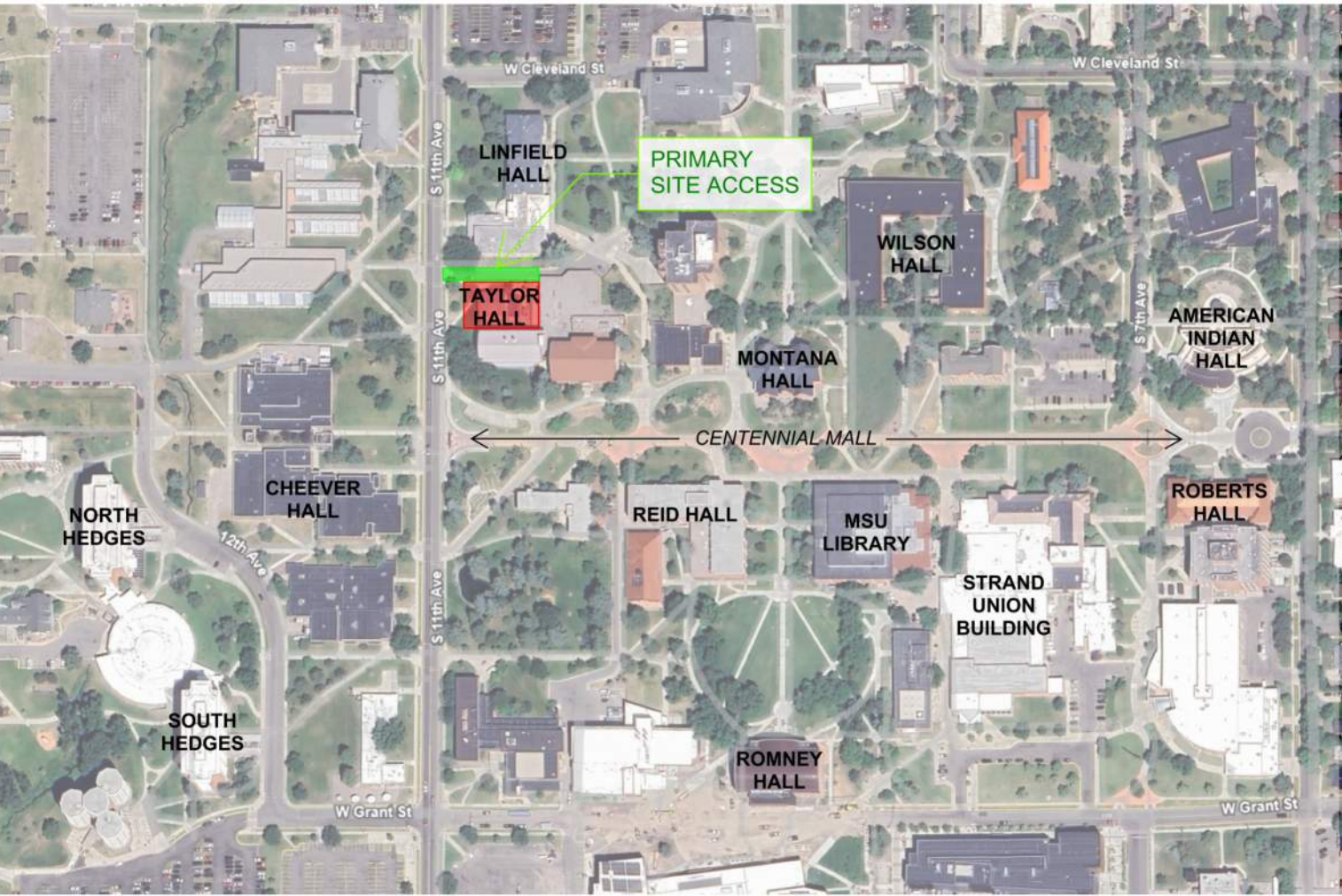


TAYLOR HALL MASONRY AND ENTRY IMPROVEMENTS

MONTANA STATE UNIVERSITY, BOZEMAN, MONTANA



VICINITY MAP - NOT TO SCALE



TAYLOR HALL IN 1894 (MSU HISTORICAL PHOTOGRAPH COLLECTION, No. hres-parc-000211_Taylor Hall, 1894)



PRESENT DAY TAYLOR HALL

GENERAL NOTES

ALL WORK INCLUDED IN THIS CONTRACT, SHALL COMPLY WITH THE LATEST EDITION OF INTERNATIONAL BUILDING CODE, INTERNATIONAL PLUMBING CODE, INTERNATIONAL MECHANICAL CODE, ICC ELECTRICAL CODE, AND ALL OTHER LAWS, CODES, OF LOCAL, COUNTY, STATE, AND LOCAL JURISDICTION INVOLVED.

THE GENERAL CONTRACTOR SHALL VISIT THE SITE PRIOR TO STARTING THE WORK. THE CONTRACTOR SHALL VERIFY GRADES, SITE CONDITIONS, AND COMPARE THAT WITH THE DIMENSIONS SHOWN ON THE DRAWINGS. WHERE CONFLICT EXISTS, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT UPON RECOGNITION OF ANY DISCREPANCY.

THE CONTRACTOR SHALL CAREFULLY STUDY ALL PLANS AND DRAWINGS, AND SHALL REPORT IMMEDIATELY TO THE ARCHITECT ANY ERRORS, INCONSISTENCIES OR OMISSIONS THEY MAY DISCOVER. THE CONTRACTOR SHALL NOT WORK WITHOUT DRAWINGS. THE CONTRACTOR SHALL CONSULT THE ARCHITECT OR SUBMIT SHOP DRAWINGS AND/OR LITERATURE TO THE ARCHITECT FOR APPROVAL PRIOR TO STARTING THE WORK.

THE GENERAL CONTRACTOR SHALL GIVE ALL NOTICES AND SHALL COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS, AND ORDERS OF PUBLIC AUTHORITY BEARING ON THE PERFORMANCE OF THE WORK. IF THE CONTRACTOR OBSERVES THAT ANY OF THE CONTRACT DOCUMENTS ARE AT VARIANCE THEREWITH IN ANY RESPECT THEY SHALL PROMPTLY NOTIFY THE ARCHITECT OF ANY CHANGES REQUIRING ADJUSTMENT WITH APPROPRIATE MODIFICATION.

ONLY APPROVED 'CONSTRUCTION SET' MARKED DRAWINGS INCORPORATING ALL ADDENDUM AND DIMENSION CLARIFICATIONS SHALL BE USED DURING THE EXECUTION OF THE WORK.

THE CONTRACTOR SHALL USE WRITTEN DIMENSIONS ONLY, OR AS DIRECTED BY ARCHITECT. THE CONTRACTOR SHALL NOT SCALE DRAWINGS.

CROSS REFERENCES SHOWN ON DRAWINGS DO NOT NECESSARILY INDICATE ALL LIKE CONDITIONS AND DO NOT LIMIT APPLICATION OF ANY DRAWING OR DETAIL. THEY MAY APPLY TO OTHER, SAME, OR SIMILAR CONDITIONS NOT REFERENCED.

INTERIOR WALL DIMENSIONS (FOR NEW WALLS ONLY) ARE TO FACE OF STUD FRAMING UNLESS OTHERWISE NOTED.

SECTION AND INTERIOR ELEVATION DIMENSIONS ARE TO THE TOP OF CONCRETE OR METAL DECKING UNLESS OTHERWISE NOTED.

CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFICATION AND COORDINATION OF SUBCONTRACTORS WORK TO SECURE COMPLIANCE OF DRAWINGS AND SPECIFICATIONS, THE ACCURATE LOCATION OF STRUCTURE MEMBERS, AND OPENINGS FOR MECHANICAL, ELECTRICAL, STAIRS, ELEVATORS, AND MISCELLANEOUS EQUIPMENT.

CONTRACTOR SHALL VERIFY SIZES AND LOCATIONS OF ALL OPENINGS FOR MECHANICAL AND ELECTRICAL EQUIPMENT WITH RESPECTIVE SUB-CONTRACTORS, AS WELL AS SHOP DRAWINGS REVIEWED BY THE ARCHITECT.

CONTRACTOR SHALL VERIFY ALL ROUGH-IN DIMENSIONS FOR EQUIPMENT AND PROVIDE ALL BUCK-OUT BLOCKING AND BACKING REQUIRED BY THIS CONTRACT AND OTHERS.

WHERE PIPING, CONDUIT, AND/OR DUCTS PASS THROUGH FIRE RATED WALLS. PACK AROUND OPENINGS WITH SAFING OR SPRAY INSULATION. PROVIDE FIRE DAMPERS WHERE NECESSARY.

ABBREVIATIONS

ALUM. ANN.	ALUMINUM ANNUNCIATOR	MECH. MFG. M.T.L.	MECHANICAL MANUFACTURER MOISTURE RESISTANT METAL
BD. BLCK'G.	BOARD BLOCKING	N.I.C.	NOT IN CONTRACT
CAB. CER. CLR. BOARD	CABINET CERAMIC CLEARANCE	O.C. O.S.B.	ON CENTER ORIENTED STRAND
COMP. CONC. INSTALLED	COMPOSITE CONCRETE	O.F.C.I.	OWNER FURNISHED CONTRACTOR
CONF. CORR. C.M.U. C.T. CUST.	CONFERENCE CORRIDOR CONCRETE MASONRY UNIT CERAMIC TILE CUSTOM	O.F.O.I.	OWNER FURNISHED OWNER INSTALLED
D.F. DISP. D.M. DR.	DRINKING FOUNTAIN DISPENSER DRYMARK BOARD DRAWER	P. P. LAM. P.T. PRE-FIN. PVC.	PAINT PLASTIC LAMINATE PAPER TOWEL PRE-FINISHED POLYVINYLCHLORIDE
E.I.F.S.	EXTERIOR INSULATION FINISH SYSTEM	R. REC. REST. REQ'D.	RADIUS RECESSED RESTROOM REQUIRED
F.D. F.E. F.F. F.S. FLR. FDN. F.O.	FLOOR DRAIN FIRE EXTINGUISHER FINISH FLOOR FLOOR SINK FLOORING FOUNDATION FACE OF	S. S.C. S.F. S.V. SIM. SPECS. STOR.	STAIN SOLID CORE SQUARE FEET SHEET VINYL SIMILAR SPECIFICATIONS STORAGE
G.B. GWB GYP. BD.	GYPSUM WALLBOARD GYPSUM WALLBOARD GYPSUM WALLBOARD	T.B. T.O. T.P. TYP. V.B. V.C.T.	TACK BOARD TOP OF TOILET PAPER TYPICAL VAPOR BARRIER VINYL COMPOSITION TILE VERIFY
HC. H.M.	HANDICAPPED HOLLOW METAL	VER.	WITH WITHOUT
INSUL.	INSULATION	W/ W/O	
JAN.	JANITOR		

NOTES AND SYMBOLS

	DETAIL REFERENCE		DOOR NUMBER
	SECTION CUT		WINDOW TYPE
	INTERIOR ELEVATION		NOTE REFERENCE
	ROOM NUMBER		WALL TYPE

MATERIALS LEGEND

	EARTH		STEEL
	COMPACTED GRAVEL		FINISH WOOD
	CONCRETE		BATT INSUL.
	BRICK		RIGID INSUL.
	C.M.U.		GYP. BD.

PROJECT TEAM:

OWNER/AGENT:
MONTANA STATE UNIVERSITY; CPDC
Contact: Donny Beebe
406.994.4547

ARCHITECT:
THINKONE ARCHITECTS
Contact: Jake St. Sauver
406.586.7020

STRUCTURAL ENGINEER:
DCI ENGINEERS
Contact: Jon Lovgren
406.556.8600

SCHEDULE OF DRAWINGS:

GENERAL	
NO.	DRAWING SHEET
A0.0	COVER SHEET
A0.1	CODE SHEET
A0.2	SITE ACCESS / PROJECT INFO
ARCHITECTURAL DEMO	
NO.	DRAWING SHEET
A0.3	DEMO BASE BID / ALT #1
A0.4	DEMO ALT #2
STRUCTURAL	
NO.	DRAWING SHEET
S0.1	GENERAL NOTES
S0.2	GENERAL NOTES
S0.3	GENERAL NOTES
S0.4	GENERAL NOTES
S0.5	GENERAL NOTES
S1.1	PLANS
S2.1	CONCRETE DETAILS
S3.1	STEEL DETAILS
ARCHITECTURAL	
NO.	DRAWING SHEET
A1.0	ENTRY CANOPY PLAN
A1.1	CANOPY ELEVATIONS
A1.2	CANOPY DETAILS
L1.0	LANDSCAPE PLAN
A2.1	EXTERIOR ELEVATION - W
A2.1a	EXISTING PHOTOS - WEST
A2.2	EXTERIOR ELEVATIONS - N
A2.2a	EXISTING PHOTOS - NORTH
A2.3	EXTERIOR ELEVATION - E
A2.3a	EXISTING PHOTOS - EAST
A2.4	EXTERIOR ELEVATION - S
A2.4a	EXISTING PHOTOS - SOUTH
A2.5	DETAILS
A3.0	WINDOW REPLACEMENT ELEVATIONS
A5.1	DOOR & WINDOW SCHEDULES
A5.2	WINDOW DETAILS
A5.3	WINDOW DETAILS



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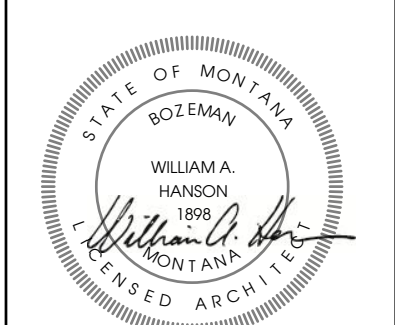
100% CONSTRUCTION DOCUMENTS

THINKONE

DRAWN BY: **Author**

REVIEWED BY: **Checker**

REV.	DESCRIPTION	DATE



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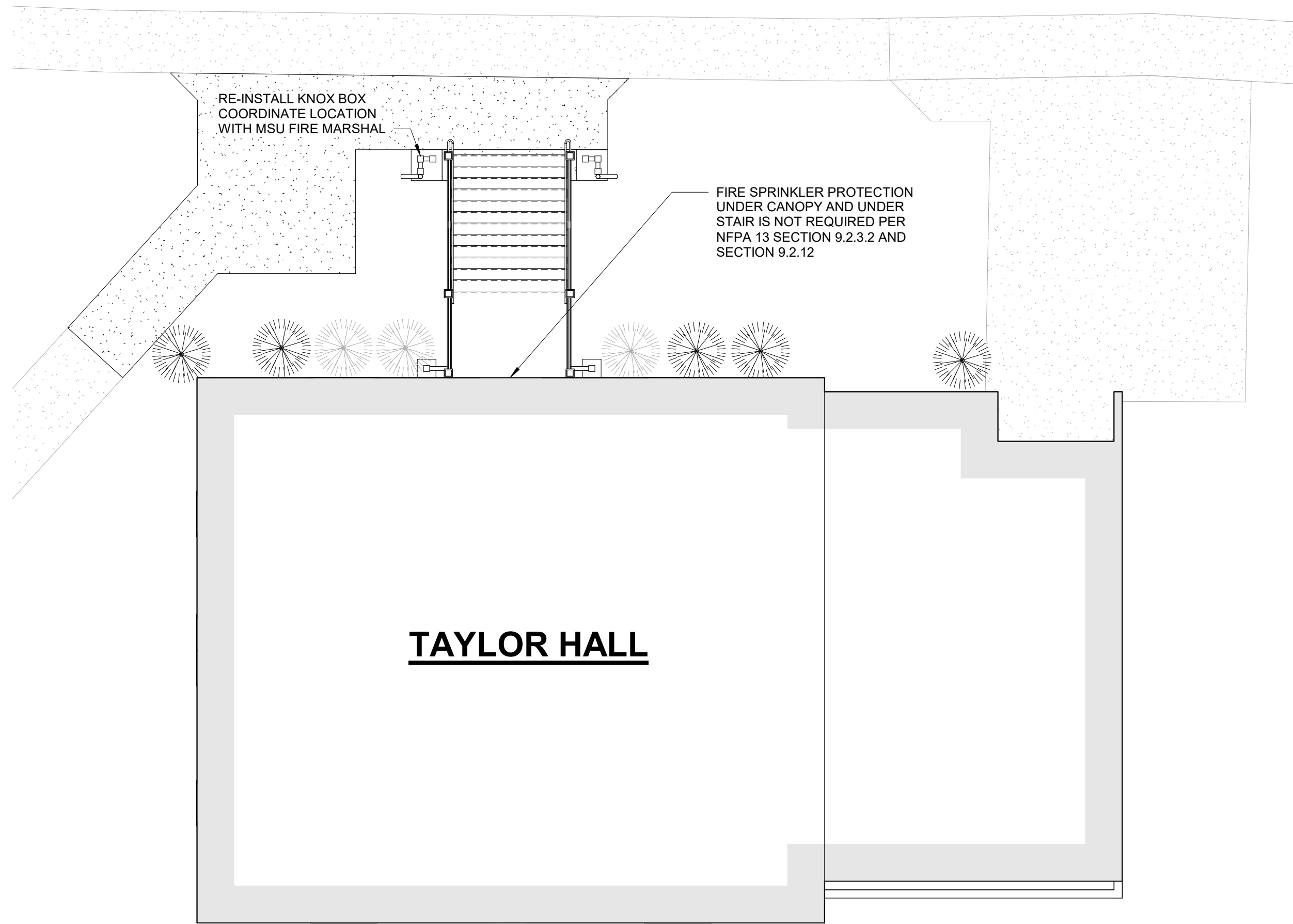
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SHEET

A0.0

DATE

03/10/25



1 FIRST FLOOR PLAN - CODE PLAN
A0.1 1/8" = 1'-0"

CODE CHECK

CODES:	INTERNATIONAL BUILDING CODE (2021)	
BUILDING	INTERNATIONAL EXISTING BUILDING CODE (2021)	
FIRE	INTERNATIONAL FIRE CODE (2021)	
ACCESSIBILITY	ANSI 117.1 (2017)	
MECHANICAL	INTERNATIONAL MECHANICAL CODE (2021)	
PLUMBING	UNIFORM PLUMBING CODE (2021)	
ELECTRICAL	NATIONAL ELECTRICAL CODE (2020)	
PERMITS	CODE SOURCE:	REQUIREMENTS:
OCCUPANCY	LOCAL JURISDICTION	LOCAL JURISDICTION
CONSTRUCTION TYPE	IBC CH. 3	B
CLASSIFICATION OF WORK	IBC CH. 6	V-B
	IEBC	REPAIR
		- MASONRY REPAIR
		LEVEL 1 ALTERATION
		- WINDOW REPLACEMENT
		- STAIR & CANOPY RECONSTRUCTION

FIRE PROTECTION: BUILDING HAS BEEN EQUIPED WITH AN AUTOMATIC SPRINKLER SYSTEM IN ACCORDANCE WITH IBC CHAPTER 9

NEW CANOPY IS NON-COMBUSTABLE AND THERE WILL BE NO STORAGE BELOW. PER 2019 NFPA 13 SECTION 9.2.3.2 SPRINKLER PROTECTION IS NOT REQUIRED.

NEW ENTRY STAIR IS NON-COMBUSTABLE, NO EXTERIOR WALLS AND NO STORAGE BELOW. PER 2019 NFPA 13 SECTION 9.2.12 SPRINKLER PROTECTION IS NOT REQUIRED



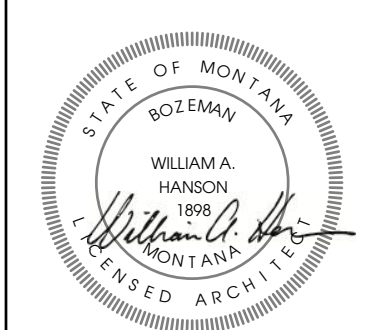
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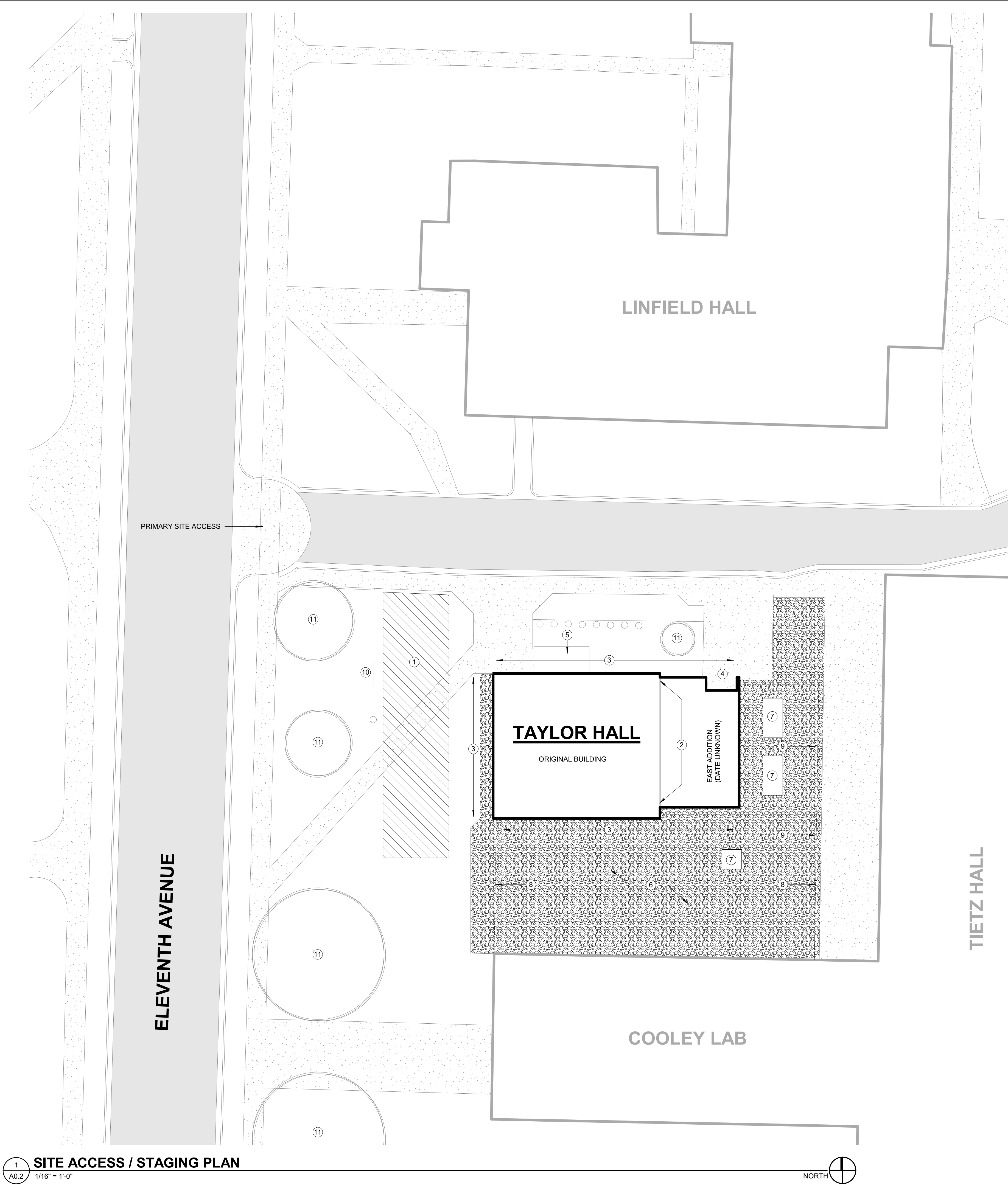
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A/E#00-0-00
CONSULTANT #2308
SHEET TITLE
CODE SHEET

SHEET
A0.1

DATE
03/10/25



KEYED NOTES FOR DRAWING 1/A0.2

- ① CONSTRUCTION STAGING AREA. CONTRACTOR SHALL PROVIDE FENCING AS REQUIRED TO PREVENT ACCESS FROM UNAUTHORIZED PERSONNEL. CONTRACTOR SHALL RESTORE AREAS USED FOR CONSTRUCTION STAGING THAT ARE DAMAGED DURING CONSTRUCTION TO PRE-CONSTRUCTION CONDITIONS
- ② ALL WORK PERFORMED ON EAST ELEVATION WILL HAVE TO OCCUR OVER TOP OF THE EAST ADDITION ROOF. CONTRACTOR IS TO PROVIDE NECESSARY ROOF PROTECTION SO AS TO NOT DAMAGE ROOF DURING CONSTRUCTION OPERATIONS
- ③ GROUND PROTECTION IS REQUIRED FOR ALL LIFTS AND/OR SCAFFOLDING AROUND BUILDING EXCEPT FOR AREAS IN THE GRAVEL EQUIPMENT YARD
- ④ SERVICE DRIVE, DO NOT BLOCK ACCESS
- ⑤ BUILDING ENTRY AND CANOPY
- ⑥ GRAVEL EQUIPMENT YARD.
- ⑦ EXISTING GENERATOR / MECHANICAL EQUIPMENT
- ⑧ EXISTING FENCE
- ⑨ EXISTING GATE ACCESS
- ⑩ EXISTING BUILDING SIGN
- ⑪ EXISTING TREES

GENERAL NOTES:

- 1. CONTRACTOR SHALL RESTORE ALL AREA EFFECTED BY CONSTRUCTION OPERATIONS TO ORIGINAL CONDITIONS. THIS SHALL INCLUDE BUT IS NOT LIMITED TO SIDEWALKS, ADJACENT BUILDINGS, GRAVEL AND GRASS YARDS
- 2. THE CONTRACTOR SHALL VERIFY THE EXISTENCE, LOCATION, DEPTH AND SIZE OF THE UTILITIES PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ANY DAMAGES TO THE EXISTING FACILITIES DUE TO THE FAILURE TO LOCATE OR PROPERLY PROVIDE PROTECTION WHEN LOCATION IS KNOW.
- 3. A HAZARDOUS MATERIALS REPORT HAS BEEN PROVIDED IN THE PROJECT MANUAL. ALL HAZARDOUS MATERIAL ABATEMENT IS THE RESPONSIBILITY OF THE CONTRACTOR.
- 4. BASE BID AND ALTERNATE SCOPES OF WORK.

BASE BID: ALL DEMOLITION, NEW CONSTRUCTION AND LANDSCAPING ASSOCIATED WITH THE NEW ENTRY CANOPY, STAIR AND ENTRY DOOR. THIS WORK IS DESCRIBED ON THE FOLLOWING SHEETS UNLESS SPECIFICALLY NOTED OTHERWISE;
A0.3
A1.0
A1.1
A1.2
L1.0

ALTERNATE #1: ALL DEMOLITION AND NEW CONSTRUCTION ASSOCIATED WITH THE MASONRY RESTORATION. THIS WORK IS DESCRIBED ON THE FOLLOWING SHEETS UNLESS SPECIFICALLY NOTED OTHERWISE;
A0.3
A2.1
A2.1a
A2.2
A2.2a
A2.3
A2.3a
A2.4
A2.4a

ALTERNATE #2: ALL DEMOLITION AND NEW CONSTRUCTION ASSOCIATED WITH THE WINDOW REPLACEMENT. THIS WORK IS DESCRIBED ON THE FOLLOWING SHEETS UNLESS SPECIFICALLY NOTED OTHERWISE;
A0.4
A5.0
A5.1
A5.2



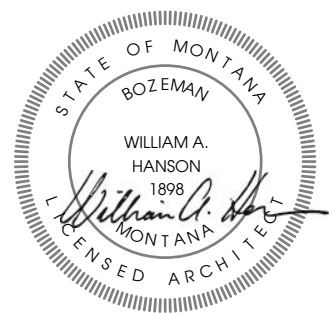


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SHEET TITLE
SITE ACCESS /
PROJECT INFO

SHEET
A0.2

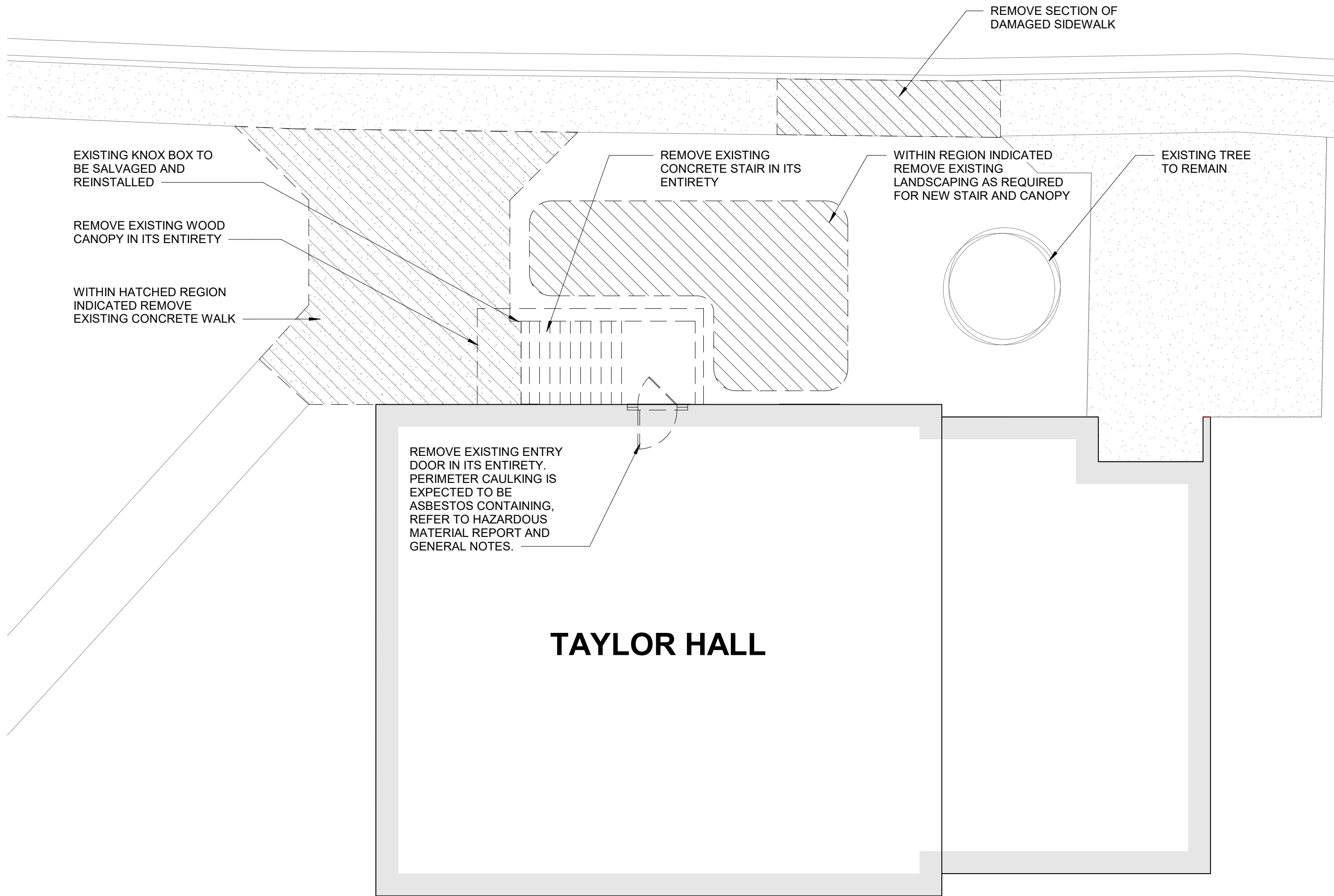
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EXISTING KNOX BOX TO BE SALVAGED AND REINSTALLED. COORDINATE REMOVAL WITH MSU FIRE MARSHAL

5 EXISTING PHOTO - KNOX BOX
A0.3 12" = 1'-0"

- NOTES:
1. ALL DEMO WORK DESCRIBED ON THIS SHEET IS BASE BID SCOPE UNLESS SPECIFICALLY NOTED OTHERWISE
2. HAZARDOUS MATERIALS REPORT HAS BEEN PROVIDED IN THE PROJECT MANUAL. ALL HAZARDOUS MATERIAL ABATEMENT IS TO BE DONE BY THE CONTRACTOR.
- CAULKING BETWEEN DOOR/WINDOW AND BUILDING IS ANTICIPATED TO BE ASBESTOS CONTAINING. ABATEMENT MUST BE PERFORMED BY A MONTANA ACCREDITED ASBESTOS ABATEMENT CONTRACTOR ACCORDING TO LOCAL, STATE, AND FEDERAL REGULATIONS.
 - EXTERIOR PAINT IS EXPECTED TO BE LEAD CONTAINING. ALL DEMOLITION OF LEAD BASED PAINT CONTAINING ITEMS MUST BE DONE ACCORDING TO LOCAL, STATE AND FEDERAL REGULATIONS.



1 SITE PLAN - DEMOLITION
A0.3 1/8" = 1'-0"



REMOVE EXISTING WOOD CANOPY IN ITS ENTIRETY

EXISTING SPRINKLER HEAD TO BE REMOVED. TYPICAL OF (2). REMOVAL REQUIRES LICENSED FIRE SPRINKLER FITTER. COORDINATE WITH MSU FIRE ALARM TECHS.

4 EXISTING PHOTO - SPRINK HEADS
A0.3 12" = 1'-0"



EXISTING TREE TO REMAIN

REMOVE EXISTING GUTTER AND DOWNSPOUT (ALTERNATE #1)

REMOVE EXISTING WOOD CANOPY IN ITS ENTIRETY

REMOVE EXISTING ENTRY DOOR IN ITS ENTIRETY

REMOVE EXISTING CONCRETE STAIR IN ITS ENTIRETY

REMOVE EXISTING LANDSCAPING AS REQUIRED FOR NEW STAIR AND CANOPY

REMOVE AND SALVAGE EXISTING BIKE RACK TO BE REINSTALLED

2 EXISTING PHOTO - NORTH ELEVATION DEMO
A0.3 12" = 1'-0"



REMOVE EXISTING GUTTER AND DOWNSPOUT (ALTERNATE #1)

REMOVE EXISTING VEGETATION (ALTERNATE #1)

3 EXISTING PHOTO - WEST ELEVATION DEMO
A0.3 12" = 1'-0"

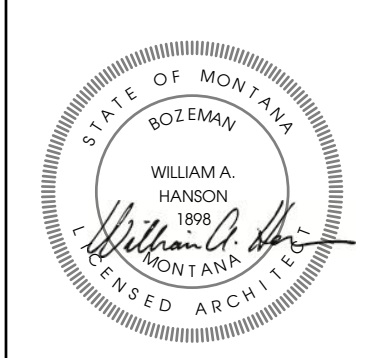


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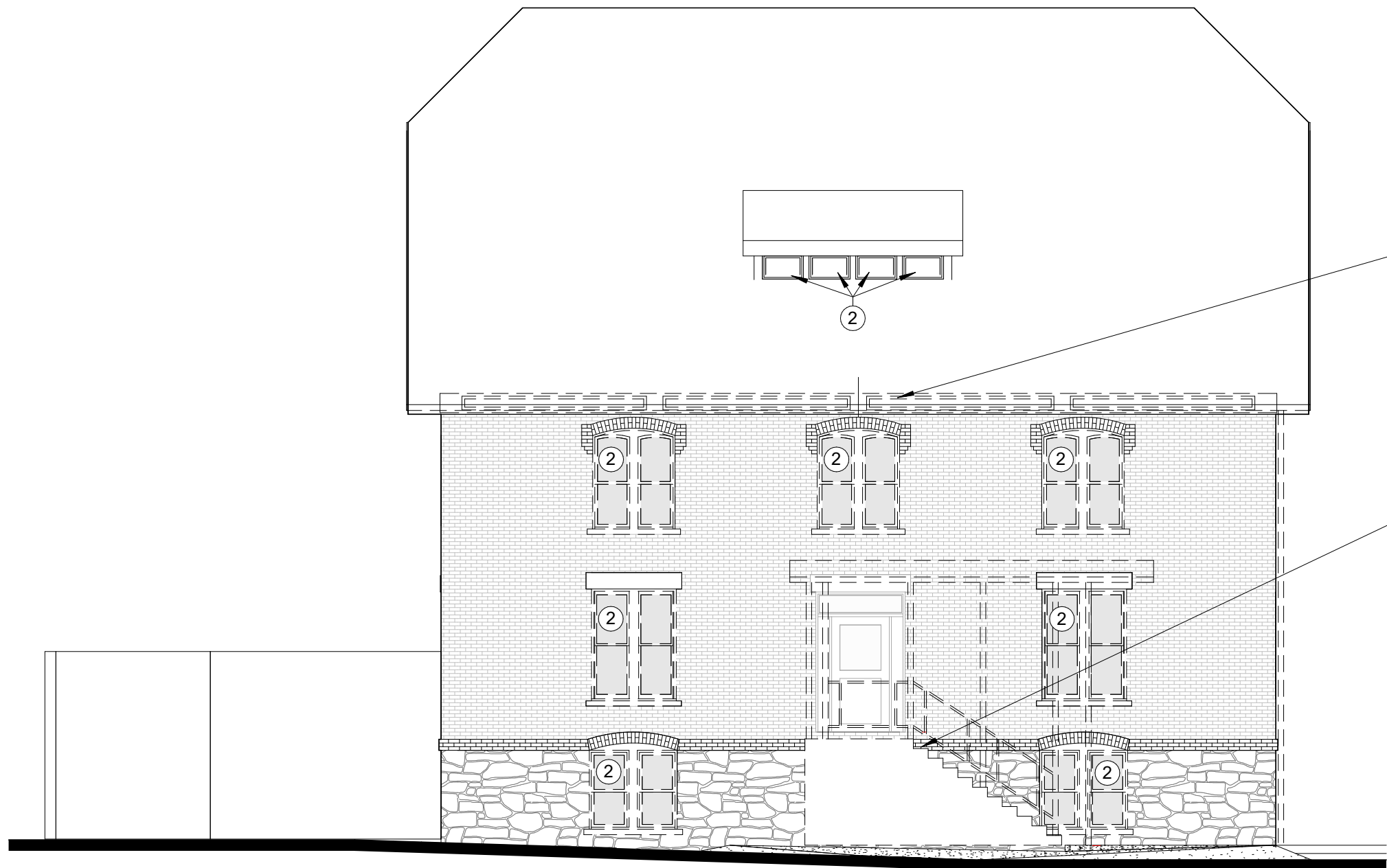
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A/E#00-0-00
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SHEET TITLE
DEMO - BASE BID
/ ALT #1

SHEET
A0.3

DATE
03/10/25

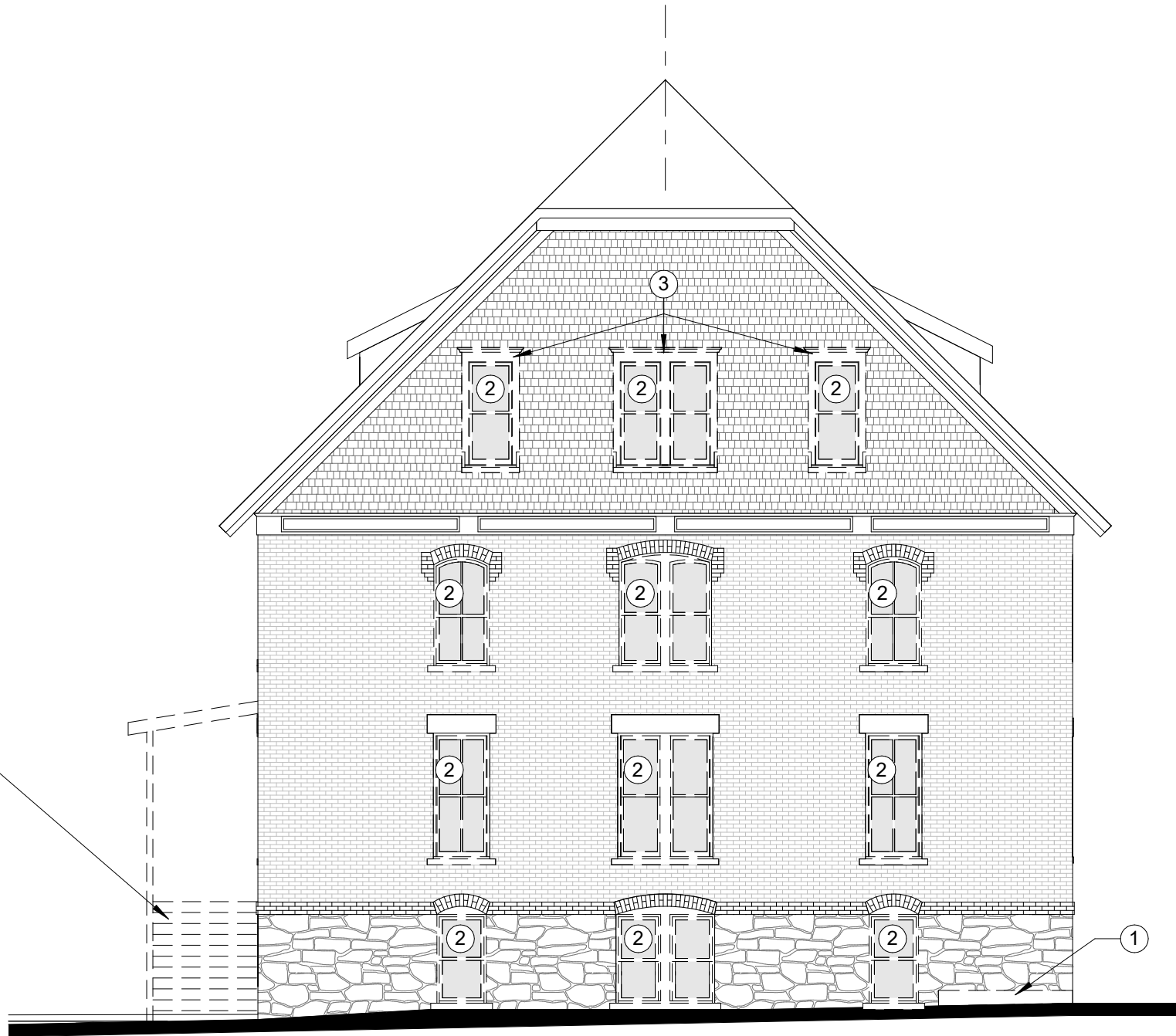
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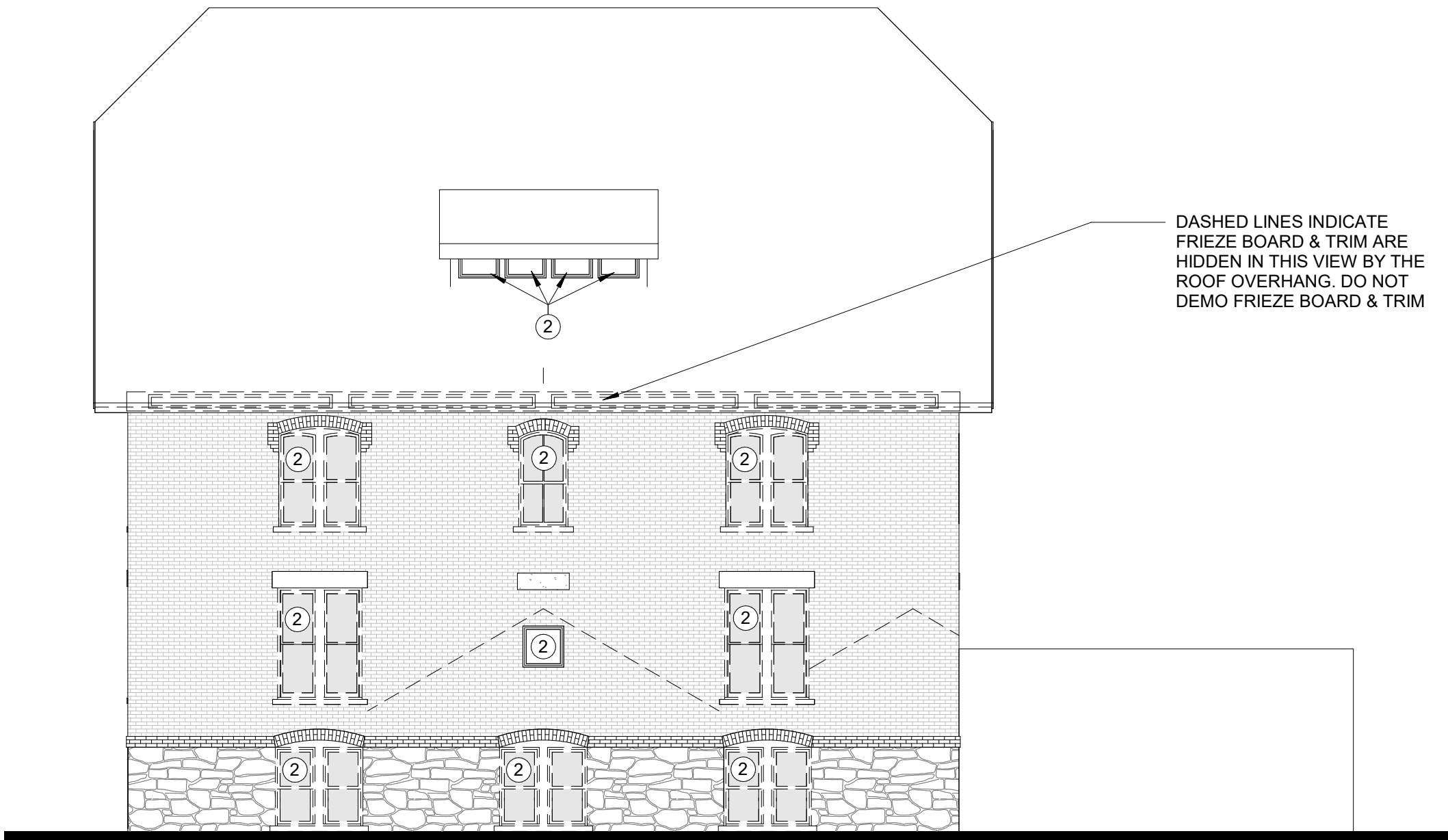
DASHED LINES INDICATE
FRIEZE BOARD & TRIM ARE
HIDDEN IN THIS VIEW BY THE
ROOF OVERHANG. DO NOT
DEMO FRIEZE BOARD & TRIM

DEMOLITION OF ENTRY
CANOPY, STAIR AND
DOOR ARE PART OF THE
BASE BID SCOPE

1 NORTH ELEVATION - DEMOLITION
A0.4 1/8" = 1'-0"

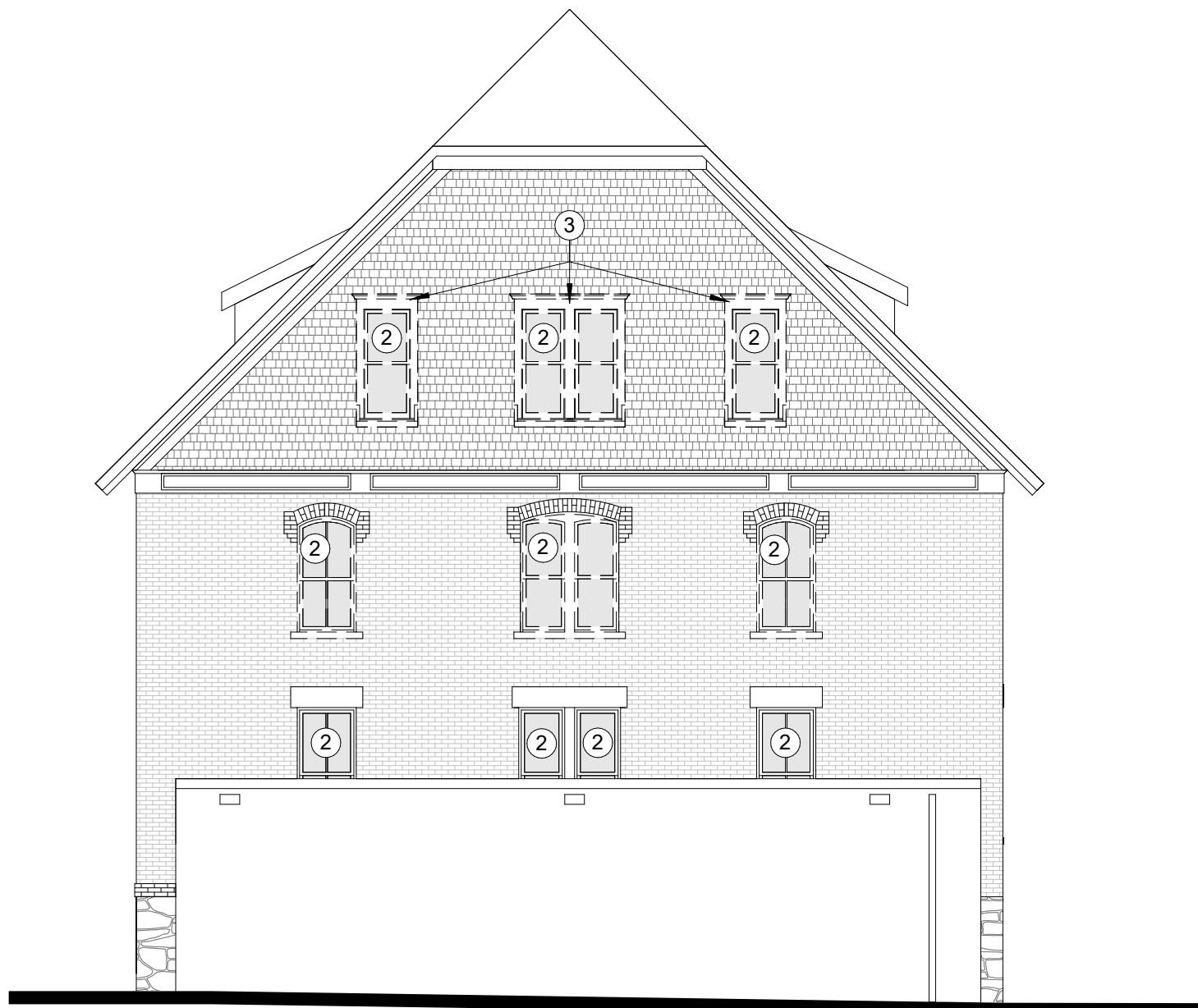


2 WEST ELEVATION - DEMOLITION
A0.4 1/8" = 1'-0"



DASHED LINES INDICATE
FRIEZE BOARD & TRIM ARE
HIDDEN IN THIS VIEW BY THE
ROOF OVERHANG. DO NOT
DEMO FRIEZE BOARD & TRIM

3 SOUTH ELEVATION - DEMOLITION
A0.4 1/8" = 1'-0"



4 EAST ELEVATION - DEMOLITION
A0.4 1/8" = 1'-0"

KEYED DEMOLITION NOTES FOR DRAWINGS 1,2,3,4 / A0.4

- 1 REMOVE EXISTING CONCRETE APRON IN ITS ENTIRETY. CLEAN SURFACE OF STONE FOUNDATION OF ALL REMAINING CONCRETE DEBRIS. REPOINT ALL MASONRY JOINTS IN THIS AREA.
- 2 REMOVE EXISTING WINDOW UNIT IN ITS ENTIRETY. CAULKING AROUND WINDOW IS EXPECTED TO BE ASBESTOS CONTAINING. REFER TO HAZARDOUS MATERIAL REPORT AND GENERAL NOTES BELOW.
- 3 CAREFULLY REMOVE EXISTING WINDOW TRIM AS REQUIRED FOR WINDOW DEMOLITION. SALVAGE TRIM TO BE REINSTALLED. IF UNABLE TO REINSTALL DOCUMENT EXISTING PROFILE SO NEW TRIM CAN MATCH

NOTES:

1. ALL DEMO WORK DESCRIBED ON THIS SHEET IS ALTERNATE #2 UNLESS SPECIFICALLY NOTED OTHERWISE

2. HAZARDOUS MATERIALS REPORT HAS BEEN PROVIDED IN THE PROJECT MANUAL. ALL HAZARDOUS MATERIAL ABATEMENT IS TO BE DONE BY THE CONTRACTOR.

- CAULKING BETWEEN DOOR/WINDOW AND BUILDING IS ANTICIPATED TO BE ASBESTOS CONTAINING. ABATEMENT MUST BE PERFORMED BY A MONTANA ACCREDITED ASBESTOS ABATEMENT CONTRACTOR ACCORDING TO LOCAL, STATE, AND FEDERAL REGULATIONS.

- EXTERIOR PAINT IS EXPECTED TO BE LEAD CONTAINING. ALL DEMOLITION OF LEAD BASED PAINT CONTAINING ITEMS MUST BE DONE ACCORDING TO LOCAL, STATE AND FEDERAL REGULATIONS.

3. IT IS THE CONTRACTORS RESPONSIBILITY TO SUPPORT EXISTING MASONRY AS REQUIRED DURING DEMO OPERATIONS



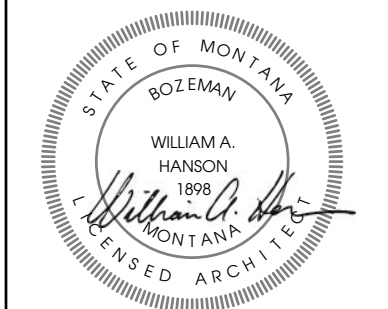
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DEMO - ALT #2

SHEET
A0.4

DATE
03/10/25

STRUCTURAL SHEET LIST		
SHEET NUMBER	SHEET TITLE	
S0.1	GENERAL NOTES	
S0.2	GENERAL NOTES	
S0.3	GENERAL NOTES	
S0.4	GENERAL NOTES	
S0.5	GENERAL NOTES	
S1.1	PLANS	
S2.1	CONCRETE DETAILS	
S3.1	STEEL DETAILS	
Sheet Total: 8		

ABBREVIATION					
L	Angle	EXCAV	Excavation	PLF	Pounds Per Linear Foot
AB	Anchor Bolt	FB	Factory-Built	PLWD	Plywood
ADDL	Additional	FD	Floor Drain	PREFAB	Prefabricated
ADH	Adhesive	FDN	Foundation	PSF	Pounds per Square Foot
ALT	Alternate	FIN	Finish	PSI	Pounds Per Square Inch
ARCH	Architectural	FLR	Floor	PSL	Parallel Strand Lumber
B or BOT	Bottom	FRP	Fiberglass Reinforced Plastic	P-T	Post-Tensioned
B/	Bottom Of	FRT	Fire Retardant Treated	PT	Pressure Treated
BLDG	Building	FTG	Footing	R	Radius
BLKG	Blocking	F/	Face of	RD	Roof Drain
BMU	Brick Masonry Unit	GA	Gage	REF	Refer/Reference
BP	Baseplate	GALV	Galvanized	REINF	Reinforcing
BRBF	Buckling Restrained	GEOTECH	Geotechnical	REQD	Required
	Braced Frame	GL	Glue Laminated Timber	RET	Retaining
BRG	Bearing	GWB	Gypsum Wall Board	SB	Site-Built
BTWN	Between	HDR	Header	SCBF	Special Concentric
C	Camber	HF	Hem-Fir		Braced Frame
CB	Castellated Beam	HGR	Hanger		Schedule
C BORE	Counterbore	HD	Hold-down	SER	Structural Engineer of
CL or C	Centerline	HORIZ	Horizontal	Record	
CLT	Cross-Laminated Timber	HP	High Point	SFRS	Seismic Force-
CIP	Cast in Place	HSS = TS	(Hollow Structural Section)		Resisting System
CFS	Cold Formed Steel	IBC	International Building Code	SHTHG	Sheathing
CJ	Construction or	ID	Inside Diameter	SIM	Similar
CJP	Control Joint	IE	Invert Elevation	SLBB	Short Leg Back-to-Back
	Complete Joint	IF	Inside Face	SMF	Special Moment Frame
	Penetration	INT	Interior	SOG	Slab on Grade
CLR	Clear	k	Kips	SP	Southern Pine
CLG	Ceiling	KSF	Kips Per Square Foot	SPEC	Specification
CMU	Concrete Masonry Unit	LF	Lineal Foot	SQ	Square
COL	Column	LL	Live Load	SR	Studrail
CONC	Concrete	LLBB	Long Leg Back-to-Back	SF	Square Foot
CONN	Connection	LLH	Long Leg Horizontal	SST	Stainless Steel
CONST	Construction	LLV	Long Leg Vertical	STAGG	Stagger/Staggered
CONT	Continuous	LP	Low Point	STD	Standard
C SINK	Countersink	LONGIT	Longitudinal	STIFF	Stiffener
CTRD	Centered	LSL	Laminated Strand Lumber	STL	Steel
DIA	Diameter	LVL	Laminated Veneer Lumber	STRUCT	Structural
DB	Drop Beam	MAS	Masonry	SWWJ	Solid Web Wood Joist
DBA	Deformed Bar Anchor	MAX	Maximum	SYM	Symmetrical
DBL	Double	MECH	Mechanical	T	Top
DEMO	Demolish	MEP	Mechanical, Electrical,	T/	Top Of
DEV	Development		Plumbing	T&B	Top & Bottom
DF	Douglas Fir	MEZZ	Mezzanine	TC AX LD	Top Chord Axial Load
DIAG	Diagonal	MFR	Manufacturer	TCX	Top Chord Extension
DIST	Distributed	MIN	Minimum	TDS	Tie Down System
DL	Dead Load	MISC	Miscellaneous	T&G	Tongue & Groove
DN	Down	NIC	Not In Contract	THKND	Thickened
DO	Ditto	NLT	Nail-Laminated Timber	THRD	Threaded
DP	Depth/Deep	NTS	Not To Scale	THRU	Through
DWG	Drawing	OC	On Center	TRANSV	Transverse
(EI)	Existing	OCBF	Ordinary Concentric Braced	TYP	Typical
EA	Each		Frame	UNO	Unless Noted Otherwise
EF	Each Face	OD	Outside Diameter	URM	Unreinforced Masonry
EL	Elevation	OF	Outside Face	Unit	
ELEC	Electrical	OPNG	Opening	VERT	Vertical
ELEV	Elevator	OPP	Opposite	W	Wide
EMBED	Embedment	OWSJ	Open Web Steel Joist	W/	With
EQ	Equal	OWWJ	Open Web Wood Joist	W/O	Without
EQUIP	Equipment	PL	Plate	WHS	Welded Headed Stud
EW	Each Way	PAF	Powder Actuated Fastener	WP	Working Point
EXP	Expansion	PC	Precast	WWF	Welded Wire Fabric
EXP JT	Expansion Joint	PERP	Perpendicular	±	Plus or Minus
EXT	Exterior	PJP	Partial Joint Penetration		

DRAWING LEGEND			
MARK	DESCRIPTION	MARK	DESCRIPTION
F2.0	FOOTING SYMBOL (REFER TO SPREAD FOOTING SCHEDULE)	I	INDICATES WIDE FLANGE COLUMN
P1	PILE CAP SYMBOL (REFER TO PILE CAP SCHEDULE)	□	INDICATES HOLLOW STRUCTURAL SECTION (HSS) COLUMN OR TUBE STEEL (TS) COLUMN
1	TILT-UP/PRECAST CONCRETE WALL CONNECTION SYMBOL (REFER TO CONNECTION DETAIL)	○	INDICATES HOLLOW STRUCTURAL SECTION (HSS) COLUMN OR STEEL PIPE COLUMN
2W4	SHEAR WALL SYMBOL (REFER TO SHEAR WALL SCHEDULE)	⌘	INDICATES WOOD POST
0 RFI 00	REVISION TRIANGLE	■	INDICATES BUNDLED STUDS
1	TILT-UP/PRECAST CONCRETE WALL PANEL NUMBER (REFER TO TILT-UP/ PRECAST CONCRETE WALL ELEVATIONS)	■	INDICATES CONCRETE COLUMN
1	CMU WALL REINFORCING SYMBOL (REFER TO CMU WALL REINFORCING SCHEDULE)	■	INDICATES PRECAST CONCRETE COLUMN
8"	CONTINUITY PLATE LENGTH (REFER TO TYPICAL DETAIL)	┐	INDICATES MOMENT FRAME CONNECTION
DS	INDICATES DOUBLE SHEAR CONNECTION (REFER TO THE DOUBLE SHEAR PLATE CONNECTIONS DETAIL)	┐	INDICATES PARTIALLY RESTRAINED MOMENT FRAME CONNECTION
00TB	INDICATES REINFORCING TYPE (REFER TO THE REINFORCING SCHEDULE)	┐	INDICATES CANTILEVER CONNECTION
SR	INDICATES NUMBER OF STUD RAIL REQUIRED AT COLUMN (REFER TO STUD RAIL DETAILS)	┐	INDICATES DRAG CONNECTION
1	ROOF/FLOOR DIAPHRAGM NAILING SYMBOL (REFER TO DIAPHRAGM NAILING SCHEDULE)	┐ - - - ┐	INDICATES A LEDGER
C1	STEEL/CONCRETE COLUMN SYMBOL (REFER TO STEEL COLUMN SCHEDULE)	┐ - - - ┐	INDICATES WOOD OR STEEL STUD BEARING WALL LINE PER KEY ON SHEET
T/FTG = X'-X"	ELEVATION SYMBOL (T/ REFERS TO COMPONENT THAT THE ELEVATION REFERENCES)	┐ - - - ┐ OR ┐ - - - ┐	INDICATES WOOD OR STEEL STUD SHEAR WALL LINE AND HOLD-DOWNS PER KEY ON SHEET
3	STUD BUBBLE INDICATES NUMBER OF STUDS REQUIRED IF EXCEEDS NUMBER SPECIFIED IN PLAN NOTE)	┐ - - - ┐	INDICATES MASONRY/CMU WALL
3	INDICATES STEP IN FOOTING (REFER TO TYPICAL STEP IN FOOTING DETAIL)	┐ - - - ┐	INDICATES CONCRETE/TILT-UP CONCRETE WALL
XX (SXXX)	DETAILS OR SECTION CUT (DETAIL NUMBER/SHEET NUMBER)	┐ - - - ┐	INDICATES BEARING WALL BELOW
00 S0.0	DETAILS OR SECTION CUT IN PLAN VIEW (DETAIL NUMBER/SHEET NUMBER)	┐ - - - ┐	INDICATES EXISTING WALL
XXVSXX,XX	INDICATES LOCATION OF CONCRETE WALLS, SHEAR WALLS OR BRACED FRAME ELEVATIONS	┐	POST-TENSION DEAD END (PLAN)
┐	STRUCTURAL EXTENT SYMBOL SINGLE ARROW - END OF EXTENT DOUBLE ARROW - CONTINUOUS EXTENT ALONG THE ELEMENT LINE UNTIL THE ELEMENT IS INTERRUPTED	┐	POST-TENSION STRESSING END (PLAN)
┐		┐ 3 ┐	POST-TENSION PROFILE (PLAN) (IN INCHES)
┐	INDICATES DIRECTION OF DECK SPAN	┐ - - - ┐	INTERMEDIATE STRESSING (PLAN)
┐	INDICATES AN OPENING IN A STEEL OR WOOD BEAM	┐ - - - ┐	INDICATES REQUIRED SHORING (PLAN)

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CIVIL / STRUCTURAL

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MSU-CPDC

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100% CONSTRUCTION DOCUMENTS

TAYLOR HALL
RENOVATIONS

MONTANA STATE
UNIVERSITY

THINKONE

DRAWN BY: WCG		
REVIEWED BY: JAL		
REV.	DESCRIPTION	DATE

MONTANA

JONATHAN A. LOVGREN

No. 46847 PE

PPA#17-0190

A/E#18-02-03

CONSULTANT #1805

SHEET TITLE
GENERAL NOTES

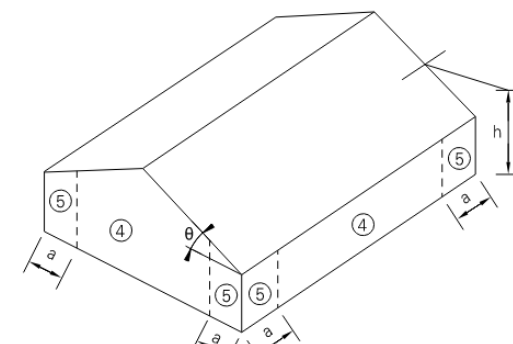
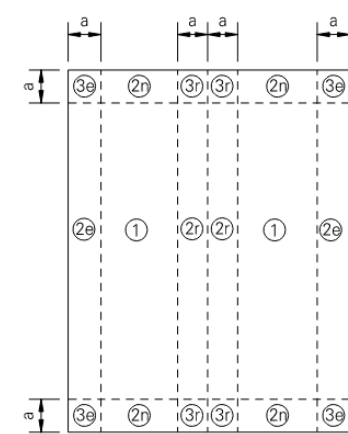
SHEET
S0.1

DATE
3/10/2025

01.00.00 GENERAL REQUIREMENTS

1. **GOVERNING CODE:** THE DESIGN AND CONSTRUCTION OF THIS PROJECT IS GOVERNED BY THE "INTERNATIONAL BUILDING CODE (IBC)," 2021 EDITION, HEREAFTER REFERRED TO AS THE IBC, AS ADOPTED AND MODIFIED BY THE CITY OF BOZEMAN, MT UNDERSTOOD TO BE THE AUTHORITY HAVING JURISDICTION (AHJ).
2. **REFERENCE STANDARDS:** REFER TO CHAPTER 35 OF THE 2021 IBC. WHERE OTHER STANDARDS ARE NOTED IN THE DRAWINGS, USE THE LATEST EDITION OF THE STANDARD UNLESS A SPECIFIC DATE IS INDICATED. REFERENCE TO A SPECIFIC SECTION IN A CODE DOES NOT RELIEVE THE CONTRACTOR FROM COMPLIANCE WITH THE ENTIRE STANDARD.
3. **DEFINITIONS:** THE FOLLOWING DEFINITIONS COVER THE MEANINGS OF CERTAIN TERMS USED IN THESE NOTES:
- 3.1. "ARCHITECT/ENGINEER" - THE ARCHITECT OF RECORD AND THE STRUCTURAL ENGINEER OF RECORD.
- 3.2. "STRUCTURAL ENGINEER OF RECORD" (SER) - THE STRUCTURAL ENGINEER WHO IS LICENSED TO STAMP AND SIGN THE STRUCTURAL DOCUMENTS FOR THE PROJECT. THE SER IS RESPONSIBLE FOR THE DESIGN OF THE PRIMARY STRUCTURAL SYSTEM.
- 3.3. "SUBMIT FOR REVIEW" - SUBMIT TO THE ARCHITECT/SER FOR REVIEW PRIOR TO FABRICATION OR CONSTRUCTION.
- 3.4. "PER PLAN" - INDICATES REFERENCES TO THE STRUCTURAL PLANS, ELEVATIONS, AND STRUCTURAL GENERAL NOTES.
- 3.5. "SPECIALTY STRUCTURAL ENGINEER" (SSE) - A PROFESSIONAL ENGINEER (PE OR SE), LICENSED IN THE STATE IN WHICH THE PROJECT IS LOCATED (TYPICALLY NOT THE SER), WHO PERFORMS SPECIALTY STRUCTURAL ENGINEERING SERVICES FOR SELECTED SPECIALTY-ENGINEERED ELEMENTS IDENTIFIED IN THE CONTRACT DOCUMENTS AND WHO HAS EXPERIENCE AND TRAINING IN THE SPECIALTY. DOCUMENTS STAMPED AND SIGNED BY THE SSE SHALL BE COMPLETED BY, OR UNDER, THE DIRECT SUPERVISION OF THE SSE.
- 3.6. "BIDDER-DESIGNED" - COMPONENTS OF THE STRUCTURE THAT REQUIRE THE GENERAL CONTRACTOR, SUBCONTRACTOR, OR SUPPLIER WHO IS RESPONSIBLE FOR THE DESIGN, FABRICATION, AND INSTALLATION OF SPECIALTY-ENGINEERED ELEMENTS IDENTIFIED IN THE CONTRACT DOCUMENTS TO RETAIN THE SERVICES OF AN SSE. SUBMITTALS OF "BIDDER-DESIGNED" ELEMENTS SHALL BE STAMPED AND SIGNED BY THE SSE.
4. **SPECIFICATIONS:** REFER TO THE PROJECT SPECIFICATIONS ISSUED AS PART OF THE CONTRACT DOCUMENTS FOR INFORMATION SUPPLEMENTAL TO THESE DRAWINGS.
5. **OTHER DRAWINGS:** REFER TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, CIVIL AND PLUMBING DRAWINGS FOR ADDITIONAL INFORMATION INCLUDING, BUT NOT LIMITED TO, DIMENSIONS, ELEVATIONS, SLOPES, DOOR AND WINDOW OPENINGS, NON-BEARING WALLS, STAIRS, FINISHES, DRAINS, WATERPROOFING, RAILINGS, AND OTHER NON-STRUCTURAL ITEMS.
6. **STRUCTURAL DETAILS:** THE STRUCTURAL DRAWINGS ARE INTENDED TO SHOW THE GENERAL CHARACTER AND EXTENT OF THE PROJECT AND ARE NOT INTENDED TO SHOW ALL DETAILS OF THE WORK. USE ENTIRE DETAIL SHEETS AND SPECIFIC DETAILS REFERENCED IN THE PLANS AS "TYPICAL" WHEREVER THEY APPLY. SIMILARLY, USE DETAILS ON ENTIRE SHEETS WITH "TYPICAL" IN THE NAME WHEREVER THEY APPLY.
7. **STRUCTURAL RESPONSIBILITIES:** THE SER IS RESPONSIBLE FOR THE STRENGTH AND STABILITY OF THE PRIMARY STRUCTURE IN ITS COMPLETED FORM.
8. **COORDINATION:** THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING DETAILS AND ACCURACY OF THE WORK, CONFIRMING AND CORRELATING ALL QUANTITIES AND DIMENSIONS, SELECTING FABRICATION PROCESSES, TECHNIQUES OF ASSEMBLY, AND PERFORMING WORK IN A SAFE AND SECURE MANNER.
9. **EXISTING CONDITIONS:** INFORMATION SHOWN ON THE DRAWINGS RELATED TO EXISTING CONDITIONS REPRESENT THE PRESENT KNOWLEDGE BUT WITHOUT GUARANTEE OF ACCURACY. REPORT CONDITIONS THAT CONFLICT WITH CONTRACT DOCUMENTS TO THE ARCHITECT OR SER. DO NOT DEVIATE FROM THE CONTRACT DOCUMENTS WITHOUT WRITTEN DIRECTION FROM THE ARCHITECT AND/OR SER. ALL EXISTING DIMENSIONS AND INFORMATION SHALL BE FIELD VERIFIED PRIOR TO FABRICATION AS REQUIRED TO COORDINATE WITH NEW CONSTRUCTION.
10. **NEW CONSTRUCTION:** THE CONTRACTOR SHALL REMOVE ALL INTERFERING ITEMS FOR NEW CONSTRUCTION AND SHALL REPAIR OR REPLACE ALL REMOVED ITEMS TO MATCH THE EXISTING CONDITIONS IN ACCORDANCE WITH THE ARCHITECTURAL DRAWINGS. NEW CONSTRUCTION ELEMENTS SHALL BE DESIGNED AND INSTALLED PER THE CURRENT IBC AS ALLOWED BY THE IBC.
11. **PRE-CONSTRUCTION MEETINGS:** THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING PRE-CONSTRUCTION MEETINGS PRIOR TO COMMENCING WORK. PRE-CON MEETINGS, SCHEDULED APPROXIMATELY TWO WEEKS PRIOR TO THE START OF THE RELEVANT WORK, ARE REQUIRED FOR THE FOLLOWING PHASES OF CONSTRUCTION: STRUCTURAL STEEL, ATTENDEES FOR PRE-CONSTRUCTION MEETING ARE TO INCLUDE THE CONTRACTOR, RELEVANT SUBCONTRACTORS, FABRICATORS, INSPECTORS, ARCHITECT/ENGINEER, AND A REPRESENTATIVE OF THE AHJ WHERE REQUIRED. MEETING AGENDAS ARE TO INCLUDE REVIEW OF THE WORK SCOPE, PROJECT SCHEDULE RELEVANT TO THE WORK, CONTACT INFORMATION OF RESPONSIBLE PARTIES, INSPECTION POINTS, REVIEW OF MATERIALS AND ANY SPECIAL CASES OR ISSUES, PROCEDURES FOR CLARIFICATIONS IF REQUIRED, TESTING AND ACCEPTANCE, ETC.
12. **MEANS, METHODS, AND SAFETY REQUIREMENTS:** THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION AND ALL JOB-RELATED SAFETY STANDARDS SUCH AS OSHA AND DOSH (DEPARTMENT OF OCCUPATIONAL SAFETY AND HEALTH). THE CONTRACTOR IS RESPONSIBLE TO ADHERE TO OSHA REGULATIONS REGARDING STEEL ERECTION ITEMS SPECIFICALLY ADDRESSED IN THE LATEST OSHA REGULATIONS. BOLTING AND FIELD WELDING AT ALL MEMBER CONNECTIONS IS TO BE COMPLETED PRIOR TO THE RELEASE OF THE MEMBER FROM THE HOISTING MECHANISM UNLESS REVIEWED AND APPROVED BY THE GENERAL CONTRACTOR'S TEMPORARY BRACING AND/OR SHORING DESIGN ENGINEER. THE CONSTRUCTION DOCUMENTS REPRESENT THE COMPLETED STRUCTURE. THE CONTRACTOR IS RESPONSIBLE FOR MEANS AND METHODS OF CONSTRUCTION RELATED TO THE INTERMEDIATE STRUCTURAL CONDITIONS (I.E., MOVEMENT OF THE STRUCTURE DUE TO MOISTURE AND THERMAL EFFECTS, CONSTRUCTION SEQUENCE, TEMPORARY BRACING, ETC.).
13. **BRACING/SHORING DESIGN ENGINEER:** THE CONTRACTOR SHALL, AT THEIR DISCRETION, EMPLOY AN SSE FOR THE DESIGN OF ANY TEMPORARY BRACING AND SHORING. SUBMIT CONSTRUCTION SEQUENCE TO ARCHITECT/ENGINEER FOR REVIEW.
14. **TEMPORARY SHORING, BRACING:** THE CONTRACTOR IS RESPONSIBLE FOR THE STRENGTH AND STABILITY OF THE STRUCTURE DURING CONSTRUCTION AND SHALL PROVIDE TEMPORARY SHORING, BRACING, AND OTHER ELEMENTS REQUIRED TO MAINTAIN STABILITY UNTIL THE STRUCTURE IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO BE FAMILIAR WITH THE WORK REQUIRED IN THE CONSTRUCTION DOCUMENTS AND THE REQUIREMENTS FOR EXECUTING IT PROPERLY.
15. **CONSTRUCTION LOADS:** LOADS ON THE STRUCTURE DURING CONSTRUCTION SHALL NOT EXCEED THE DESIGN LOADS AS NOTED IN DESIGN CRITERIA AND LOADS SECTION OF THESE GENERAL NOTES OR THE CAPACITY OF PARTIALLY COMPLETED CONSTRUCTION AS DETERMINED BY THE CONTRACTOR'S SSE FOR BRACING/SHORING.
16. **CHANGES IN LOADING:** THE CONTRACTOR HAS THE RESPONSIBILITY TO NOTIFY THE SER OF ANY ARCHITECTURAL, MECHANICAL, ELECTRICAL, OR PLUMBING LOAD IMPOSING ONTO THE STRUCTURE THAT DIFFERS FROM, OR THAT IS NOT DOCUMENTED ON, THE ORIGINAL CONTRACT DOCUMENTS (ARCHITECTURAL, STRUCTURAL, MECHANICAL, ELECTRICAL, OR PLUMBING DRAWINGS). PROVIDE DOCUMENTATION OF THE LOCATION, LOAD, SIZE, AND ANCHORAGE OF ALL UNDOCUMENTED LOADS IN EXCESS OF 400 LB. PROVIDE MARKED-UP STRUCTURAL PLANS INDICATING THE LOCATIONS OF ANY NEW EQUIPMENT OR LOADS. SUBMIT THESE PLANS TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO INSTALLATION.
17. **NOTE PRIORITIES:** PLAN AND DETAIL NOTES AND SPECIFIC LOADING DATA PROVIDED ON INDIVIDUAL PLANS AND DETAIL DRAWINGS SUPPLEMENTS INFORMATION IN THESE GENERAL NOTES.
18. **DISCREPANCIES:** IN CASE OF DISCREPANCIES BETWEEN THE GENERAL NOTES, SPECIFICATIONS, PLANS/DETAILS, OR REFERENCE STANDARDS, THE ARCHITECT/ENGINEER SHALL DETERMINE WHICH SHALL GOVERN. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER BEFORE PROCEEDING WITH THE WORK. SHOULD ANY DISCREPANCY BE FOUND IN THE CONTRACT DOCUMENTS, THE CONTRACTOR WILL BE DEEMED TO HAVE INCLUDED IN THE PRICE THE MOST EXPENSIVE WAY OF COMPLETING THE WORK UNLESS PRIOR TO THE SUBMISSION OF THE PRICE, THE CONTRACTOR ASKS FOR A DECISION FROM THE ARCHITECT AS TO WHICH SHALL GOVERN. ACCORDINGLY, ANY CONFLICT IN OR BETWEEN THE CONTRACT DOCUMENTS SHALL NOT BE A BASIS FOR ADJUSTMENT IN THE CONTRACT PRICE.
19. **SITE VERIFICATION:** THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AT THE SITE. CONFLICTS BETWEEN THE DRAWINGS AND ACTUAL SITE CONDITIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER BEFORE PROCEEDING WITH THE WORK.
20. **ADJACENT UTILITIES:** THE CONTRACTOR SHALL DETERMINE THE LOCATION OF ALL ADJACENT UNDERGROUND UTILITIES PRIOR TO EARTHWORK, FOUNDATION WORK, SHORING AND EXCAVATION. ANY UTILITY INFORMATION SHOWN ON THE DRAWINGS AND DETAILS IS APPROXIMATE AND NOT NECESSARILY COMPLETE.
21. **ALTERNATES:** ALTERNATE PRODUCTS OF SIMILAR STRENGTH, NATURE, AND FORM FOR SPECIFIED ITEMS MAY BE SUBMITTED WITH ADEQUATE TECHNICAL DOCUMENTATION (PROPER TEST REPORT, ETC.) TO THE ARCHITECT/ENGINEER FOR REVIEW. ALTERNATE MATERIALS THAT ARE SUBMITTED WITHOUT ADEQUATE TECHNICAL DOCUMENTATION OR THAT SIGNIFICANTLY DEVIATE FROM THE DESIGN INTENT OF MATERIALS SPECIFIED MAY BE RETURNED WITHOUT REVIEW. ALTERNATES THAT REQUIRE SUBSTANTIAL EFFORT TO REVIEW WILL NOT BE REVIEWED UNLESS AUTHORIZED BY THE OWNER.
22. **ADDITIONS/ALTERATIONS/REPAIRS:**
- 22.1. ADDITIONS, ALTERATIONS, AND/OR REPAIRS TO THE EXISTING STRUCTURE HAVE BEEN ANALYZED FOR ADDITIONAL LOADING AND/OR MODIFICATION DUE TO THE ADDITION, ALTERATION, OR REPAIR.
- 22.2. ALL AFFECTED EXISTING MEMBERS HAVE BEEN ANALYZED OR REINFORCED AS REQUIRED PER THE IBC.
- 22.3. ALL DEMOLITION OR REMOVAL OF ARCHITECTURAL, MECHANICAL, OR STRUCTURAL ELEMENTS SHALL NOT DAMAGE STRUCTURAL ITEMS TO REMAIN.

01.10.00 DESIGN CRITERIA AND LOADS

1. **OCCUPANCY:**
RISK CATEGORY OF BUILDING PER IBC TABLE 1604.5 II
2. **WIND DESIGN: MAIN WIND FORCE RESISTING SYSTEM:**
ULTIMATE DESIGN WIND SPEED, V_{ULT} 115 MPH
EXPOSURE CATEGORY B
INTERNAL PRESSURE COEFFICIENT, C_{pi} ±0.55
TOPOGRAPHIC FACTOR, K_z 1.0
WIND ANALYSIS PROCEDURE USED DIRECTIONAL
3. **WIND DESIGN: COMPONENTS AND CLADDING (C&C) PRESSURES FOR DESIGN (PSF, ULTIMATE):**
- 
- | | PLAN | ISOMETRIC |
|-----------|-----------|-----------------------------|
| | | EFFECTIVE WIND AREA (FT²) |
| a = 3'-0" | 10 | 20 |
| ZONE 1 | -38 / +16 | -38 / +16 |
| ZONE 2a | -38 / +16 | -38 / +16 |
| ZONE 2b | -52 / +16 | -46 / +16 |
| ZONE 2c | -52 / +16 | -46 / +16 |
| ZONE 2d | -52 / +16 | -46 / +16 |
| ZONE 2e | -52 / +16 | -46 / +16 |
| ZONE 2f | -52 / +16 | -46 / +16 |
| ZONE 2g | -52 / +16 | -46 / +16 |
| ZONE 2h | -52 / +16 | -46 / +16 |
| ZONE 2i | -52 / +16 | -46 / +16 |
| ZONE 2j | -52 / +16 | -46 / +16 |
| ZONE 2k | -52 / +16 | -46 / +16 |
| ZONE 2l | -52 / +16 | -46 / +16 |
| ZONE 2m | -52 / +16 | -46 / +16 |
| ZONE 2n | -52 / +16 | -46 / +16 |
| ZONE 2o | -52 / +16 | -46 / +16 |
| ZONE 2p | -52 / +16 | -46 / +16 |
| ZONE 2q | -52 / +16 | -46 / +16 |
| ZONE 2r | -52 / +16 | -46 / +16 |
| ZONE 2s | -52 / +16 | -46 / +16 |
| ZONE 2t | -52 / +16 | -46 / +16 |
| ZONE 2u | -52 / +16 | -46 / +16 |
| ZONE 2v | -52 / +16 | -46 / +16 |
| ZONE 2w | -52 / +16 | -46 / +16 |
| ZONE 2x | -52 / +16 | -46 / +16 |
| ZONE 2y | -52 / +16 | -46 / +16 |
| ZONE 2z | -52 / +16 | -46 / +16 |
| ZONE 3 | -61 / +16 | -53 / +16 |
| ZONE 4 | -23 / +22 | -22 / +21 |
| ZONE 5 | -27 / +22 | -26 / +21 |
- 3.1. COMPONENTS AND CLADDING WIND PRESSURES ARE BASED ON ASCE 7 CHAPTER 30 PART 1 "LOW-RISE BUILDINGS."
- 3.2. COMPONENTS AND CLADDING ZONE LOCATIONS ARE BASED ON ASCE 7 FIGURE 30.3-2B FOR GABLE ROOFS 7° < θ ≤ 20°.
- 3.3. COMPONENTS AND CLADDING ZONE LOCATIONS ARE BASED ON ASCE 7 FIGURE 30.3-1 FOR WALLS.
- 3.4. ALL PARAPET COMPONENTS AND CLADDING WIND PRESSURES SHALL BE DETERMINED THROUGH ASCE 7 FIGURE 30.6-2.
4. **SEISMIC DESIGN:**
SEISMIC DESIGN CATEGORY, SDC D
SITE CLASS PER IBC SECTION 1613.3.2 AND ASCE 7 CHAPTER 20 D
SEISMIC IMPORTANCE FACTOR PER ASCE 7 TABLE 1.5-2, I_e 1.0
SPECTRAL RESPONSE ACCELERATION (SHORT PERIOD), S_s 0.6 g
SPECTRAL RESPONSE ACCELERATION (1-SECOND PERIOD), S_1 0.2 g
DESIGN RESPONSE ACCELERATION (SHORT PERIOD), S_{DS} 0.528 g
DESIGN RESPONSE ACCELERATION (SHORT PERIOD), S_{D1} 0.293 g
COMPONENT RESPONSE MODIFICATION FACTOR PER ASCE 7 TABLE 13.5-1 R_e 2.5
COMPONENT AMPLIFICATION FACTOR PER ASCE 7 TABLE 13.5-1 a_o 1.0
SEISMIC ANALYSIS PROCEDURE USED EQUIVALENT LATERAL FORCE
5. **SNOW LOAD:**
FLAT ROOF SNOW LOAD, P_f 41 PSF [NOTE 5.1]
SNOW DRIFT LOADING REQUIRED BY AHJ? YES
SNOW LOAD IMPORTANCE FACTOR, I_s 1.00 [NOTE 5.2]
GROUND SNOW LOAD, P_g 46 PSF
SNOW EXPOSURE FACTOR, C_e 1.1
THERMAL FACTOR, C_t 1.0
- 5.1. SNOW LOAD IS UN-REDUCIBLE AND INCLUDES 5 PSF RAIN-ON-SNOW SURCHARGE WHERE GROUND SNOW LOAD IS 20 PSF OR LESS, BUT NOT ZERO, PER ASCE 7 SECTION 7.10.
- 5.2. SNOW LOAD IMPORTANCE FACTOR PER ASCE 7 TABLE 1.5-2.
6. **DESIGN DEAD LOADS:**
ROOF DEAD LOAD (IN ADDITION TO STRUCTURE SELF-WEIGHT) ROOFING 5 PSF
7. **DEFLECTION LIMITS FOR SSE / BIDDER-DESIGNED ELEMENTS:**
VERTICAL [NOTE 7.1] LIMIT
ROOF MEMBERS, DEAD + LIVE/SNOW/WIND, TOTAL LOAD (TL) L / 240
ROOF, LIVE/SNOW/WIND LOAD (RL) L / 360
HORIZONTAL [NOTE 7.2] L / 240
MEMBERS SUPPORTING BRITTLE FINISHES L / 180
MEMBERS SUPPORTING FLEXIBLE FINISHES L / 600 AT 0.7E OR 0.7 * C&C WIND LOAD
MEMBERS SUPPORTING MASONRY 0.020 * STORY HEIGHT
INTERSTORY DRIFT
- 7.1. MAXIMUM VERTICAL DEFLECTION IS WHICHEVER CRITERIA YIELDS LESS DEFLECTION. L IS THE CLEAR SPAN LENGTH IN INCHES.
- 7.2. WIND LOAD IS REDUCIBLE TO 0.42 * THE COMPONENT AND CLADDING (C&C) LOADS PER TABLE 1604.3 FOOTNOTE F.

01.20.00 SUBMITTALS

1. **SUBMIT FOR REVIEW:** SUBMITTALS OF SHOP DRAWINGS AND PRODUCT DATA ARE REQUIRED FOR ITEMS NOTED IN THE INDIVIDUAL MATERIALS SECTIONS OF THESE GENERAL NOTES AND FOR BIDDER-DESIGNED ELEMENTS.
2. **SUBMITTAL REVIEW PERIOD:** SUBMITTALS SHALL BE MADE IN TIME TO PROVIDE A MINIMUM OF TWO WEEKS OR TEN WORKING DAYS FOR REVIEW BY THE ARCHITECT/ENGINEER PRIOR TO THE ONSET OF FABRICATION.
3. **GENERAL CONTRACTOR'S PRIOR REVIEW:** PRIOR TO SUBMISSION TO THE ARCHITECT/ENGINEER, THE CONTRACTOR SHALL REVIEW THE SUBMITTAL FOR COMPLETENESS. DIMENSIONS AND QUANTITIES ARE NOT REVIEWED BY THE SER; THEREFORE, THEY MUST BE VERIFIED BY THE GENERAL CONTRACTOR. THE CONTRACTOR SHALL PROVIDE ANY NECESSARY DIMENSIONAL DETAILS REQUESTED BY THE DETAILER AND PROVIDE THE CONTRACTOR'S REVIEW STAMP AND SIGNATURE BEFORE FORWARDING TO THE ARCHITECT/ENGINEER.
4. **SHOP DRAWING REVIEW:** ONCE THE CONTRACTOR HAS COMPLETED THEIR REVIEW, THE SER WILL REVIEW THE SUBMITTAL FOR GENERAL CONFORMANCE WITH THE DESIGN CONCEPT AND THE CONTRACT DOCUMENTS OF THE BUILDING AND WILL STAMP THE SUBMITTAL ACCORDINGLY. MARKINGS OR COMMENTS SHALL NOT BE CONSTRUED AS RELIEVING THE CONTRACTOR FROM COMPLIANCE WITH THE PROJECT PLANS AND SPECIFICATIONS, NOR DEPARTURES THERE FROM. THE SER WILL RETURN SUBMITTALS IN THE FORM IN WHICH THEY ARE SUBMITTED (EITHER HARD COPY OR ELECTRONIC). FOR HARD COPY SUBMITTALS, THE CONTRACTOR IS RESPONSIBLE FOR SUBMITTING THE REQUIRED NUMBER OF COPIES TO THE SER FOR REVIEW.
5. **SHOP DRAWING DEVIATIONS:** WHEN SHOP DRAWINGS (COMPONENT DESIGN DRAWINGS) DIFFER FROM OR ADD TO THE REQUIREMENTS OF THE STRUCTURAL DRAWINGS, THEY SHALL BE DESIGNED AND STAMPED BY THE RESPONSIBLE SSE.

01.30.00 DEFERRED SUBMITTALS

1. **BIDDER-DESIGNED ELEMENTS:** SUBMIT "BIDDER-DESIGNED" DEFERRED SUBMITTALS TO THE ARCHITECT AND SER FOR REVIEW. THE DEFERRED SUBMITTALS SHALL ALSO BE SUBMITTED TO THE CITY FOR APPROVAL IF REQUIRED BY THE CITY. DESIGN OF PREFABRICATED, "BIDDER-DESIGNED," MANUFACTURED, PRE-ENGINEERED, OR OTHER FABRICATED PRODUCTS SHALL COMPLY WITH THE FOLLOWING REQUIREMENTS:
- 1.1. DESIGN CONSIDERS TRIBUTARY DEAD, LIVE, WIND, AND EARTHQUAKE LOADS IN COMBINATIONS REQUIRED BY THE IBC.
- 1.2. DESIGN WITHIN THE DEFLECTION LIMITS NOTED IN THESE GENERAL NOTES AND AS SPECIFIED OR REFERENCED IN THE IBC.
- 1.3. DESIGN SHALL CONFORM TO THE SPECIFICATIONS AND REFERENCE STANDARDS OF THE GOVERNING CODE.
- 1.4. SUBMITTAL SHALL INCLUDE:
- 1.4.1. CALCULATIONS PREPARED, STAMPED, AND SIGNED BY THE SSE DEMONSTRATING CODE CONFORMANCE.
- 1.4.2. ENGINEERED COMPONENT DESIGN DRAWINGS ARE PREPARED, STAMPED, AND SIGNED BY THE SSE.
- 1.4.3. PRODUCT DATA, TECHNICAL INFORMATION AND MANUFACTURER'S WRITTEN REQUIREMENTS, AND AGENCY APPROVALS AS APPLICABLE.
- 1.4.4. THE SSE MAY SUBMIT TO THE ARCHITECT/ENGINEER A REQUEST TO UTILIZE RELEVANT ALTERNATE DESIGN CRITERIA OF SIMILAR NATURE AND GENERAL EQUIVALENCY WHICH IS RECOGNIZED BY THE CODE AND ACCEPTABLE TO THE AHJ. SUBMIT ADEQUATE DOCUMENTATION OF DESIGN.
2. **GENERAL CONTRACTOR'S PRIOR REVIEW:**
- 2.1. ONCE THE CONTRACTOR HAS COMPLETED THEIR REVIEW OF THE SSE COMPONENT DRAWINGS, THE SER WILL REVIEW THE SUBMITTAL FOR GENERAL CONFORMANCE WITH THE DESIGN OF THE BUILDING AND WILL STAMP THE SUBMITTAL ACCORDINGLY.
- 2.2. REVIEW OF THE SSE'S SHOP DRAWINGS (COMPONENT DESIGN DRAWINGS) IS FOR COMPLIANCE WITH DESIGN CRITERIA AND COMPATIBILITY WITH THE DESIGN OF THE PRIMARY STRUCTURE AND DOES NOT RELIEVE THE SSE OF RESPONSIBILITY FOR THAT DESIGN.
- 2.3. ALL NECESSARY BRACING, TIES, ANCHORAGE, AND PROPRIETARY PRODUCTS SHALL BE FURNISHED AND INSTALLED PER MANUFACTURER'S INSTRUCTIONS OR THE SSE'S DESIGN DRAWINGS AND CALCULATIONS.
- 2.4. BIDDER-DESIGNED ELEMENTS INCLUDE BUT ARE NOT LIMITED TO:
- 2.4.1. STEEL STAIRS
- 2.4.2. HANDRAILS AND GUARDRAILS

01.40.00 INSPECTIONS, QUALITY ASSURANCE, AND TEST REQUIREMENTS

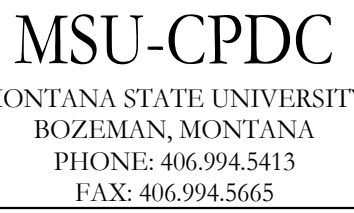
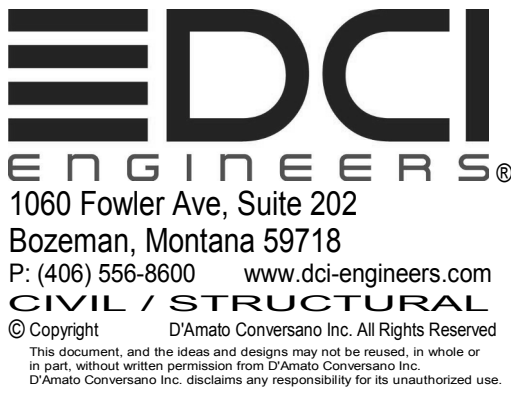
1. **INSPECTIONS:** FOUNDATIONS, FOOTINGS, AND UNDER SLAB SYSTEMS AND FRAMING ARE SUBJECT TO INSPECTION BY THE BUILDING OFFICIAL IN ACCORDANCE WITH IBC SECTION 110.3. THE CONTRACTOR SHALL COORDINATE ALL REQUIRED INSPECTIONS WITH THE BUILDING OFFICIAL.
2. **SPECIAL INSPECTIONS, VERIFICATIONS, AND TESTS:** SPECIAL INSPECTIONS, VERIFICATIONS, AND TESTING SHALL BE DONE IN ACCORDANCE WITH IBC CHAPTER 17, THE STATEMENT AND SCHEDULES OF SPECIAL INSPECTIONS LISTED IN THESE DRAWINGS, AND THE AHJ STATEMENT OF SPECIAL INSPECTION AND/OR STATEMENT OF STRUCTURAL OBSERVATIONS.
3. **STRUCTURAL OBSERVATION:**
- 3.1. STRUCTURAL OBSERVATION FOR THIS PROJECT IS REQUIRED PER IBC SECTION 1704.6. THE CONTRACTOR SHALL NOTIFY THE SER IN A TIMELY MANNER TO ALLOW REQUIRED STRUCTURAL OBSERVATIONS TO OCCUR. REPORTS WILL BE DISTRIBUTED TO THE ARCHITECT, CONTRACTOR, SPECIAL INSPECTOR, AND AHJ.
- 3.1.1. THE FREQUENCY AND EXTENT OF OBSERVATIONS IS AT THE DISCRETION OF THE STRUCTURAL OBSERVER.
- 3.1.2. ONLY SIGNIFICANT STAGES OF CONSTRUCTION IDENTIFIED BY THE STRUCTURAL OBSERVER REQUIRE OBSERVATION.
- 3.1.3. FOR REPETITIVE OR SIMILAR STRUCTURAL ELEMENTS IDENTIFIED AS SIGNIFICANT, ONLY THE FIRST ELEMENT OF A STAGE REQUIRES OBSERVATION UNLESS NOTED OTHERWISE.
- 3.1.4. THE FOLLOWING SIGNIFICANT STAGES OF CONSTRUCTION REQUIRE OBSERVATION:
- 3.1.4.1. PRIOR TO FOUNDATION CONCRETE PLACEMENT
- 3.1.4.2. PRIOR TO MAT FOUNDATION TOP BAR PLACEMENT
- 3.1.4.3. PRIOR TO FIRST MILD REINFORCED SLAB PLACEMENT
- 3.1.4.4. DURING THE FIRST ELEVATED FLOOR FRAMING
- 3.1.4.5. DURING ROOF FRAMING
- 3.1.4.6. AFTER ROOF DIAPHRAGM IS COMPLETE PRIOR TO ROOFING
4. **CONTRACTOR RESPONSIBILITY:** PRIOR TO ISSUANCE OF THE BUILDING PERMIT, THE CONTRACTOR IS REQUIRED TO PROVIDE THE AHJ A SIGNED, WRITTEN ACKNOWLEDGEMENT OF THE CONTRACTOR'S RESPONSIBILITIES ASSOCIATED WITH THE STATEMENT OF SPECIAL INSPECTIONS, PREVIOUSLY REFERENCED, ADDRESSING THE REQUIREMENTS LISTED IN IBC SECTION 1704.4. THE CONTRACTOR IS REFERRED TO IBC SECTIONS 1705.13.5 AND 1705.13.6 FOR ARCHITECTURAL AND MEP BUILDING SYSTEMS THAT MAY BE SUBJECT TO ADDITIONAL INSPECTIONS (BASED ON THE BUILDING'S DESIGNATED SEISMIC DESIGN CATEGORY LISTED IN THE DESIGN CRITERIA AND LOADS SECTION OF THESE GENERAL NOTES), INCLUDING ANCHORAGE OF HVAC DUCTWORK CONTAINING HAZARDOUS MATERIALS, PIPING SYSTEMS AND MECHANICAL UNITS CONTAINING FLAMMABLE, COMBUSTIBLE, OR HIGHLY TOXIC MATERIALS, ELECTRICAL EQUIPMENT USED FOR EMERGENCY OR STANDBY POWER, EXTERIOR WALL PANELS, AND SUSPENDED CEILING SYSTEMS.

02.00.00 SOILS AND FOUNDATION

1. **REFERENCE STANDARDS:** CONFORM TO IBC CHAPTER 18 "SOILS AND FOUNDATIONS"
2. **GEOTECHNICAL REPORT:** NO GEOTECHNICAL REPORT WAS OBTAINED PRIOR TO DESIGN. DESIGN BASED ON ALLOWABLE MINIMUM VALUES PER IBC CHAPTER 18.
3. **DESIGN SOIL VALUES:**
ALLOWABLE FOUNDATION BEARING PRESSURE 1500 PSF - NATIVE
4. **FOUNDATIONS AND FOOTINGS:** FOUNDATIONS SHALL BEAR ON COMPETENT NATIVE SOIL.
5. **FOOTING DEPTH:** TOPS OF FOOTINGS SHALL BE AS SHOWN ON PLANS WITH VERTICAL CHANGES AS INDICATED WITH STEPS IN THE FOOTINGS. LOCATIONS OF STEPS SHOWN ARE APPROXIMATE AND SHALL BE COORDINATED WITH THE CIVIL GRADING PLANS.
6. **SLABS-ON-GRADE:** ALL SLABS-ON-GRADE SHALL BEAR ON COMPETENT NATIVE SOIL.

03.20.00 CONCRETE REINFORCEMENT

1. **REFERENCE STANDARDS:** CONFORM TO:
- 1.1. ACI 301 "STANDARD SPECIFICATIONS FOR STRUCTURAL CONCRETE," SECTION 3 "REINFORCEMENT AND REINFORCEMENT SUPPORTS"
- 1.2. ACI SP-66 "ACI DETAILING MANUAL"
- 1.3. CRSI MSP "MANUAL OF STANDARD PRACTICE"
- 1.4. ANSIIAWS D1.4 "STRUCTURAL WELDING CODE - REINFORCING STEEL"
- 1.5. IBC CHAPTER 19 "CONCRETE"
- 1.6. ACI 318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"
- 1.7. ACI 117 "SPECIFICATIONS FOR TOLERANCES FOR CONCRETE CONSTRUCTION AND MATERIALS"
2. **SUBMITTALS:**
- 2.1. CONFORM TO ACI 301 SECTION 3.1.2. SUBMIT PLACING DRAWINGS SHOWING FABRICATION DIMENSIONS AND PLACEMENT LOCATIONS OF REINFORCEMENT AND REINFORCEMENT SUPPORTS.
3. **MATERIALS:**
REINFORCING BARS [NOTE 3.1] ASTM A615, GRADE 60, DEFORMED BARS
ASTM A706, GRADE 60, DEFORMED BARS
BAR SUPPORTS CRSI MSP CHAPTER 3
TIE WIRE 16 GAGE OR HEAVIER, BLACK ANNEALED
- 3.1. ASSUME GRADE 60 REINFORCEMENT UNLESS NOTED OTHERWISE ON PLAN. REFERENCE PLANS FOR HIGH STRENGTH REINFORCING LOCATIONS, INDICATED BY (GR 80) OR (GR 100).
4. **FABRICATION:** CONFORM TO ACI 301 SECTION 3.2.2 AND ACI SP-66.
5. **WELDING:** BARS SHALL NOT BE WELDED UNLESS AUTHORIZED. WHEN AUTHORIZED, CONFORM TO ACI 301 SECTION 3.2.2.2 AND AWS D1.4, AND PROVIDE ASTM A706, GRADE 60 REINFORCEMENT.



DRAWN BY: WCG	
REVIEWED BY: JAL	
REV.	DESCRIPTION



PPA#17-0190
A/E#18-02-03

CONSULTANT #1805

SHEET TITLE
GENERAL NOTES

SHEET
S0.2

DATE
3/10/2025

100% CONSTRUCTION DOCUMENTS

6. **PLACING:** CONFORM TO ACI 301 SECTION 3.3.2. PLACING TOLERANCES SHALL CONFORM TO ACI 117.
7. **CONCRETE COVER:** CONFORM TO THE FOLLOWING COVER REQUIREMENTS UNLESS NOTED OTHERWISE ON PLAN:
- | | |
|--------------------------------------|--------|
| CONCRETE CAST AGAINST EARTH | 3" |
| CONCRETE EXPOSED TO EARTH OR WEATHER | 2" |
| TIES IN COLUMNS AND BEAMS | 1-1/2" |
| BAR IN SLABS | 3/4" |
| BAR IN WALLS | 3/4" |
| EXTERIOR BARS IN TILT-UP PANELS | 1" |
8. **SPLICES:** CONFORM TO ACI 301 SECTION 3.3.2.7. REFER TO TYPICAL LAP SPlice AND DEVELOPMENT LENGTH SCHEDULE IN THESE DRAWINGS FOR TYPICAL REINFORCEMENT SPLICES.
9. **FIELD BENDING:** CONFORM TO ACI 301 SECTION 3.3.2.8. BAR SIZES #3 THROUGH #5 MAY BE FIELD BENT COLD THE FIRST TIME. SUBSEQUENT BENDS AND OTHER BAR SIZES REQUIRE PREHEATING. DO NOT TWIST BARS. BARS SHALL NOT BE BENT PAST 45°.

03.30.00 CAST-IN-PLACE CONCRETE

1. **REFERENCE STANDARDS:** CONFORM TO:
- 1.1. ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE"
 - 1.2. IBC CHAPTER 19 "CONCRETE"
 - 1.3. ACI 318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"
 - 1.4. ACI 117 "SPECIFICATIONS FOR TOLERANCES FOR CONCRETE CONSTRUCTION AND MATERIALS"
2. **FIELD REFERENCE:** THE CONTRACTOR SHALL KEEP A COPY OF ACI FIELD REFERENCE MANUAL SP-15.
3. **CONCRETE MIXTURES:** CONFORM TO ACI 301 SECTION 4 AND IBC SECTION 1904.1.
4. **MATERIALS:** CONFORM TO ACI 301 SECTION 4.2.1 FOR REQUIREMENTS FOR CEMENTITIOUS MATERIALS, AGGREGATES, MIXING WATER, AND ADMIXTURES.
5. **SUBMITTALS:**
- 5.1. PROVIDE ALL SUBMITTALS REQUIRED BY ACI 301 SECTION 4.1.2. SUBMIT MIX DESIGNS FOR EACH MIX IN THE TABLE BELOW. SUBSTANTIATING STRENGTH RESULTS FROM PAST TESTS SHALL NOT BE OLDER THAN 24 MONTHS PER ACI 318 SECTION 26.4.3.1(B).
6. **MIX DESIGN REQUIREMENTS [NOTES 7.1 TO 7.10, TYPICAL UNO]**
- | | |
|-------------------------------------|-----------|
| FOOTINGS & PIERS | |
| STRENGTH, f _c | 3,000 PSI |
| TEST AGE | 28 DAYS |
| NOMINAL MAXIMUM AGGREGATE | 1" |
| EXTERIOR SLABS ON GRADE & SIDEWALKS | |
| STRENGTH, f _c | 4,500 PSI |
| TEST AGE | 28 DAYS |
| NOMINAL MAXIMUM AGGREGATE | 1" |
| EXPOSURE CLASS | F2 |
| MAX W/C RATIO | 0.45 |
| AIR CONTENT | 6% |
7. **MIX DESIGN REQUIREMENT NOTES:**
- 7.1. W/C RATIO: WATER-CEMENTITIOUS MATERIAL RATIOS SHALL BE BASED ON THE TOTAL WEIGHT OF CEMENTITIOUS MATERIALS. MAXIMUM RATIOS ARE CONTROLLED BY STRENGTH NOTED IN THE TABLE OF MIX DESIGN REQUIREMENTS AND DURABILITY REQUIREMENTS GIVEN IN ACI 318 SECTION 19.3. W/C RATIOS MAY BE EXCEEDED WITH APPROVAL OF SER AS LONG AS POTENTIAL SHRINKAGE IMPACTS ARE ACCOUNTED FOR.
 - 7.2. CEMENTITIOUS MATERIALS:
 - 7.2.1. DCI ENCOURAGES THE REDUCTION OF CEMENT CONTENT AND/OR THE USE OF ALTERNATE CEMENTITIOUS MATERIALS. WHERE REQUIREMENTS OF THIS SECTION PROHIBIT INCLUSION OF ANY OF THESE MIXES, CONTACT DCI FOR FURTHER COORDINATION.
 - 7.2.2. CEMENTITIOUS MATERIALS SHALL CONFORM TO THE RELEVANT ASTM STANDARDS LISTED IN ACI 318 SECTION 26.4.1.1.1(A).
 - 7.2.3. FOR CONCRETE USED IN ELEVATED FLOORS, MINIMUM CEMENTITIOUS MATERIALS CONTENT SHALL CONFORM TO ACI 301 TABLE 4.2.1.1(B). ACCEPTANCE OF LOWER CEMENT CONTENT IS CONTINGENT ON PROVIDING SUPPORTING DATA TO THE SER FOR REVIEW AND ACCEPTANCE.
 - 7.3. GENERAL CONTRACTOR SHALL COORDINATE MEANS AND METHODS NECESSARY TO SUPPORT EXTENDED TEST AGES INCLUDING, BUT NOT LIMITED TO, DELAYED STRENGTH GAIN, SHORING SEQUENCING, MODULUS OF ELASTICITY REQUIREMENTS, DEFLECTION, AND APPEARANCE.
 - 7.4. AIR CONTENT: CONFORM TO ACI 318 SECTION 19.3.3.1. MINIMUM STANDARDS FOR EXPOSURE CLASS ARE NOTED IN THE TABLE. IF FREEZING AND THAWING CLASS IS NOT NOTED, AIR CONTENT GIVEN IS THAT REQUIRED BY THE SER. TOLERANCE IS ±1-1/2%. AIR CONTENT SHALL BE MEASURED AT POINT OF PLACEMENT.
 - 7.5. AGGREGATES SHALL CONFORM TO ASTM C33.
 - 7.6. SLUMP: CONFORM TO ACI 301 SECTION 4.2.2.1. SLUMP SHALL BE DETERMINED AT POINT OF PLACEMENT.
 - 7.7. CHLORIDE CONTENT: CONFORM TO ACI 318 TABLE 19.3.2.1.
 - 7.8. ACI 318 SECTION 19.3.1.1 EXPOSURE CLASS SHALL BE ASSUMED TO BE F2 UNLESS DIFFERENT EXPOSURE CLASSES ARE LISTED IN THE TABLE OF MIX DESIGN REQUIREMENTS THAT MODIFY THESE BASE REQUIREMENTS.
 - 7.9. RECYCLED CARBON DIOXIDE (CO₂) IS PERMISSIBLE TO BE INJECTED INTO THE MIX AS AN INGREDIENT DURING MIXING, SUCH THAT CO₂ IS CHEMICALLY MINERALIZED INTO CONCRETE. CO₂ INJECTED INTO THE MIX MUST BE POST-INDUSTRIAL CO₂ SOURCED FROM AN EMITTER.
 - 7.10. SHRINKAGE LIMIT: CONCRETE USED IN ELEVATED SLABS AND BEAMS SHALL HAVE A SHRINKAGE LIMIT OF 0.045%. CONCRETE USED IN SLAB-ON-GRADE SHALL HAVE A SHRINKAGE LIMIT OF 0.040% WHEN EXPOSED OR USED IN INDUSTRIAL APPLICATIONS AND A SHRINKAGE LIMIT OF 0.045% FOR ALL OTHER CONDITIONS AND APPLICATIONS. SUBMIT LABORATORY TEST RESULTS AT 28 DAYS MEASURED IN ACCORDANCE WITH ASTM C157 TO SER FOR APPROVAL PRIOR TO CONSTRUCTION.
 - 7.11. MODULUS OF ELASTICITY SHALL BE A MINIMUM OF 57,000 * √f_c FOR ALL MIX DESIGNS.
 - 7.12. STRUCTURAL DESIGN IS BASED ON STRENGTH OF 2,500 PSI. THEREFORE, IT DOES NOT REQUIRE SPECIAL INSPECTION. THE 3,000/4,500 PSI COMPRESSIVE STRENGTH IS SPECIFIED FOR SERVICEABILITY.
8. **STRENGTH TESTING AND ACCEPTANCE:**
- 8.1. TESTING: OBTAIN SAMPLES AND CONDUCT TESTS IN ACCORDANCE WITH ACI 301 SECTION 17.3.3. ADDITIONAL SAMPLES MAY BE REQUIRED TO OBTAIN CONCRETE STRENGTHS AT ALTERNATE INTERVALS THAN SHOWN BELOW AND SHOULD BE STANDARD CURED PER ACI SECTION 26.5.3.2.
 - 8.1.1. CURE FOUR CYLINDERS FOR 28-DAY TEST AGE. TEST ONE CYLINDER AT 7 DAYS, TEST TWO CYLINDERS AT 28 DAYS, AND HOLD ONE CYLINDER IN RESERVE FOR USE AS THE ENGINEER DIRECTS. AFTER 56 DAYS, UNLESS NOTIFIED BY THE ENGINEER TO THE CONTRARY, THE RESERVE CYLINDER MAY BE DISCARDED WITHOUT BEING TESTED FOR SPECIMENS MEETING 28-DAY STRENGTH REQUIREMENTS.
 - 8.1.2. THE NUMBER OF CYLINDERS INDICATED ABOVE REFERENCE 6"x12" CYLINDERS. IF 4"x8" CYLINDERS ARE TO BE USED, ADDITIONAL CYLINDERS MUST BE CURED FOR TESTING OF THREE CYLINDERS AT TEST AGE PER THE TABLE OF MIX DESIGN REQUIREMENTS IN THESE GENERAL NOTES.
 - 8.2. ACCEPTANCE:
 - 8.2.1. STRENGTH IS SATISFACTORY WHEN THE AVERAGES OF ALL SETS OF THREE CONSECUTIVE TESTS EQUAL OR EXCEED THE SPECIFIED STRENGTH AND NO INDIVIDUAL TEST FALLS BELOW THE SPECIFIED STRENGTH BY MORE THAN 500 PSI.
9. **FORMWORK & RESHOREING:** CONFORM TO ACI 301 SECTION 2. REMOVAL OF FORMS SHALL CONFORM TO SECTION 2.3.2 EXCEPT THE STRENGTH INDICATED IN SECTION 2.3.2.4 SHALL BE 0.75 * f_c. RESHOREING SHALL CONFORM TO SECTION 2.3.3. IN ADDITION, COLD REINFORCED (NON-POST-TENSIONED) SLABS SHALL BE CONTINUOUSLY SHORED (BACKSHORED) FOR A MINIMUM OF 14 DAYS FOLLOWING PLACEMENT OF CONCRETE OR 7 DAYS AFTER CONCRETE HAS REACHED 0.75 * f_c, WHICHEVER IS LONGER. CONTRACTOR SHALL SUBMIT FORMWORK REMOVAL AND RESHORE INSTALLATION PROCEDURE AND DRAWINGS, AS APPLICABLE, SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE IN WHICH THE PROJECT IS LOCATED, FOR THE SER'S INFORMATION.
10. **MEASURING, MIXING, AND DELIVERY:** CONFORM TO ACI 301 SECTION 4.3.
11. **HANDLING, PLACING, CONSTRUCTING AND CURING:** CONFORM TO ACI 301 SECTION 5. IN ADDITION, HOT WEATHER CONCRETING SHALL CONFORM TO ACI 305R AND COLD WEATHER CONCRETING SHALL CONFORM TO ACI 306R.
12. **EMBEDDED ITEMS:** POSITION AND SECURE IN PLACE EXPANSION JOINT MATERIAL, ANCHORS, AND OTHER STRUCTURAL AND NON-STRUCTURAL EMBEDDED ITEMS BEFORE PLACING CONCRETE. CONTRACTOR SHALL REFER TO MECHANICAL, ELECTRICAL, PLUMBING, AND ARCHITECTURAL DRAWINGS AND COORDINATE OTHER EMBEDDED ITEMS.
13. **GROUT:** USE 7,000 PSI NON-SHRINK GROUT UNDER COLUMN BASE PLATES
14. **POST-INSTALLED ANCHORS TO CONCRETE:** ANCHOR LOCATION, TYPE, DIAMETER, AND EMBEDMENT SHALL BE AS INDICATED ON DRAWINGS. REFERENCE THE POST-INSTALLED ANCHORS SECTION OF THESE GENERAL NOTES FOR APPLICABLE POST-INSTALLED ANCHOR ADHESIVES. ANCHORS SHALL BE INSTALLED AND INSPECTED IN STRICT ACCORDANCE WITH THE APPLICABLE ICC-EVALUATION SERVICE REPORT (ESR). SPECIAL INSPECTION SHALL BE PER THE TESTS AND INSPECTIONS SECTION OF THESE GENERAL NOTES.

15. **SHRINKAGE:** CONVENTIONAL AND POST-TENSIONED CONCRETE SLABS WILL CONTINUE TO SHRINK AFTER INITIAL PLACEMENT AND STRESSING OF CONCRETE. CONTRACTOR AND SUBCONTRACTOR SHALL COORDINATE JOINTING AND INTERIOR MATERIAL FINISHES TO PROVIDE ADEQUATE TOLERANCE FOR EXPECTED STRUCTURAL FRAME SHRINKAGE AND SHALL INCLUDE, BUT NOT BE LIMITED TO, CURTAIN WALL, DRYVIT, STOREFRONT, SKYLIGHT, FLOOR FINISH, AND CEILING SUPPLIES. CONTACT ENGINEER FOR EXPECTED RANGE OF SHRINKAGE.

16. **CONCRETE PLACEMENT TOLERANCE:** CONFORM TO ACI 117 FOR CONCRETE PLACEMENT TOLERANCE.

05.05.19 POST-INSTALLED ANCHORS (INTO CONCRETE AND MASONRY)

1. **REFERENCE STANDARDS:** CONFORM TO:
- 1.1. IBC CHAPTER 19 "CONCRETE"
 - 1.2. ACI 318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"
 - 1.3. IBC CHAPTER 21 "MASONRY"
 - 1.4. TMS 402 "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES"
2. **POST-INSTALLED ANCHORS:** INSTALL ONLY WHERE SPECIFICALLY SHOWN IN THE DETAILS OR ALLOWED BY THE SER. ALL POST-INSTALLED ANCHOR TYPES AND LOCATIONS SHALL BE APPROVED BY THE SER AND SHALL HAVE A CURRENT ICC-ESR THAT PROVIDES RELEVANT DESIGN VALUES NECESSARY TO VALIDATE THE AVAILABLE STRENGTH EXCEEDS THE REQUIRED STRENGTH. SUBMIT CURRENT MANUFACTURER'S DATA AND ICC-ESR TO THE SER FOR APPROVAL REGARDLESS OF WHETHER OR NOT IT IS A PRE-APPROVED ANCHOR. ANCHORS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH ICC-ESR AND THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPI) IN CONJUNCTION WITH EDGE DISTANCE, SPACING, AND EMBEDMENT DEPTH AS INDICATED ON THE DRAWINGS. THE CONTRACTOR SHALL ARRANGE FOR A MANUFACTURER'S FIELD REPRESENTATIVE TO PROVIDE INSTALLATION TRAINING FOR ALL PRODUCTS TO BE USED PRIOR TO THE COMMENCEMENT OF WORK. ONLY TRAINED INSTALLERS SHALL PERFORM POST-INSTALLED ANCHOR INSTALLATION. A RECORD OF TRAINING SHALL BE KEPT ON SITE AND BE MADE AVAILABLE TO THE SER AS REQUESTED. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS SHALL BE PERFORMED BY A CERTIFIED ADHESIVE ANCHOR INSTALLER (AAI) AS CERTIFIED THROUGH AIC/CRSI OR AN APPROVED EQUIVALENT. PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO COMMENCEMENT OF INSTALLATION. NO REINFORCING BARS SHALL BE DAMAGED DURING INSTALLATION OF POST-INSTALLED ANCHORS. SPECIAL INSPECTION SHALL BE PER THE TESTS AND INSPECTIONS SECTION OF THESE GENERAL NOTES. ANCHOR TYPE, DIAMETER, AND EMBEDMENT SHALL BE AS INDICATED ON DRAWINGS.
- 2.1. ADHESIVE ANCHORS: THE FOLLOWING ADHESIVE-TYPE ANCHORING SYSTEMS HAVE BEEN USED IN THE DESIGN AND SHALL BE USED FOR ANCHORAGE TO CONCRETE OR MASONRY AS APPLICABLE AND IN ACCORDANCE WITH THE CORRESPONDING CURRENT ICC ESR REPORT. REFERENCE THE CORRESPONDING ICC-ESR FOR REQUIRED MINIMUM AGE OF CONCRETE, CONCRETE TEMPERATURE RANGE, MOISTURE CONDITION, LIGHT WEIGHT CONCRETE, AND HOLE DRILLING AND PREPARATION REQUIREMENTS. DRILLED-IN ANCHOR EMBEDMENT LENGTHS SHALL BE AS SHOWN ON DRAWINGS OR NOT LESS THAN SEVEN TIMES THE ANCHOR NOMINAL DIAMETER (7D). ADHESIVE ANCHORS ARE TO BE INSTALLED IN CONCRETE AGED A MINIMUM OF 21 DAYS UNLESS OTHERWISE SPECIFIED IN THE ICC-ESR.

HILTI "HIT-HY 200-R V3" - ICC-ESR 4868 FOR ANCHORAGE TO CONCRETE

HILTI "HIT-RE 500 V3" - ICC-ESR 3814 FOR ANCHORAGE TO CONCRETE
 - 2.2. SCREW ANCHORS: THE FOLLOWING SCREW TYPE ANCHOR IS PRE-APPROVED FOR ANCHORAGE TO CONCRETE OR MASONRY IN ACCORDANCE WITH CORRESPONDING CURRENT ICC-ESR:

SIMPSON "TITEN HD" - ICC-ESR 2713 FOR CARBON STEEL TO CONCRETE, ICC-ESR 1056 FOR MASONRY ONLY, IAPMO 493 FOR STAINLESS STEEL TO CONCRETE

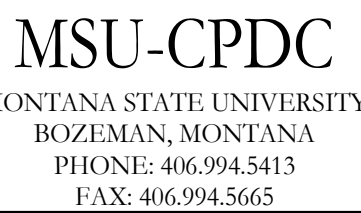
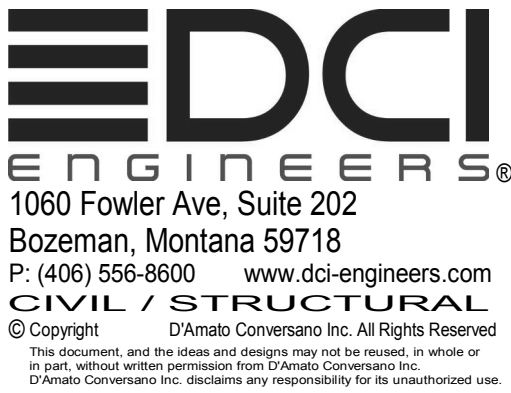
05.12.00 STRUCTURAL STEEL

1. **REFERENCE STANDARDS:** CONFORM TO:
- 1.1. IBC CHAPTER 22 "STEEL"
 - 1.2. ANSI/AISC 303 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES"
 - 1.3. AISC "MANUAL OF STEEL CONSTRUCTION"
 - 1.4. ANSI/AISC 360 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS"
 - 1.5. AWS D1.1 "STRUCTURAL WELDING CODE - STEEL"
 - 1.6. RCSC "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS"
2. **SUBMITTALS:** SUBMIT THE FOLLOWING DOCUMENTS TO THE SER FOR REVIEW:
- 2.1. SHOP DRAWINGS COMPLYING WITH AISC 360 SECTIONS M1 AND N3 AND AISC 303 SECTION 4.
 - 2.2. ERECTION DRAWINGS COMPLYING AISC 360 SECTIONS M1 AND N3 AND AISC 303 SECTION 4.
 - 2.3. WELD PROCEDURE SPECIFICATIONS (WPS'S) FOR SHOP AND FIELD WELDING.
 - 2.4. MANUFACTURER'S CERTIFICATES OF CONFORMANCE FOR ELECTRODES, FLUXES, AND GASES (WELDING CONSUMABLES).
3. **COPIES:** MAKE COPIES OF THE FOLLOWING DOCUMENTS AVAILABLE UPON REQUEST TO THE SER OR OWNER'S INSPECTION AGENCY IN ELECTRONIC OR PRINTED FORM PRIOR TO FABRICATION PER AISC 360 SECTION N.2 REQUIREMENTS:
- 3.1. FABRICATOR'S WRITTEN QUALITY CONTROL MANUAL THAT INCLUDES, AS A MINIMUM:
 - 3.1.1. MATERIAL CONTROL PROCEDURES
 - 3.1.2. INSPECTION PROCEDURES
 - 3.1.3. NON-CONFORMANCE PROCEDURES
 - 3.2. STEEL AND ANCHOR ROD SUPPLIERS' MATERIAL TEST REPORTS (MTRS) INDICATING COMPLIANCE WITH SPECIFICATIONS
 - 3.3. FASTENER MANUFACTURER'S CERTIFICATION DOCUMENTING CONFORMANCE WITH THE SPECIFICATION.
 - 3.4. FILLER METAL MANUFACTURER'S PRODUCT DATA FOR SMAW, FCAW, AND GMAW INDICATING:
 - 3.4.1. PRODUCT SPECIFICATION COMPLIANCE
 - 3.4.2. RECOMMENDED WELDING PARAMETERS
 - 3.4.3. RECOMMENDED STORAGE AND EXPOSURE REQUIREMENTS INCLUDING BAKING
 - 3.4.4. LIMITATIONS OF USE
 - 3.5. WELDED HEADED (SHEAR) STUD ANCHORS MANUFACTURER'S CERTIFICATION INDICATING THEY MEET SPECIFICATIONS.
 - 3.6. WPS'S FOR SHOP AND FIELD WELDING.
 - 3.7. MANUFACTURER'S CERTIFICATES OF CONFORMANCE FOR ELECTRODES, FLUXES, AND GASES (WELDING CONSUMABLES).
 - 3.8. PROCEDURE QUALIFICATION RECORDS (PQRs) FOR WPS'S THAT ARE NOT PREQUALIFIED IN ACCORDANCE WITH AWS.
 - 3.9. WELDING PERSONNEL PERFORMANCE QUALIFICATION RECORDS (WPQRs) AND CONTINUITY RECORDS CONFORMING TO AWS STANDARDS
4. **MATERIALS:** STRUCTURAL STEEL MATERIALS SHALL CONFORM TO MATERIALS AND REQUIREMENTS LISTED IN AISC 360 SECTION A3 INCLUDING, BUT NOT LIMITED TO:
- | | |
|------------------------------------|---|
| SHAPE | TYPE |
| ANGLE (L) SHAPES | ASTM A36, F _y = 36 KSI |
| PLATE (PL) | ASTM A36, F _y = 36 KSI |
| SQUARE/RECT HSS | ASTM A500, GRADE C, F _y = 50 KSI |
| HIGH-STRENGTH, HEAVY HEX BOLTS | ASTM F3125 GRADE A325/F1852, TYPE 1 OR 3, PLAIN |
| HEAVY HEX NUTS | ASTM A563, GRADE AND FINISH PER RCSC TABLE 2.1 |
| WASHERS (HARDENED FLAT OR BEVELED) | ASTM F436, GRADE AND FINISH PER RCSC TABLE 2.1 |
5. **STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS:**
- 5.1. ASTM F3125 GRADE A325-N BOLTS - "THREADS NOT EXCLUDED IN THE SHEAR PLANE."
 - 5.2. HIGH-STRENGTH BOLTED JOINTS HAVE BEEN DESIGNED AS "BEARING" CONNECTIONS.
 - 5.3. PROVIDE ASTM BOLT GRADE AND TYPE AS SPECIFIED IN THE MATERIALS SECTION OF THESE GENERAL NOTES.
 - 5.4. PROVIDE WASHERS OVER OUTER PLY OF SLOTTED HOLES AND OVERSIZE HOLES PER RCSC TABLE 6.1.
 - 5.5. PROVIDE NUT AND WASHER GRADES, TYPES, AND FINISHES CONFORMING TO RCSC TABLE 2.1.
 - 5.6. PROVIDE FASTENER ASSEMBLIES FROM A SINGLE SUPPLIER.
 - 5.7. JOINT TYPES SHALL BE:
 - 5.7.1. ST - "SNUG TIGHT," FOR TYPICAL BEAM END "SHEAR" CONNECTIONS UNLESS NOTED OTHERWISE.
 - 5.8. INSTALL BOLTS IN JOINTS IN ACCORDANCE WITH RCSC SECTION 8 AND TABLE 4.1.
 - 5.9. INSPECTION IS PER RCSC SECTION 9.
6. **ANCHORAGE TO CONCRETE:**

- 6.1. SHEAR STUDS ON STEEL BEAMS FOR COMPOSITE CONSTRUCTION: HEADED SHEAR STUDS WELDED (WHS) TO TOPS OF WIDE FLANGE BEAMS SHALL BE 3/4" DIAMETER WITH NOMINAL STUD LENGTHS AS INDICATED. UNLESS NOTED OTHERWISE, PROVIDE MINIMUM SHEAR STUD HEIGHT EQUAL TO THE METAL DECK DEPTH + 1-1/2" AND A MAXIMUM SHEAR STUD HEIGHT THAT ALLOWS FOR 1/2" OF CONCRETE COVER OVER THE STUD.
 - 6.2. COLUMN ANCHOR RODS AND BASE PLATES: ALL COLUMNS (VERTICAL MEMBER ASSEMBLIES WEIGHING OVER 300 LB) SHALL BE PROVIDED WITH A MINIMUM OF FOUR 3/4" DIAMETER ANCHOR RODS. COLUMN BASE PLATES SHALL BE AT LEAST 3/4" THICK UNLESS NOTED OTHERWISE. CAST-IN-PLACE ANCHOR RODS SHALL BE PROVIDED UNLESS OTHERWISE APPROVED BY THE ENGINEER. UNLESS NOTED OTHERWISE, EMBEDMENT OF CAST-IN-PLACE ANCHOR RODS SHALL BE 12 TIMES THE ANCHOR DIAMETER (12D).
 - 6.3. EMBEDDED STEEL PLATES FOR ANCHORAGE TO CONCRETE: PL EMBEDDED IN CONCRETE WITH WHS OR DOWEL BAR ANCHORS (DBA) SHALL BE OF THE SIZES AND LENGTHS AS INDICATED ON THE PLANS WITH MINIMUM 1/2" DIA x 8" LONG WHS, BUT PROVIDE NOT LESS THAN 3/4" INTERIOR COVER OR 1-1/2" EXTERIOR COVER TO THE OPPOSITE FACE OF CONCRETE UNLESS NOTED OTHERWISE.
7. **FABRICATION:**
- 7.1. CONFORM TO AISC 360 SECTION M2 AND AISC 303 SECTION 6.
 - 7.2. QUALITY CONTROL (QC) SHALL CONFORM TO:
 - 7.2.1. AISC 360 CHAPTER N
 - 7.2.2. AISC 303 SECTION 8
 - 7.2.3. FABRICATOR AND ERECTOR SHALL ESTABLISH AND MAINTAIN WRITTEN QC PROCEDURES PER AISC 360 SECTION N3.
 - 7.2.4. FABRICATOR SHALL PERFORM SELF-INSPECTIONS PER AISC 360 SECTION N5 TO ENSURE THAT THEIR WORK IS PERFORMED IN ACCORDANCE WITH CODE OF STANDARD PRACTICE, THE AISC SPECIFICATION, CONTRACT DOCUMENTS, AND THE APPLICABLE BUILDING CODE.
 - 7.2.5. QC INSPECTIONS MAY BE COORDINATED WITH QUALITY ASSURANCE (QA) INSPECTIONS PER AISC 360 SECTION N5 WHERE FABRICATORS QA PROCEDURES PROVIDE THE NECESSARY BASIS FOR MATERIAL CONTROL, INSPECTION, AND CONTROL OF THE WORKMANSHIP EXPECTED BY THE SPECIAL INSPECTOR.
8. **WELDING:**
- 8.1. WELDING SHALL CONFORM TO AWS D1.1 WITH PREQUALIFIED WELDING PROCESSES EXCEPT AS MODIFIED BY AISC 360 SECTION J2 WELDERS SHALL BE QUALIFIED IN ACCORDANCE WITH AWS D1.1 REQUIREMENTS.
 - 8.2. USE 70 KSI STRENGTH, LOW-HYDROGEN TYPE ELECTRODES (E7018) OR E71T AS APPROPRIATE FOR THE PROCESS SELECTED.
9. **ERECTION:**
- 9.1. CONFORM TO AISC 360 SECTION M4 AND AISC 303 SECTION 7.
 - 9.2. CONFORM TO AISC 360 CHAPTER N AND AISC 303 SECTION 8.
 - 9.2.1. THE ERECTOR SHALL MAINTAIN DETAILED ERECTION QC PROCEDURES THAT ENSURE THAT THE WORK IS PERFORMED IN ACCORDANCE WITH THESE REQUIREMENTS AND THE CONTRACT DOCUMENTS.
 - 9.3. STEEL WORK SHALL BE CARRIED UP TRUE AND PLUMB WITHIN THE LIMITS DEFINED IN AISC 303 SECTION 7.13.
 - 9.4. HIGH-STRENGTH BOLTING SHALL COMPLY WITH THE RCSC REQUIREMENTS INCLUDING RCSC SECTION 7.2, AS APPLICABLE AND AISC 360 CHAPTER J AND SECTIONS M2.5 AND N5.6.
 - 9.5. WELDING OF HEADED STUD ANCHORS SHALL BE IN ACCORDANCE WITH AWS D1.1 CHAPTER 7.
 - 9.6. PROVIDE HEADED (SHEAR) STUD ANCHORS WELDED THROUGH THE METAL DECK TO TOPS OF BEAMS DENOTED IN PLANS.
 - 9.7. THE CONTRACTOR SHALL PROVIDE TEMPORARY BRACING AND SAFETY PROTECTION REQUIRED BY AISC 360 SECTION M4.2 AND AISC 303 SECTIONS 7.10 AND 7.11.
10. **PROTECTIVE COATING REQUIREMENTS:**
- 10.1. SHOP PAINTING: CONFORM TO AISC 360 SECTION M3 AND AISC 303 SECTION 6.5 UNLESS OTHERWISE SPECIFIED BY THE PROJECT SPECIFICATIONS.
 - 10.2. INTERIOR STEEL:
 - 10.2.1. UNLESS NOTED OTHERWISE, DO NOT PAINT ANY OF THE STEEL SURFACES MEETING THE FOLLOWING CONDITIONS:
 - 10.2.1.1. CONCEALED BY THE INTERIOR BUILDING FINISHES
 - 10.2.1.2. FIREPROOFED
 - 10.2.1.3. EMBEDDED IN CONCRETE
 - 10.2.1.4. SPECIALLY PREPARED AS A "FAYING SURFACE" FOR TYPE-SC "SLIP-CRITICAL" CONNECTIONS INCLUDING BOLTED CONNECTIONS THAT FORM A PART OF THE SFRS GOVERNED BY AISC 341 UNLESS THE COATING CONFORMS TO REQUIREMENTS OF THE RCSC AND IS APPROVED BY THE ENGINEER.
 - 10.2.1.5. WELDED; IF AREA REQUIRES PAINTING, DO NOT PAINT UNTIL AFTER WELD INSPECTIONS AND NON-DESTRUCTIVE TESTING REQUIREMENT, IF ANY, ARE SATISFIED.
 - 10.2.2. INTERIOR STEEL EXPOSED TO VIEW SHALL BE PAINTED WITH ONE COAT OF SHOP PRIMER UNLESS OTHERWISE INDICATED IN THE PROJECT SPECIFICATIONS. FIELD TOUCH-UPS TO MATCH THE FINISH COAT OR AS OTHERWISE INDICATED IN THE PROJECT SPECIFICATIONS.
 - 10.3. EXTERIOR STEEL: EXPOSED EXTERIOR STEEL SHALL BE PROTECTED BY EITHER:
 - 10.3.1. PAINT WITH AN EXTERIOR MULTI-COAT SYSTEM AS PER THE PROJECT SPECIFICATIONS. FIELD TOUCH-UP PAINTING SHALL BE AS PER THE PROJECT SPECIFICATIONS.
 - 10.3.2. GALVANIZING: UNLESS PROTECTED WITH A PAINT SYSTEM, EXPOSED STEEL (OUTSIDE THE BUILDING ENVELOPE) SHALL BE HOT-DIPPED GALVANIZED WHERE NOTED ON THE PLANS OR OTHERWISE INDICATED BY THE FINISHES SPECIFIED BY THE ARCHITECT. APPLY FIELD TOUCH-UPS PER PROJECT SPECIFICATIONS.

05.31.00 METAL ROOF AND FLOOR DECK

1. **REFERENCE STANDARDS:** CONFORM TO:
- 1.1. SDI-DDM "DIAPHRAGM DESIGN MANUAL"
 - 1.2. ANSI/SDI NC "STANDARD FOR NON-COMPOSITE STEEL FLOOR DECK"
 - 1.3. ANSI/SDI RD "STANDARD FOR STEEL ROOF DECK"
 - 1.4. ANSI/SDI C "STANDARD FOR COMPOSITE STEEL FLOOR DECK-SLABS"
 - 1.5. AISI S100 "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS"
 - 1.6. AWS D1.3 "STRUCTURAL WELDING CODE - SHEET STEEL"
2. **SUBMITTALS:**
- 2.1. SUBMIT SHOP DRAWINGS TO THE ARCHITECT/ENGINEER FOR REVIEW. SHOP DRAWINGS SHALL INCLUDE MATERIAL TYPE, DESIGN LOADS, DIAPHRAGM CAPACITIES, SPAN LAYOUT, DECK ATTACHMENTS, METAL DECK EDGE FORM DESIGN AND CALCULATIONS, AND SHORING REQUIREMENTS. ALL OPENINGS SHALL BE INDICATED. ANY ALTERNATE DECK TYPES AND GAGES SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO FABRICATION AND SHALL INCLUDE A VALID ICC EVALUATION REPORT, CALCULATIONS, AND SHOP DRAWINGS (COMPONENT DESIGN DRAWINGS) STAMPED BY THE SSE.
3. **PRE-CONSTRUCTION MEETING:** THE CONTRACTOR SHALL ARRANGE A PRE-CONSTRUCTION MEETING FOR COMPOSITE STEEL CONSTRUCTION. THE CONTRACTOR, SER, SUBCONTRACTORS INVOLVED WITH THE STEEL ERECTION AND PLACEMENT OF CONCRETE ON COMPOSITE FLOORS WORK, AND REPRESENTATIVES OF THE SPECIAL INSPECTION AGENCY SHALL ATTEND THIS MEETING. AGENDA ITEMS INCLUDE BUT ARE NOT LIMITED TO:
- 3.1. POUR SEQUENCE: POURS SHOULD START OVER THE JOISTS AND WORK TOWARDS THE GIRDERS.
 - 3.2. CONSTRUCTION JOINT LOCATIONS AND DETAILS: DOWEL CONSTRUCTION JOINTS WITH #4 x 5'-0" @ 18" OC UNLESS NOTED OTHERWISE. PLAN, CONSTRUCTION JOINTS SHALL BE LOCATED TO FULLY LOAD COMPOSITE STEEL FRAMING PRIOR TO ACHIEVING COMPOSITE ACTION WITH THE CONCRETE.
 - 3.3. SCREED POINTS SHALL BE PLACED TO ACHIEVE A UNIFORM THICKNESS OF CONCRETE OVER THE DECK.
4. **MATERIAL:** ASTM A653 - SS DESIGNATION, GRADE 33 KSI. STEEL DECK AND ACCESSORIES SHALL BE GALVANIZED TO G60 MINIMUM IN ACCORDANCE WITH ASTM A653.
5. **TYPE:** DECK SHALL BE AS SHOWN ON THE STRUCTURAL DRAWINGS BASED ON THREE-SPAN, UNSHORED CONDITION. HEAVY GAUGE DECK OR SHORING MAY BE REQUIRED FOR CONDITIONS OTHER THAN THREE-SPAN BASED ON MANUFACTURER CAPACITIES, ACTUAL DECK LAYOUT, AND SPAN CONDITIONS. DEVIATIONS FROM DECK GAUGES SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED TO THE SER FOR APPROVAL. MAXIMUM DEAD LOAD DEFLECTION IS 3/4" OR L/180, WHICHEVER IS LESS.
- 5.1. VERCO - IAPMO ER-217
 - 5.2. VULCRAFT - ICC-ESR 1227
 - 5.3. EPIC - ICC-ESR 2047
6. **DIAPHRAGM CAPACITY:** DECK AND ATTACHMENTS SHALL BE CAPABLE OF RESISTING THE DIAPHRAGM SHEARS WHERE INDICATED ON THE DRAWINGS. SUBMIT ICC ESR AS PROOF OF COMPLIANCE.



DRAWN BY: **WCG**

REVIEWED BY: **JAL**

REV.	DESCRIPTION	DATE



PPA#17-0190

A/E#18-02-03

CONSULTANT #1805

SHEET TITLE
GENERAL NOTES

SHEET
S0.3
DATE
3/10/2025

100% CONSTRUCTION DOCUMENTS

11.1.3. #10 SCREW @ 12" OC OR PUNCH-LOK SIDE LAP CONNECTIONS.

DATE
3/10/2025

50.00.00 SPECIAL INSPECTIONS

1. THE FOLLOWING STATEMENT AND SCHEDULES OF INSPECTIONS ARE THOSE SPECIAL INSPECTIONS AND TESTS THAT SHALL BE PERFORMED FOR THIS PROJECT. SPECIAL INSPECTORS SHALL REFERENCE THESE PLANS AND IBC CHAPTER 17 FOR ALL SPECIAL INSPECTION REQUIREMENTS. THE OWNER SHALL RETAIN AN "APPROVED AGENCY" PER IBC SECTION 1703 TO PROVIDE SPECIAL INSPECTIONS FOR THIS PROJECT. SPECIAL INSPECTORS SHALL BE QUALIFIED PERSONS PER IBC SECTION 1704.2.4. SPECIAL INSPECTION REPORTS SHALL BE PROVIDED ON A WEEKLY BASIS. SUBMIT COPIES OF ALL INSPECTION REPORTS TO THE ARCHITECT/ENGINEER AND THE AUTHORITY HAVING JURISDICTION FOR REVIEW. IN ADDITION TO SPECIAL INSPECTION REPORTS AND TESTS, SUBMIT REPORTS AND CERTIFICATES NOTED IN IBC SECTION 1704.5 TO THE AUTHORITY HAVING JURISDICTION. FINAL SPECIAL INSPECTION REPORTS WILL BE REQUIRED BY EACH SPECIAL INSPECTION FIRM PER IBC SECTION 1704.2.4.

2. STATEMENT OF SPECIAL INSPECTIONS:

- 2.1. THIS STATEMENT OF SPECIAL INSPECTIONS HAS BEEN WRITTEN WITH THE UNDERSTANDING THAT THE BUILDING OFFICIAL WILL:
- 2.1.1. REVIEW AND APPROVE THE QUALIFICATIONS OF THE SPECIAL INSPECTOR.
- 2.1.2. MONITOR THE SPECIAL INSPECTION ACTIVITY ON THE PROJECT SITE TO ENSURE THAT SPECIAL INSPECTORS ARE QUALIFIED AND PERFORMING THEIR DUTY AS STATED WITHIN THIS STATEMENT.
- 2.1.3. REVIEW ALL SPECIAL INSPECTION REPORTS SUBMITTED TO THEM BY THE SPECIAL INSPECTOR.
- 2.1.4. PERFORM INSPECTIONS AS REQUIRED BY IBC SECTION 110.3.
- 2.2. THE FOLLOWING SPECIAL INSPECTIONS ARE APPLICABLE TO THIS PROJECT:

SPECIAL INSPECTIONS FOR STANDARD BUILDINGS (PER IBC SECTION 1705.1)	REQUIRED
SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE (PER IBC SECTION 1705.13)	NOT REQUIRED
TESTING FOR SEISMIC RESISTANCE (PER IBC SECTION 1705.14)	NOT REQUIRED
SPECIAL INSPECTIONS FOR WIND RESISTANCE (PER IBC SECTION 1705.12)	NOT REQUIRED

3. SPECIAL INSPECTION OF SHOP FABRICATED, GRAVITY LOAD-BEARING MEMBERS AND ASSEMBLIES:

- 3.1. SPECIAL INSPECTION OF SHOP FABRICATED, GRAVITY LOAD-BEARING MEMBERS AND ASSEMBLIES SHALL BE VERIFIED BY THE SPECIAL INSPECTOR AS STATED IN SECTION 1704.2.5.
- 3.2. INCLUDE THE FOLLOWING:
- 3.2.1. PRIOR TO THE START OF FABRICATION: SPECIAL INSPECTOR(S), REPRESENTING THE OWNER, SHALL VISIT THE FABRICATOR'S SHOP(S) WHERE THE WORK IS TO BE PERFORMED, AND VERIFY THAT THE FABRICATOR MAINTAINS DETAILED FABRICATION AND QUALITY CONTROL PROCEDURES THAT PROVIDE A BASIS FOR INSPECTION, CONTROL OF WORKMANSHIP, MATERIAL CONTROL, AND FABRICATOR'S ABILITY TO CONFORM TO APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS.
- 3.2.2. FABRICATOR SHALL HAVE AVAILABLE FOR INSPECTOR'S REVIEW DETAILED PROCEDURES FOR MATERIAL CONTROL THAT DEMONSTRATE THE FABRICATOR'S ABILITY TO MAINTAIN SUITABLE RECORDS AND PROCEDURES SUCH THAT, AT ANY TIME DURING THE FABRICATION PROCESS, THE MATERIAL SPECIFICATION, GRADE, AND APPLICABLE TEST REPORTS FOR PRIMARY LOAD-CARRYING MEMBERS ARE CAPABLE OF BEING DETERMINED.

4. STRUCTURAL STEEL: PER IBC SECTION 1705.2.1

- 4.1. A QUALIFIED SPECIAL INSPECTOR OF AN "APPROVED AGENCY" PROVIDING QUALITY ASSURANCE (QA) SPECIAL INSPECTIONS FOR THE PROJECT SHALL REVIEW AND CONFIRM THE FABRICATOR AND ERECTOR'S QUALITY CONTROL (QC) PROCEDURES FOR COMPLETENESS AND ADEQUACY RELATIVE TO AISC 360 CHAPTER N, AISC 303, AWS D1.1, AND 2021 IBC CODE REQUIREMENTS FOR THE FABRICATOR'S SCOPE OF WORK.
- 4.2. REQUIREMENTS:
- 4.2.1. QA AGENCY PROVIDING SPECIAL INSPECTIONS SHALL PROVIDE PERSONNEL MEETING THE MINIMUM QUALIFICATION REQUIREMENTS FOR INSPECTION AND NONDESTRUCTIVE TESTING (NDT) PER AISC 360 SECTION N4.
- 4.2.1.1. QC AGENCY SHALL SUBMIT QUALIFICATION DOCUMENTS PER AISC 341 SECTION J2 ON PROJECTS SUBJECT TO SPECIAL INSPECTIONS ON SEISMIC FORCE RESISTING SYSTEMS WITH R > 3.
- 4.2.1.2. NDT PERSONNEL SHALL BE QUALIFIED PER AISC 341 SECTION J4.
- 4.2.1.3. PROVIDE QC AND QA INSPECTIONS PER AISC 341 SECTION J5 THROUGH J10 AS APPLICABLE.
- 4.2.2. VERIFY FABRICATOR AND ERECTOR QC PROGRAM PER AISC 360 SECTION N2.
- 4.2.3. INSPECTION OF WELDS AND BOLTS BY BOTH QC AND QA PERSONNEL SHALL BE PER THE SCHEDULE OF SPECIAL INSPECTIONS IN THESE DRAWINGS. ALL PROVISIONS OF AWS D1-1 FOR STATICALLY LOADED STRUCTURES SHALL APPLY.
- 4.2.4. NONDESTRUCTIVE TESTING (NDT) OF WELDS:
- 4.2.4.1. NDT OF WELDED JOINTS PER AISC 360 SECTION N5
- 4.2.4.2. RISK CATEGORY FOR DETERMINATION OF EXTENT OF NDT PER AISC 360 N5.5B IS NOTED IN THE DESIGN CRITERIA AND LOADS SECTION OF THESE GENERAL REQUIREMENTS.
- 4.2.4.3. NDT PERFORMED SHALL BE DOCUMENTED AND REPORTS SHALL IDENTIFY THE TESTED WELD BY PIECE MARK AND LOCATION OF THE PIECE.
- 4.2.4.4. FOR FIELD WORK, THE NDT REPORT SHALL IDENTIFY THE TESTED WELD BY LOCATION IN THE STRUCTURE, PIECE MARK AND LOCATION OF THE PIECE.
- 4.2.5. ADDITIONAL INSPECTION TASKS PER AISC 360 SECTION N5.8.
- 4.2.6. INSPECTION FOR COMPOSITE CONSTRUCTION SHALL BE DONE PER AISC 360 SECTION N6.
- 4.3. POST-INSTALLED ANCHORS TO CONCRETE AND MASONRY: SHALL COMPLY WITH IBC SECTION 1703. INSPECTIONS SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS SET FORTH IN THE APPROVED ICC EVALUATION SERVICE REPORT (ICC-ESR) AND AS INDICATED BY THE DESIGN REQUIREMENTS SPECIFIED ON THE DRAWINGS. REFER TO THE POST-INSTALLED ANCHORS SECTION OF THE GENERAL NOTES FOR ANCHORS THAT ARE THE BASIS OF THE DESIGN. SPECIAL INSPECTOR SHALL VERIFY ANCHORS ARE AS SPECIFIED IN THE POST-INSTALLED ANCHORS SECTION OF THE GENERAL NOTES OR AS OTHERWISE SPECIFIED ON THE DRAWINGS. SUBSTITUTIONS REQUIRE APPROVAL BY THE SER AND REQUIRE SUBSTANTIATING CALCULATIONS AND A CURRENT 2021 IBC RECOGNIZED ICC-ESR. SPECIAL INSPECTOR SHALL DOCUMENT IN THEIR SPECIAL INSPECTION REPORT COMPLIANCE WITH EACH OF THE ELEMENTS REQUIRED WITHIN THE APPLICABLE ICC-ESR.
- 4.4. PREFABRICATED CONSTRUCTION: ALL PREFABRICATED CONSTRUCTION SHALL CONFORM TO IBC SECTION 1703.

5. SCHEDULES OF SPECIAL INSPECTIONS:

5.1. TABLE 1705.6 - REQUIRED SPECIAL INSPECTIONS AND TESTS OF SOILS

NO.	SPECIAL INSPECTION	CONTINUOUS	PERIODIC
1	VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY	-	X
2	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL	-	X
3	PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS	-	X
4	DURING FILL PLACEMENT, VERIFY USE OF PROPER MATERIALS AND PROCEDURES IN ACCORDANCE WITH THE PROVISIONS OF THE APPROVED GEOTECHNICAL REPORT. VERIFY DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL	X	-
5	PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY	-	X

5.2. TABLE 1705.3 - REQUIRED SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION

NO.	SPECIAL INSPECTION	CONTINUOUS	PERIODIC	REFERENCE STANDARD
1	INSPECT REINFORCEMENT, INCLUDING PRE-STRESSING TENDONS, AND VERIFY PLACEMENT	-	X	ACI 318 CHAPTER 20 ACI 318 SECTIONS 25.2, 25.3, & 26.6.1-26.6.3
2	REINFORCING BAR WELDING:	-	-	-
2a	VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706	-	X	AWS D1.4 ACI 318 SECTION 26.6.4
2b	INSPECT SINGLE PASS FILLET WELD MAXIMUM 5/16"	-	X	-
2c	INSPECT ALL OTHER WELDS	X	-	-
3	INSPECT ANCHORS CAST IN CONCRETE	-	X	ACI 318 SECTION 17.8.2
4	INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS:	-	-	-
4a	ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS	X	-	ACI 318 SECTION 17.8.2.4
4b	MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4a	-	X	ACI 318 SECTION 17.8.2
5	VERIFY USE OF REQUIRED DESIGN MIX	-	X	ACI 318 CHAPTER 19 ACI SECTIONS 26.4.3 & 26.4.4 IBC SECTION 1904.1, 1904.2 ASTM C172 & C31 ACI 318 SECTIONS 26.5 & 26.12
6	PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE	X	-	-
7	INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	X	-	ACI 318 SECTION 26.5
8	VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES	-	X	ACI 318 SECTIONS 26.5.3-26.5.5
14	INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED	-	X	ACI 318 SECTION 26.11.1.2(B)

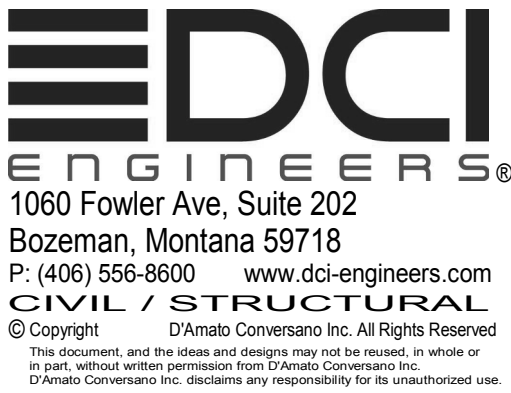
5.3. MINIMUM REQUIREMENTS FOR INSPECTIONS OF STRUCTURAL STEEL CONSTRUCTION

NO.	SPECIAL INSPECTION	QC	QA	REFERENCE STANDARD
INSPECTION TASKS PRIOR TO WELDING				
1	WELDER QUALIFICATION RECORDS AND CONTINUITY RECORDS	P	O	AISC 360 TABLE N5.4-1
2	WELDING PROCEDURE SPECIFICATIONS (WPS) AVAILABLE	P	P	AISC 360 TABLE N5.4-1
3	MANUFACTURING CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	P	P	AISC 360 TABLE N5.4-1
4	MATERIAL IDENTIFICATION (TYPE/GRADE)	O	O	AISC 360 TABLE N5.4-1
5	WELDER IDENTIFICATION SYSTEM	O	O	AISC 360 TABLE N5.4-1
6	FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY)	O	O	AISC 360 TABLE N5.4-1
JOINT PREPARATION				
DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL)				
CLEANLINESS (CONDITION OF STEEL SURFACES)				
TACKING (TACK WELDING QUALITY AND LOCATION)				
BACKING TYPE AND FIT (IF APPLICABLE)				
7	FIT-UP OF CJP GROOVE WELDS OF HSS T-, Y-, AND K-JOINTS WITHOUT BACKING (INCLUDING JOINT GEOMETRY)	P	O	AISC 360 TABLE N5.4-1
JOINT PREPARATION				
DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL)				
CLEANLINESS (CONDITION OF STEEL SURFACES)				
TACKING (TACK WELDING QUALITY AND LOCATION)				
8	CONFIGURATION AND FINISH OF ACCESS HOLES	O	O	AISC 360 TABLE N5.4-1
9	FIT-UP OF FILLET WELDS	O	O	AISC 360 TABLE N5.4-1
DIMENSIONS (ALIGNMENT, GAPS AT ROOT)				
CLEANLINESS (CONDITION OF STEEL SURFACES)				
TACKING (TACK WELDING QUALITY AND LOCATION)				
10	CHECK WELDING EQUIPMENT	O	-	AISC 360 TABLE N5.4-1
INSPECTION TASKS DURING WELDING				
1	USE OF QUALIFIED WELDERS	O	O	AISC 360 TABLE N5.4-2
2	CONTROL AND HANDLING OF WELDING CONSUMABLES	O	O	AISC 360 TABLE N5.4-2
PACKAGING				
EXPOSURE CONTROL				
3	NO WELDING OVER CRACKED TACK WELDS	O	O	AISC 360 TABLE N5.4-2
4	ENVIRONMENTAL CONDITIONS	O	O	AISC 360 TABLE N5.4-2
WIND SPEED WITHIN LIMITS				
PRECIPITATION AND TEMPERATURE				
5	WPS FOLLOWED	O	O	AISC 360 TABLE N5.4-2
SETTINGS ON WELDING EQUIPMENT				
TRAVEL SPEED				
SELECTED WELDING MATERIALS				
SHIELDING GAS TYPE/ FLOWRATE				
PREHEAT APPLIED				
INTERPASS TEMPERATURE MAINTAINED (MIN/MAX)				
PROPER POSITION (F, V, H, OH)				
6	WELDING TECHNIQUES	O	O	AISC 360 TABLE N5.4-2
INTERPASS AND FINAL CLEANING				
EACH PASS WITHIN PROFILE				
EACH PASS MEETS QUALITY REQUIREMENTS				
INSPECTION TASKS AFTER WELDING				
1	WELDS CLEANED	O	O	AISC 360 TABLE N5.4-3
2	SIZE, LENGTH, AND LOCATIONS OF WELDS	P	P	AISC 360 TABLE N5.4-3

3	WELDS MEET VISUAL ACCEPTANCE CRITERIA CRACK PROHIBITION WELD/BASE-METAL FUSION CRATER CROSS SECTION WELD PROFILES WELD SIZE UNDERCUT POROSITY	P	P	AISC 360 TABLE N5.4-3
4	ARC STRIKES	P	P	AISC 360 TABLE N5.4-3
5	K-AREA	P	P	AISC 360 TABLE N5.4-3
6	WELD ACCESS HOLES IN ROLLED HEAVY SHAPES AND BUILT-UP HEAVY SHAPES	P	P	AISC 360 TABLE N5.4-3
7	BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)	P	P	AISC 360 TABLE N5.4-3
8	REPAIR ACTIVITIES	P	P	AISC 360 TABLE N5.4-3
9	DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER	P	P	AISC 360 TABLE N5.4-3
10	NO PROHIBITED WELDS HAVE BEEN ADDED WITHOUT THE APPROVAL OF THE EOR	P	P	AISC 360 TABLE N5.4-3
INSPECTION TASKS PRIOR TO BOLTING				
1	MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS	O	P	AISC 360 TABLE N5.6-1
2	FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	O	O	AISC 360 TABLE N5.6-1
3	CORRECT FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE)	O	O	AISC 360 TABLE N5.6-1
4	CORRECT BOLTING PROCEDURE SELECTED FOR JOINT DETAIL	O	O	AISC 360 TABLE N5.6-1
5	CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FINISH SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS	O	O	AISC 360 TABLE N5.6-1
6	PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED	P	O	AISC 360 TABLE N5.6-1
7	PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS, AND OTHER FASTENERS COMPONENTS	O	O	AISC 360 TABLE N5.6-1
INSPECTION TASKS DURING BOLTING				
1	FASTENER ASSEMBLIES, OF SUITABLE CONDITION, PLACED IN ALL HOLES AND WASHERS ARE POSITIONED AS REQUIRED	O	O	AISC 360 TABLE N5.6-2
2	JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRE-TENSIONING OPERATION	O	O	AISC 360 TABLE N5.6-2
3	FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING	O	O	AISC 360 TABLE N5.6-2
4	FASTENERS ARE PRE-TENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES	O	O	AISC 360 TABLE N5.6-2
INSPECTION TASKS AFTER BOLTING				
1	NO PROHIBITED WELDS HAVE BEEN ADDED WITHOUT THE APPROVAL OF THE EOR	P	P	AISC 360 TABLE N5.6-3
INSPECTION OF STEEL ELEMENTS OF COMPOSITE CONSTRUCTION PRIOR TO CONCRETE PLACEMENT				
1	PLACEMENT AND INSTALLATION OF STEEL DECK	P	P	AISC 360 TABLE N6.1
2	PLACEMENT AND INSTALLATION OF STEEL HEADED STUD ANCHORS	P	P	AISC 360 TABLE N6.1
3	DOCUMENT ACCEPTANCE OR REJECTION OF STEEL ELEMENTS	P	P	AISC 360 TABLE N6.1

5.3.1. (O) - OBSERVE THESE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS

5.3.2. (P) - PERFORM THESE TASKS FOR EACH WELDED JOINT OR MEMBER, EACH BOLTED CONNECTION, OR EACH STEEL ELEMENT



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REVIEWED BY: JAL

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A/E#18-02-03

CONSULTANT #1805

SHEET TITLE
GENERAL NOTES

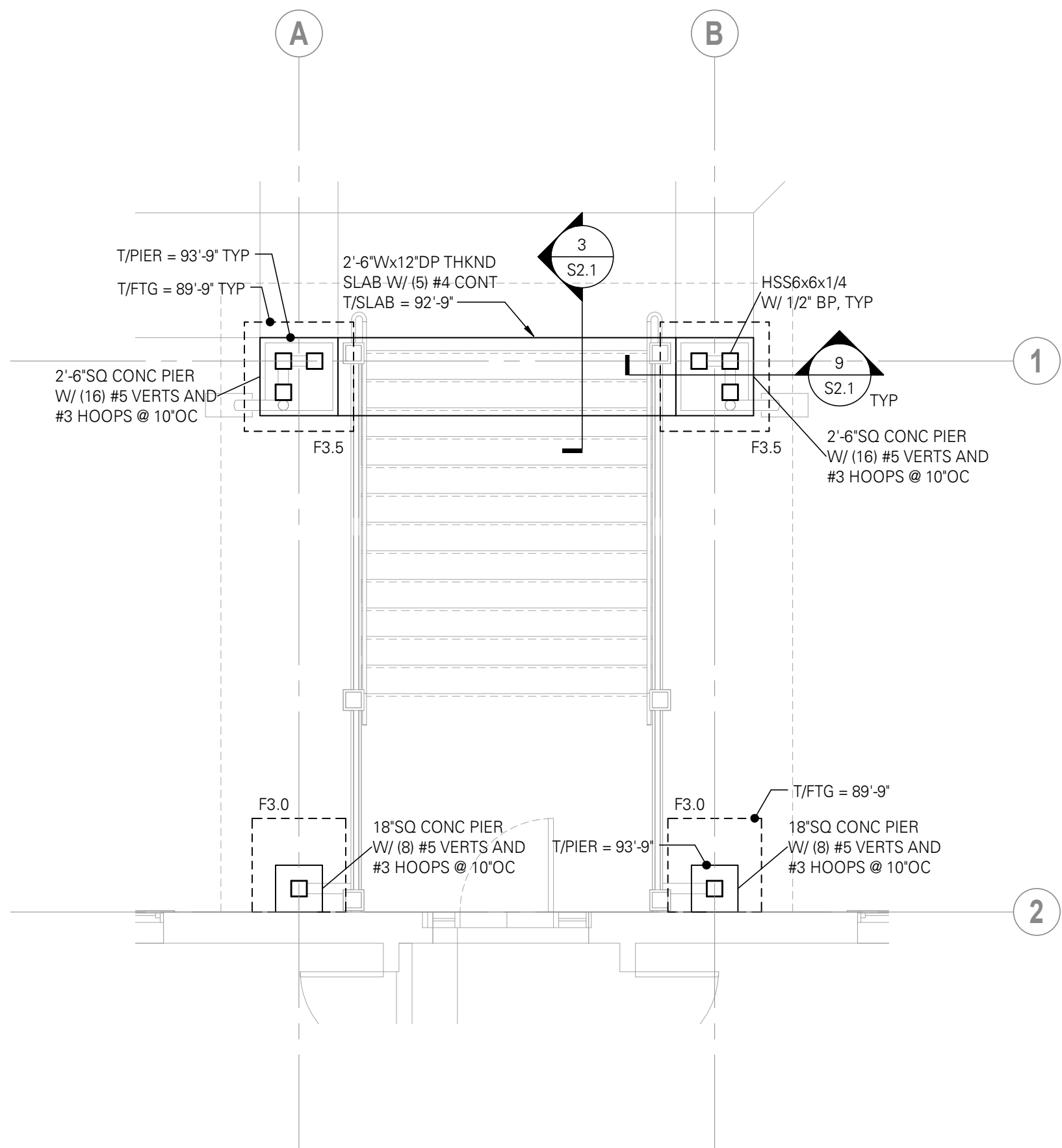
SHEET
S0.5

DATE
3/10/2025

FOUNDATION PLAN
NOTES:

- STRUCTURAL GENERAL NOTES, DESIGN CRITERIA, ABBREVIATIONS AND LEGEND PER S0.1 - S0.5.
- VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. ALL EXISTING DIMENSIONS SHALL BE FIELD VERIFIED.
- ALL FOOTINGS AND SLABS TO BEAR ON COMPETENT NATIVE SOIL AND/OR STRUCTURAL FILL. SUBGRADE PREPARATION, STRUCTURAL FILL, DRAINAGE SYSTEM, AND OTHER REQUIREMENTS PER GEOTECH REPORT AS NOTED IN THE STRUCTURAL GENERAL NOTES.
- TYPICAL DETAILS PER:
S2.1 FOUNDATION DETAILS

SPREAD FOOTING SCHEDULE					
MARK	SIZE			REINFORCING	COMMENTS
	LENGTH	WIDTH	DEPTH		
F3.0	3'-0"	3'-0"	1'-0"	(A) #4B EW	
F3.5	3'-6"	3'-6"	1'-0"	(B) #4B EW	

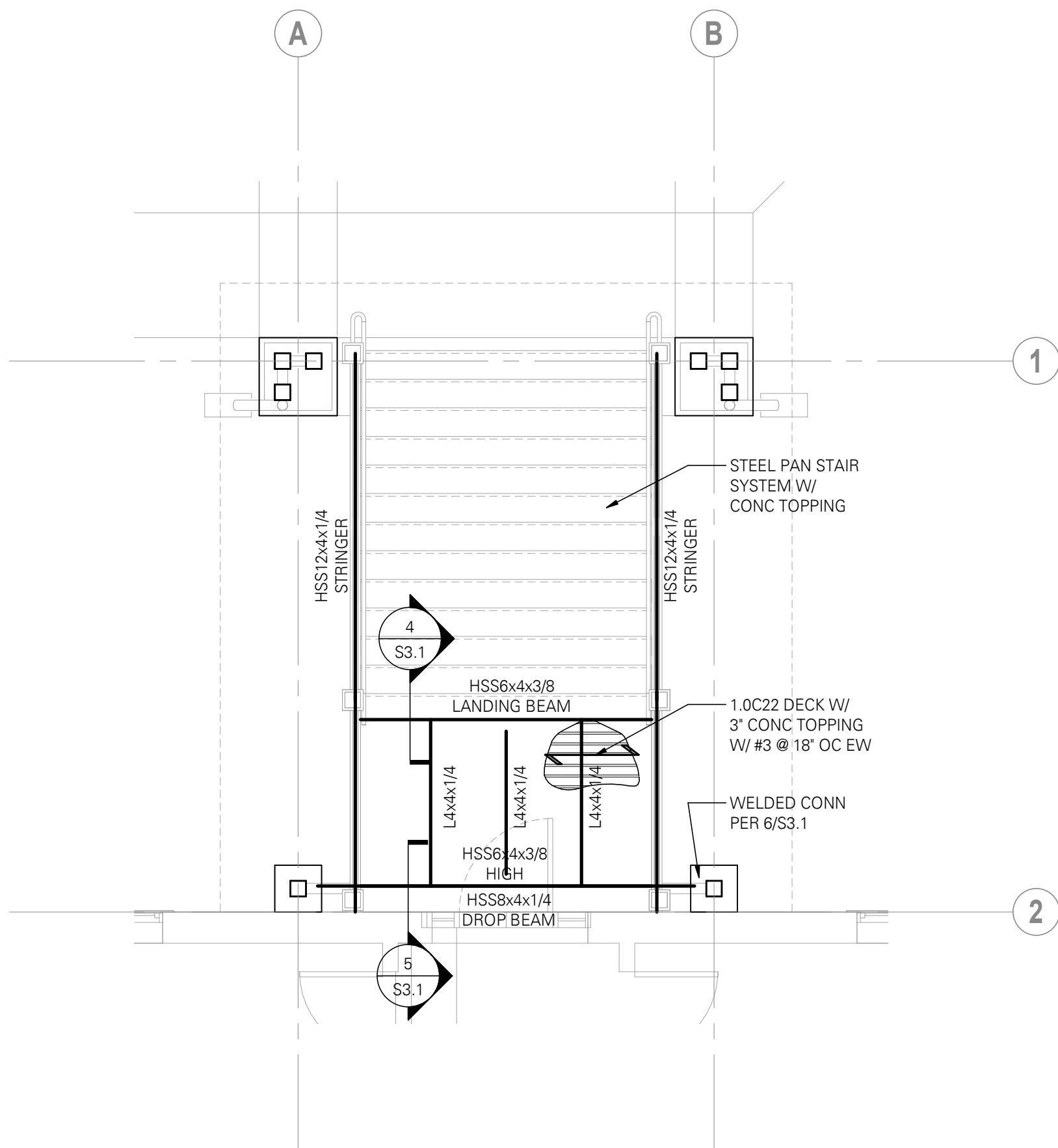


FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FLOOR FRAMING PLAN
NOTES:

- STRUCTURAL GENERAL NOTES, DESIGN CRITERIA, ABBREVIATIONS AND LEGEND PER S0.1 - S0.5.
- VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. ALL EXISTING DIMENSIONS SHALL BE FIELD VERIFIED.
- CONCRETE OVER METAL DECK PER PLAN AND STRUCTURAL GENERAL NOTES. METAL DECK SHALL BE 3-SPAN MINIMUM. PROVIDE REINFORCING AS SHOWN IN PLAN AND DETAILS.
- TYPICAL DETAILS PER:
S3.1 STEEL DETAILS

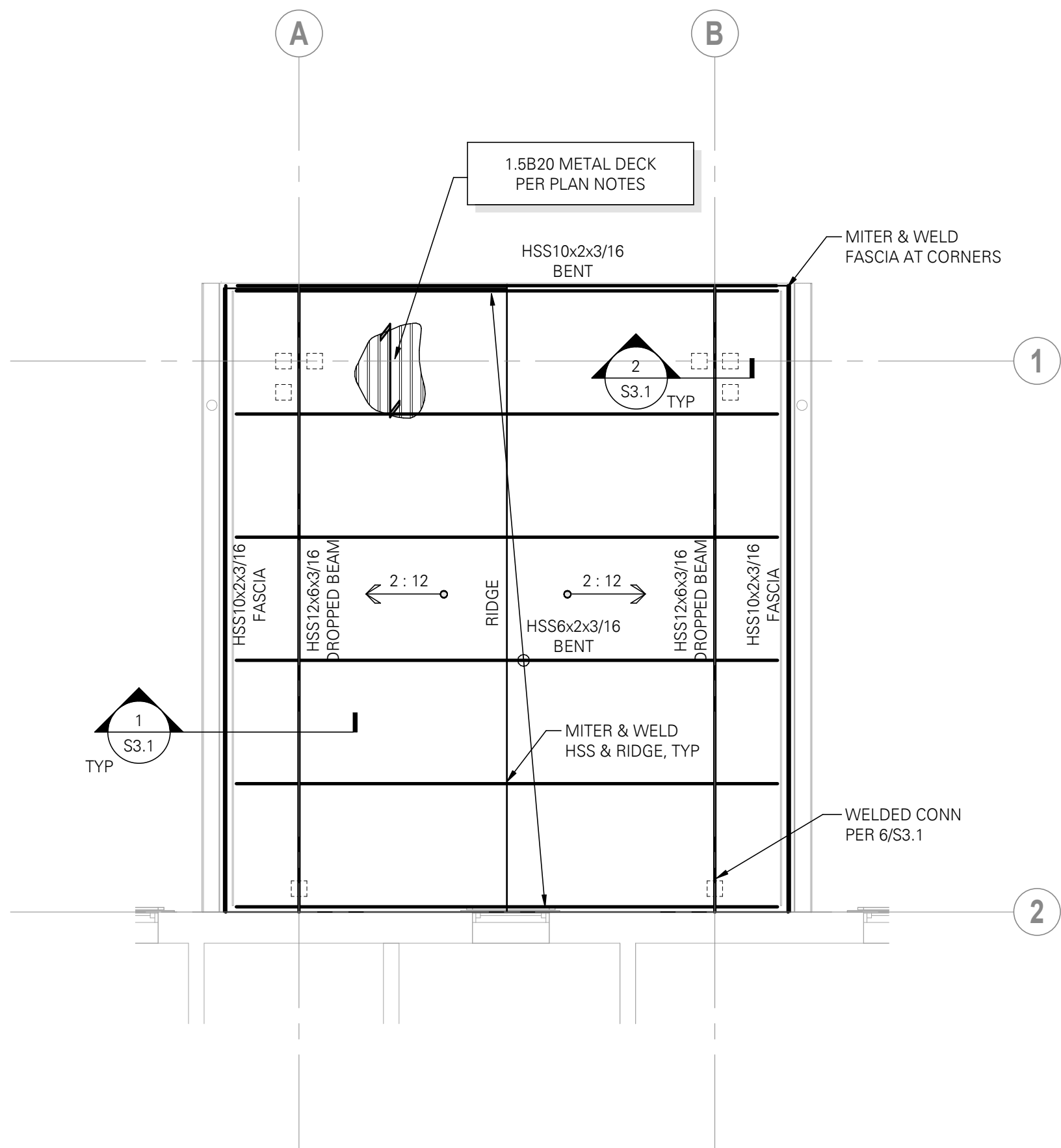


STAIR FRAMING PLAN

SCALE: 1/4" = 1'-0"

ROOF FRAMING PLAN NOTES:

- STRUCTURAL GENERAL NOTES, DESIGN CRITERIA, ABBREVIATIONS AND LEGEND PER S0.1 - S0.5.
- VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. ALL EXISTING DIMENSIONS SHALL BE FIELD VERIFIED.
- ALL DUCTS, CHASES AND PIPES SHALL BE PER MECHANICAL, PLUMBING, ELECTRICAL AND SPRINKLER DRAWINGS.
- T/STL=X'-X" INDICATES TOP OF STEEL (T/STL) ELEVATION (T/STL = B/DECK) UNO.
- METAL DECK PER PLAN AND STRUCTURAL GENERAL NOTES. DECK ATTACHMENT REQUIREMENTS PER STRUCTURAL GENERAL NOTES.
- CONTRACTOR IS RESPONSIBLE FOR ALL TEMPORARY SHORING.
- TYPICAL DETAILS PER:
S3.1 STEEL DETAILS



ROOF FRAMING PLAN

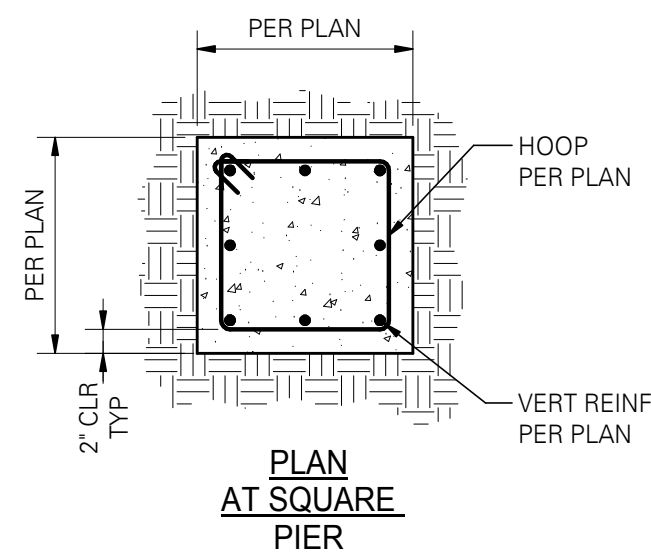
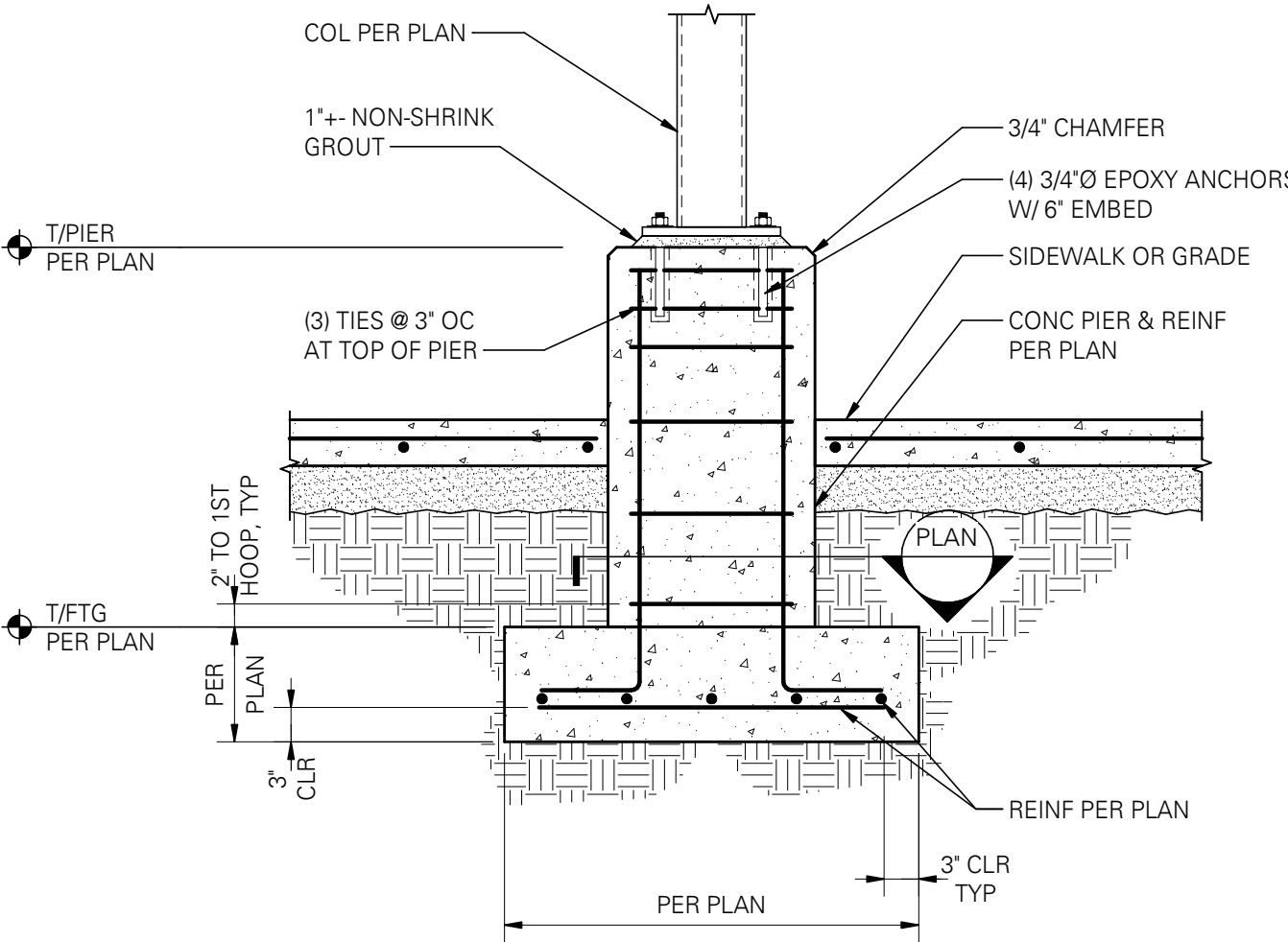
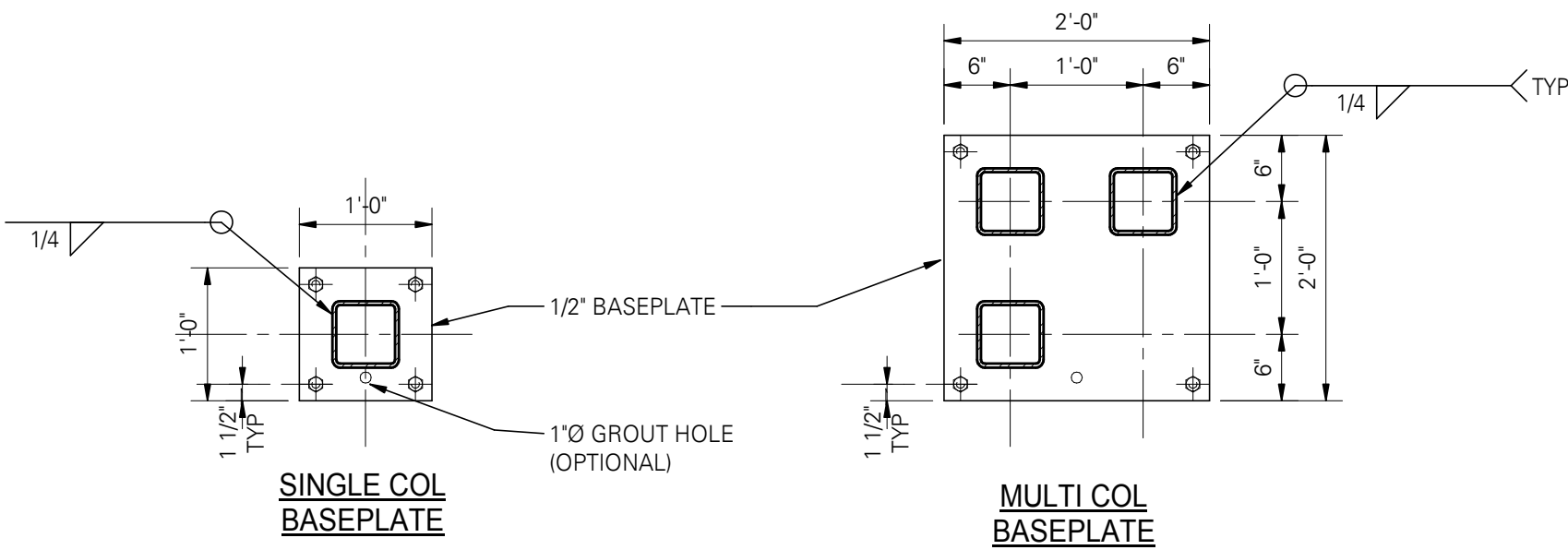
SCALE: 1/4" = 1'-0"

01403B						01403B					
GRADE 60 REINFORCING						GRADE 60 REINFORCING					
BAR SIZE	MISCELLANEOUS BARS		TOP BARS (see note #3)		HOOKED BARS	BAR SIZE	MISCELLANEOUS BARS		TOP BARS (see note #5)		HOOKED BARS
	Ld	Splice	Ld	Splice	Ldh		Ld	Splice	Ld	Splice	Ldh
f'c = 3000psi						f'c = 4000psi					
#3	17	22	22	28	9	#3	15	19	19	25	8
#4	22	29	29	38	11	#4	19	25	25	33	10
#5	28	36	36	47	14	#5	24	31	31	41	12
#6	33	43	43	56	17	#6	29	37	37	49	15
#7	48	63	63	81	20	#7	42	54	54	71	17
#8	55	72	72	93	22	#8	48	62	62	81	19
#9	62	81	81	105	25	#9	54	70	70	91	22
#10	70	91	91	118	28	#10	61	79	79	102	25

- NOTES:**
- ALL TABULATED VALUES ARE IN INCHES.
 - VALUES FOR UNCOATED REINFORCING AND NORMAL WEIGHT CONCRETE WITH CLEAR SPACING $> d_b$, CLEAR COVER $> d_b$ AND MINIMUM STIRRUPS OR TIES THROUGHOUT L_d OR CLEAR SPACING $> 2d_b$ AND CLEAR COVER $> d_b$.
 - DEVELOP ALL REINFORCING IN STRUCTURAL SLABS WITH MINIMUM DEVELOPMENT LENGTH L_d .
 - L_{dh} = DEVELOPMENT LENGTH OF BAR WITH STANDARD HOOK.
 - TOP BAR = HORIZONTAL BAR WITH MORE THAN 12" OF FRESH CONCRETE BELOW OR AS NOTED ON DOCUMENTS AS 'TOP BAR'.
 - LAP SPLICE OF DIFFERENT SIZED BARS TO BE THE LARGER OF L_d OF THE LARGER BAR OR SPLICE LENGTH OF THE SMALLER BAR.
 - LAP SPLICE OF DIFFERENT GRADES OF REINFORCING TO BE THE LARGER OF L_d OF THE HIGHER GRADE BAR OR SPLICE LENGTH OF THE LOWER GRADE BAR.
 - FOR INTERMEDIATE CONCRETE STRENGTHS, USE TABLE FOR CLOSEST LOWER VALUE OF f'_c .

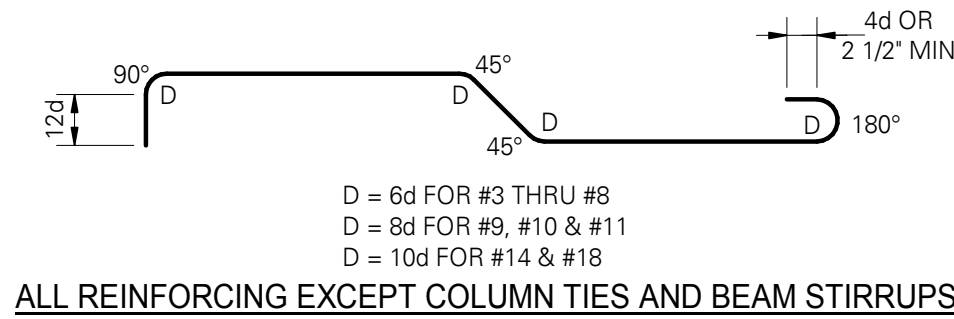
1 LAP SPLICE AND DEVELOPMENT LENGTH

SCALE: 3/4" = 1'-0" (01403B)

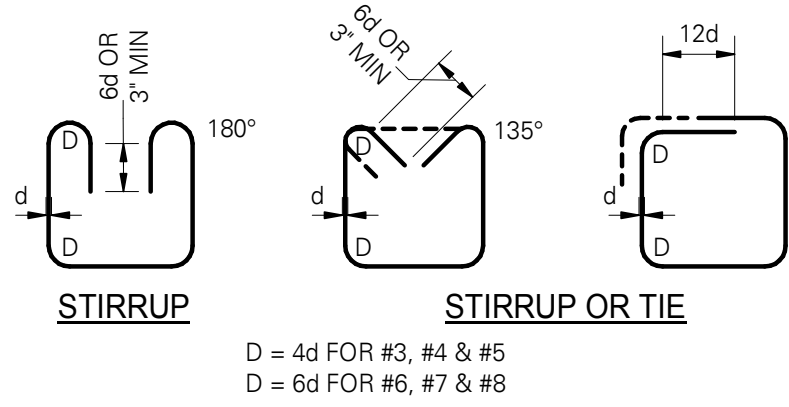


9 SPREAD FOOTING AT STEEL COLUMN (BELOW GRADE)

SCALE: 3/4" = 1'-0" (03007)



ALL REINFORCING EXCEPT COLUMN TIES AND BEAM STIRRUPS



BEAM OR COLUMN CROSSTIES BEAM STIRRUPS AND COLUMN TIES

d = BAR DIAMETER, D = BEND DIAMETER

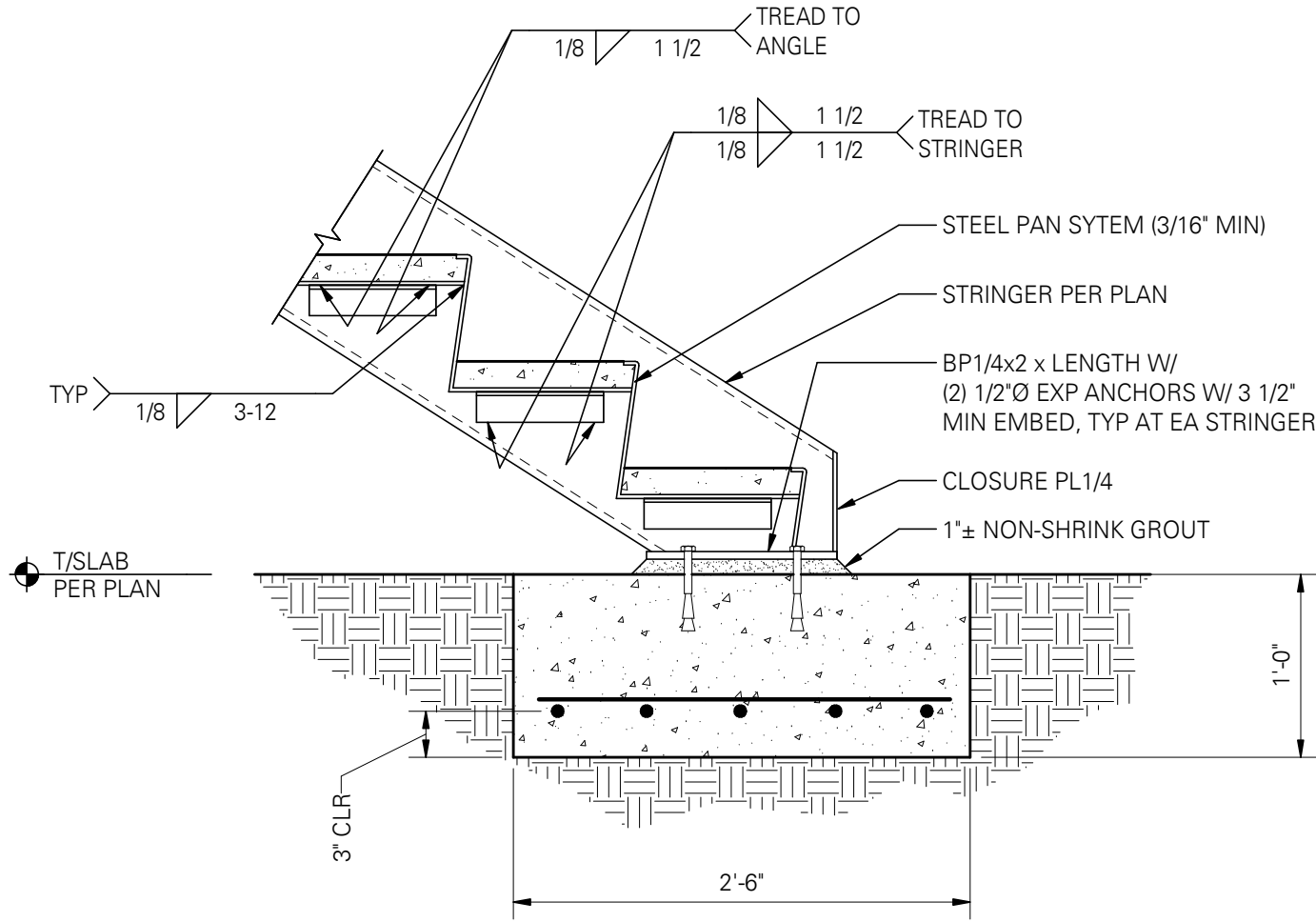
NOTE:
TIES AND CROSSTIES FOR SHEAR WALL
BOUNDARY ELEMENTS SHALL BE
DETAILED AS COLUMN TIES/CROSSTIES.

2 STANDARD HOOKS AND BENDS

SCALE: 3/4" = 1'-0" (03400)

3 STRINGER BASE DETAIL AT SLAB ON GRADE

SCALE: 1" = 1'-0" (05608)



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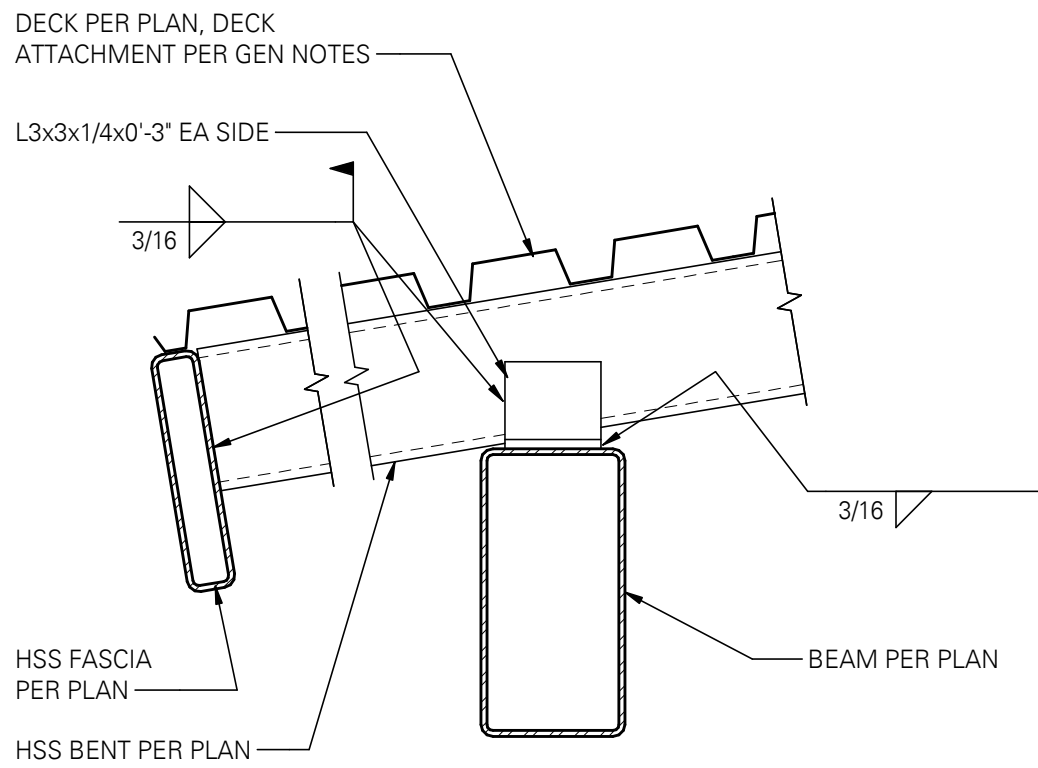


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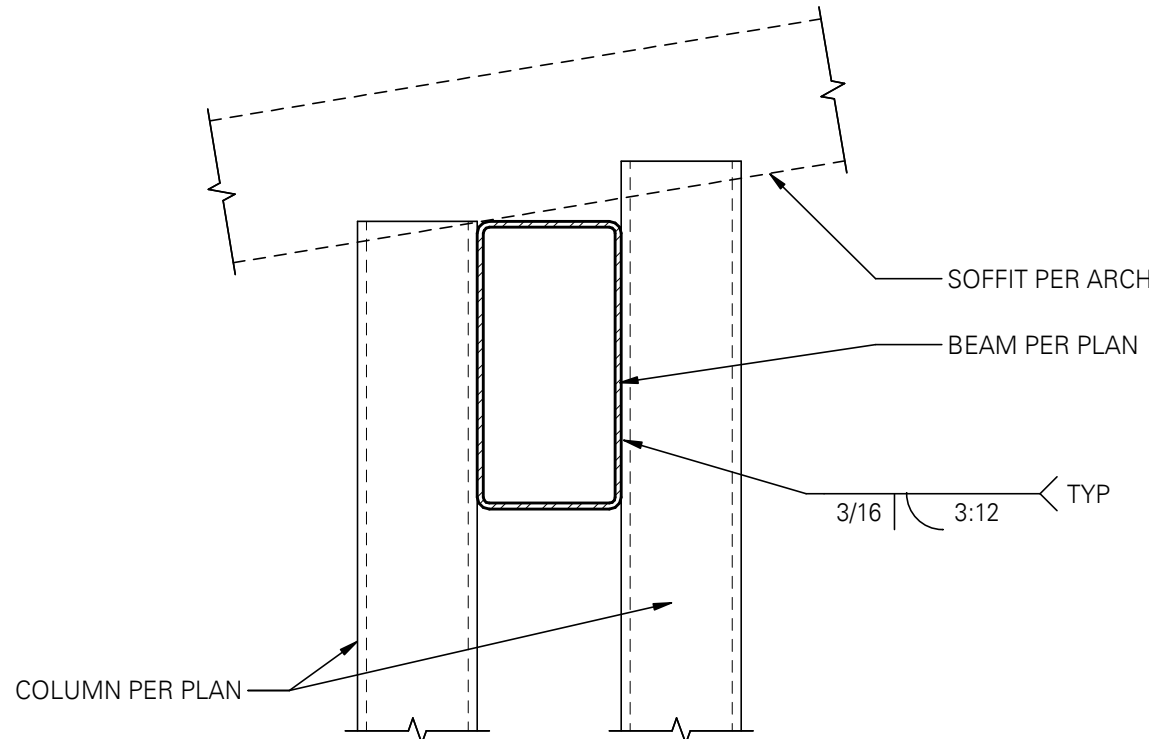
SHEET TITLE
CONCRETE
DETAILS

SHEET
S2.1

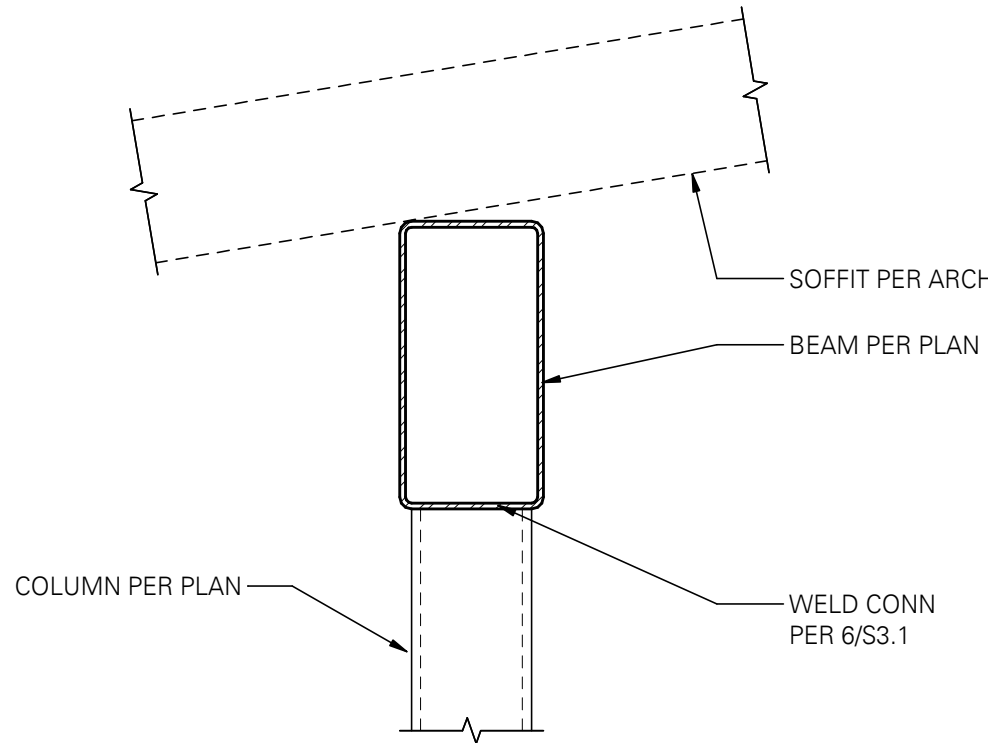
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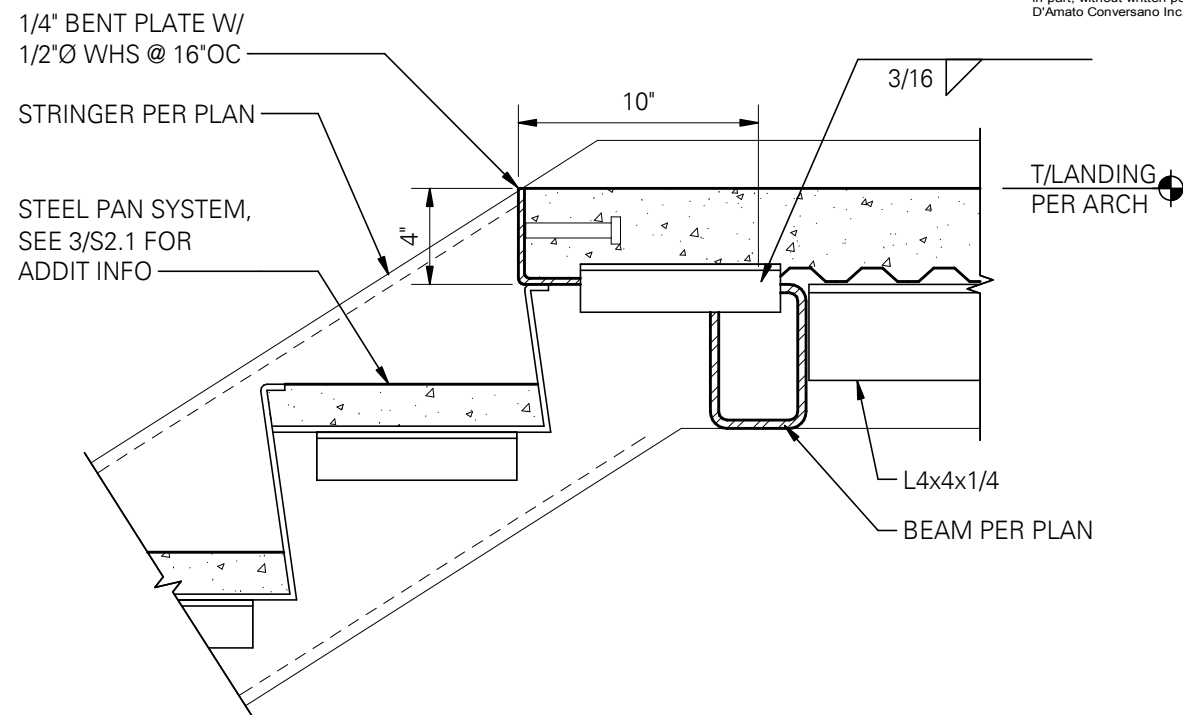
1 TYPICAL HSS BENT TO BEAM CONNECTION
SCALE: 1 1/2" = 1'-0"



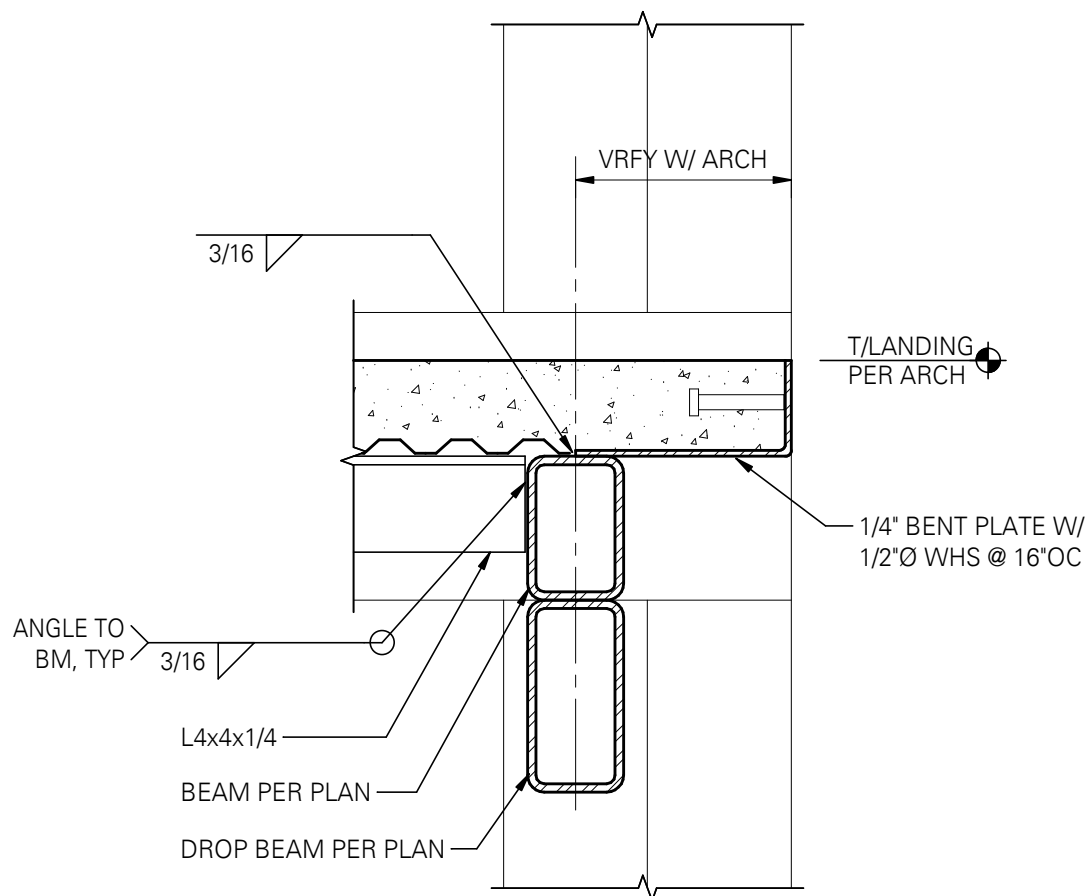
2 BEAM TO COLUMN CONNECTION AT DOUBLE COLUMN
SCALE: 1 1/2" = 1'-0"



3 BEAM TO COLUMN AT SINGLE COLUMN
SCALE: 1 1/2" = 1'-0"



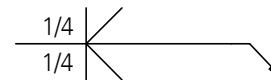
4 LANDING FRAMING AT TREADS
SCALE: 1 1/2" = 1'-0"



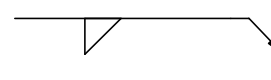
5 LANDING FRAMING AT ENTRY
SCALE: 1 1/2" = 1'-0"

NOTES:

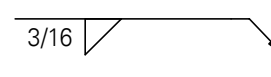
A. WHEN COLUMN WIDTH EQUALS BRACE/BEAM WIDTH USE FULL PENETRATION BEVEL WELD:



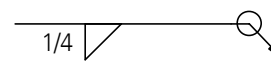
B. WHEN COLUMN WIDTH LESS THAN BEAM WIDTH USE FILLET WELD:



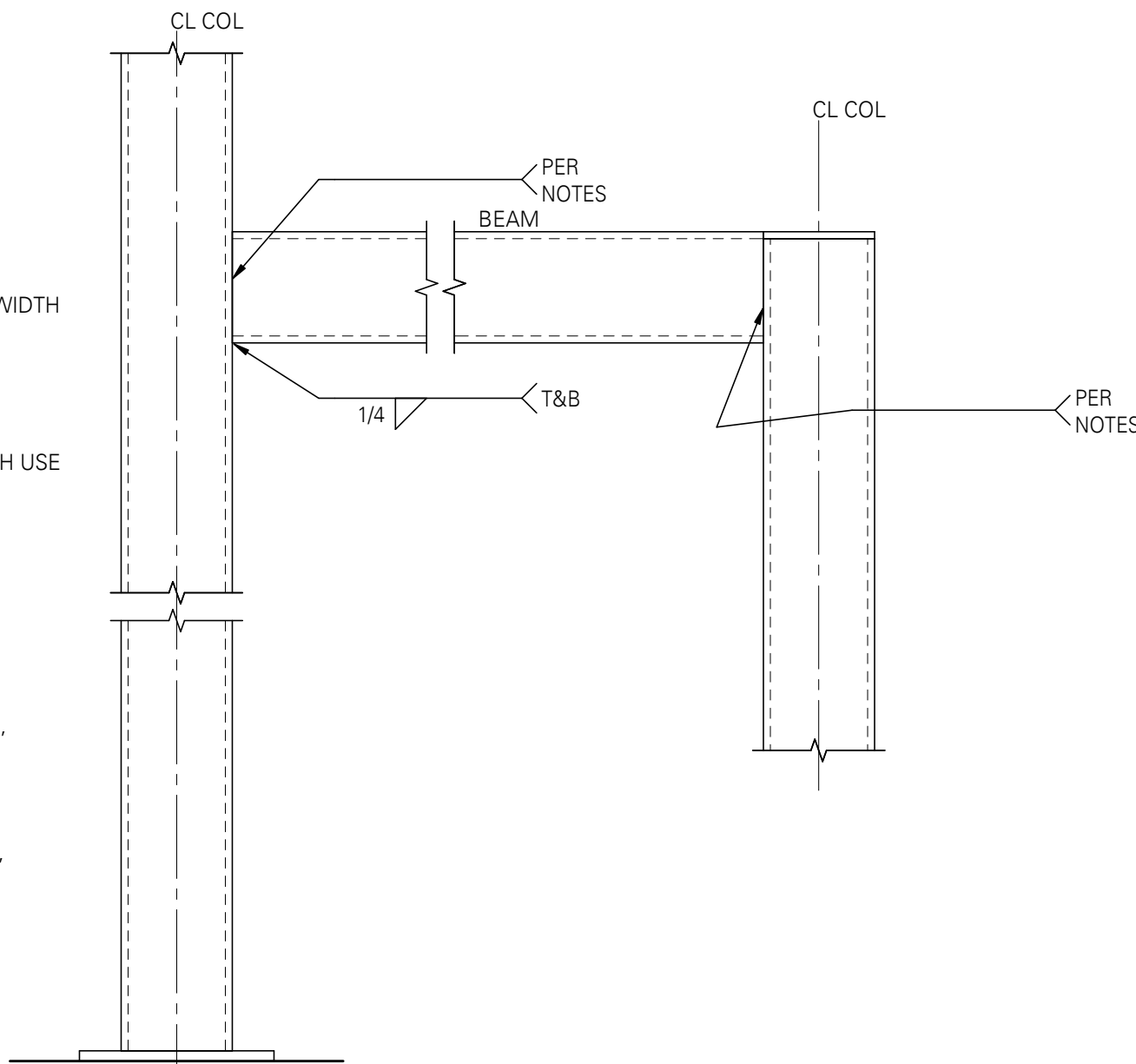
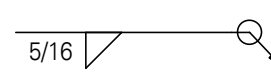
C. FOR WALL THICKNESS (T), WHERE T=1/4", USE FILLET WELD:



D. FOR WALL THICKNESS (T), WHERE T=1/4"<T<3/8", USE FILLET WELD:

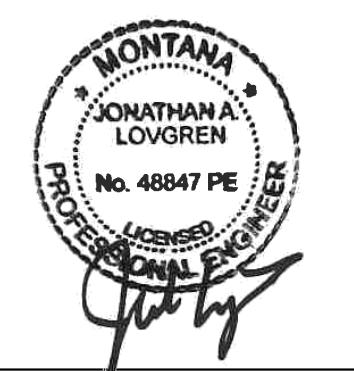


E. FOR WALL THICKNESS (T), WHERE T=3/8"<T<1/2", USE FILLET WELD:



6 TYPICAL HSS WELDED CONNECTIONS
SCALE: 1' = 1'-0" (05801)

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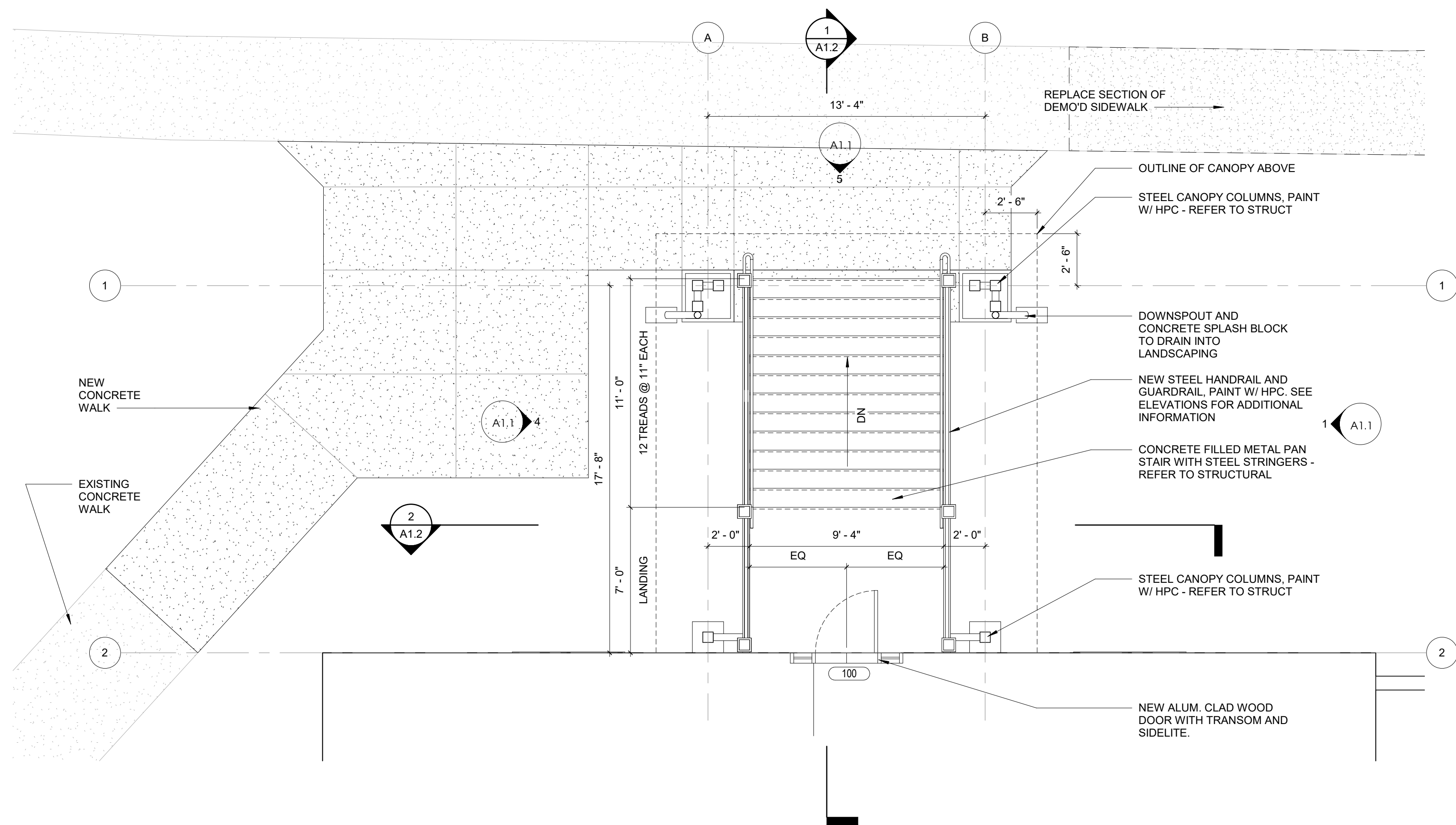


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SHEET TITLE
STEEL DETAILS

SHEET
S3.1

DATE
3/10/2025



1
A1.0
1/4" = 1'-0"

ENLARGED CANOPY PLAN



2
A1.0
1/4" = 1'-0"

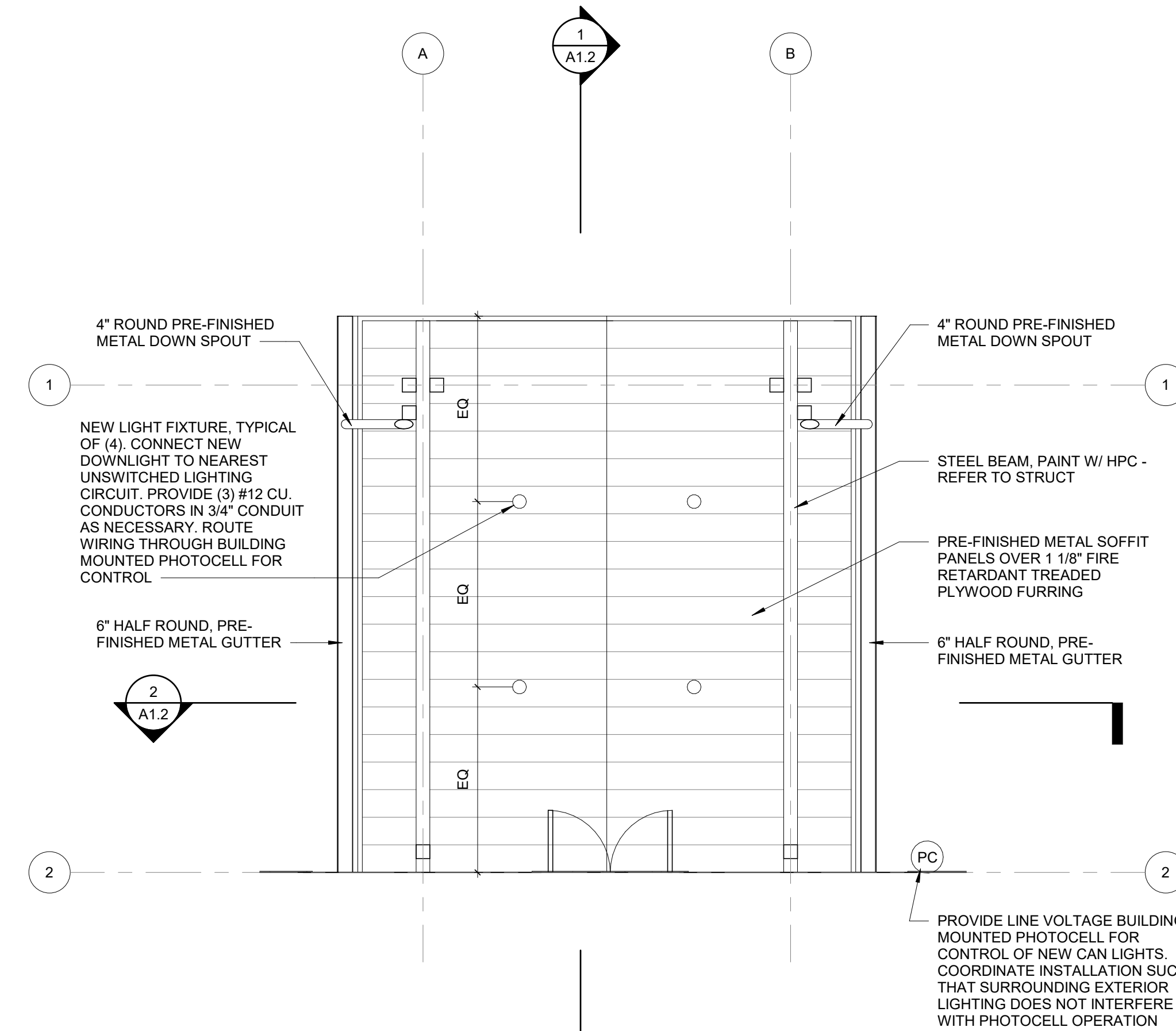
ENLARGED CANOPY ROOF PLAN



DOWNLIGHT BASIS-OF-DESIGN:

LITHONIA LIGHTING; WF6, 6" WAFER-THIN LED DOWNLIGHT

WF6	Lamp	CCT/W/Lumens ¹	CRI	Finish
Series				
WF6 6" wafer-thin LED downlight	LED LED	27K30K35K 2700K/14W/1040L 3000K/14W/1150L 3500K/14W/1110L 30K40K50K 3000K/14W/1090L 4000K/14W/1190L 5000K/14W/1120L	90CRI 90CRI	MW Matte White MB Matte Black BN Brush Nickel ORB Oil Rubbed Bronze



3
A1.0
1/4" = 1'-0"

ENLARGED CANOPY RCP



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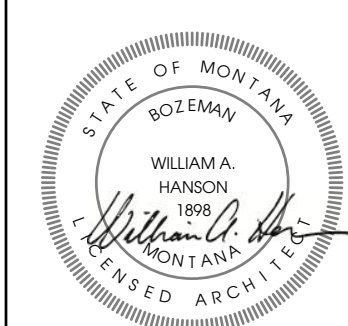
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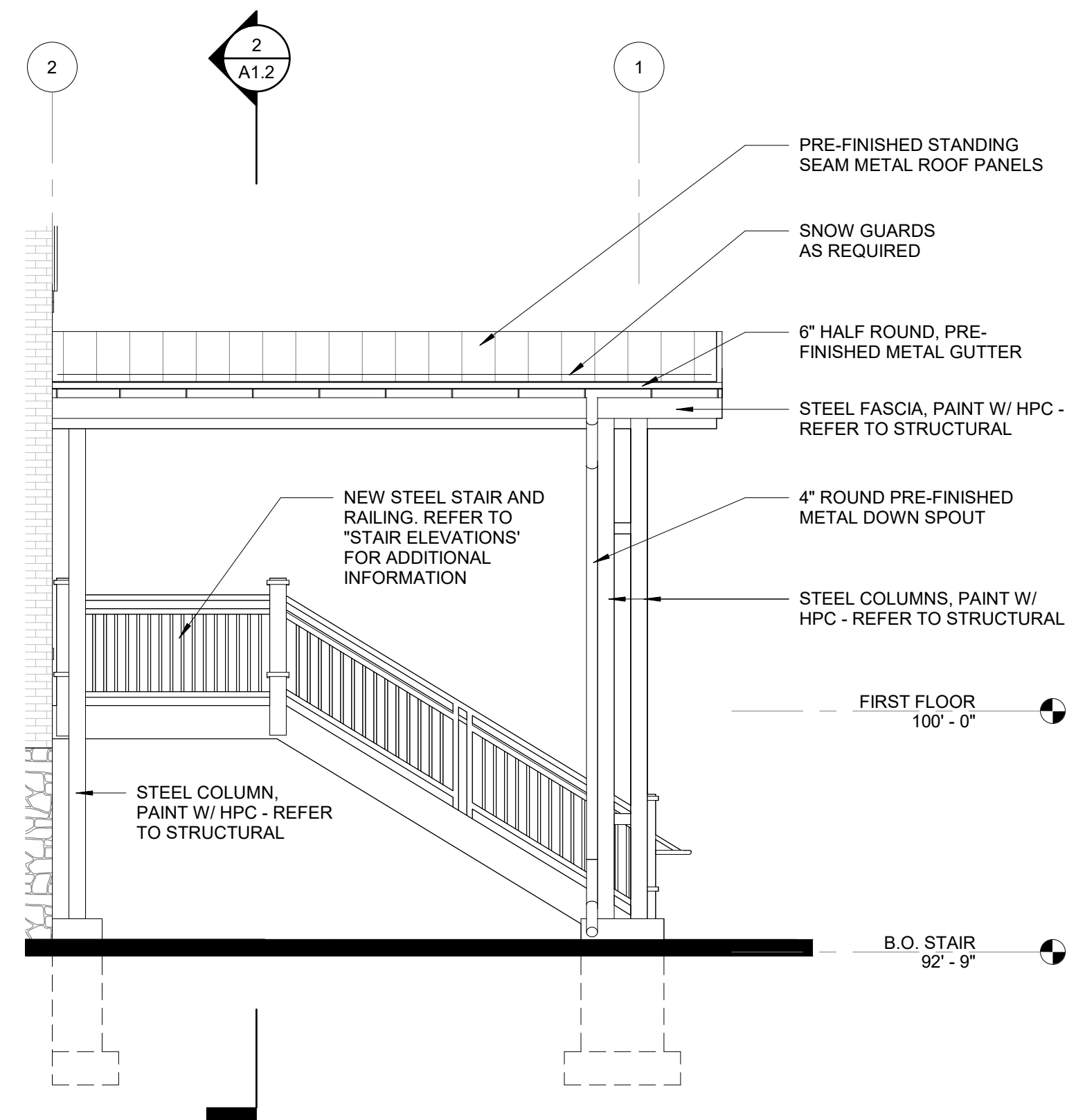


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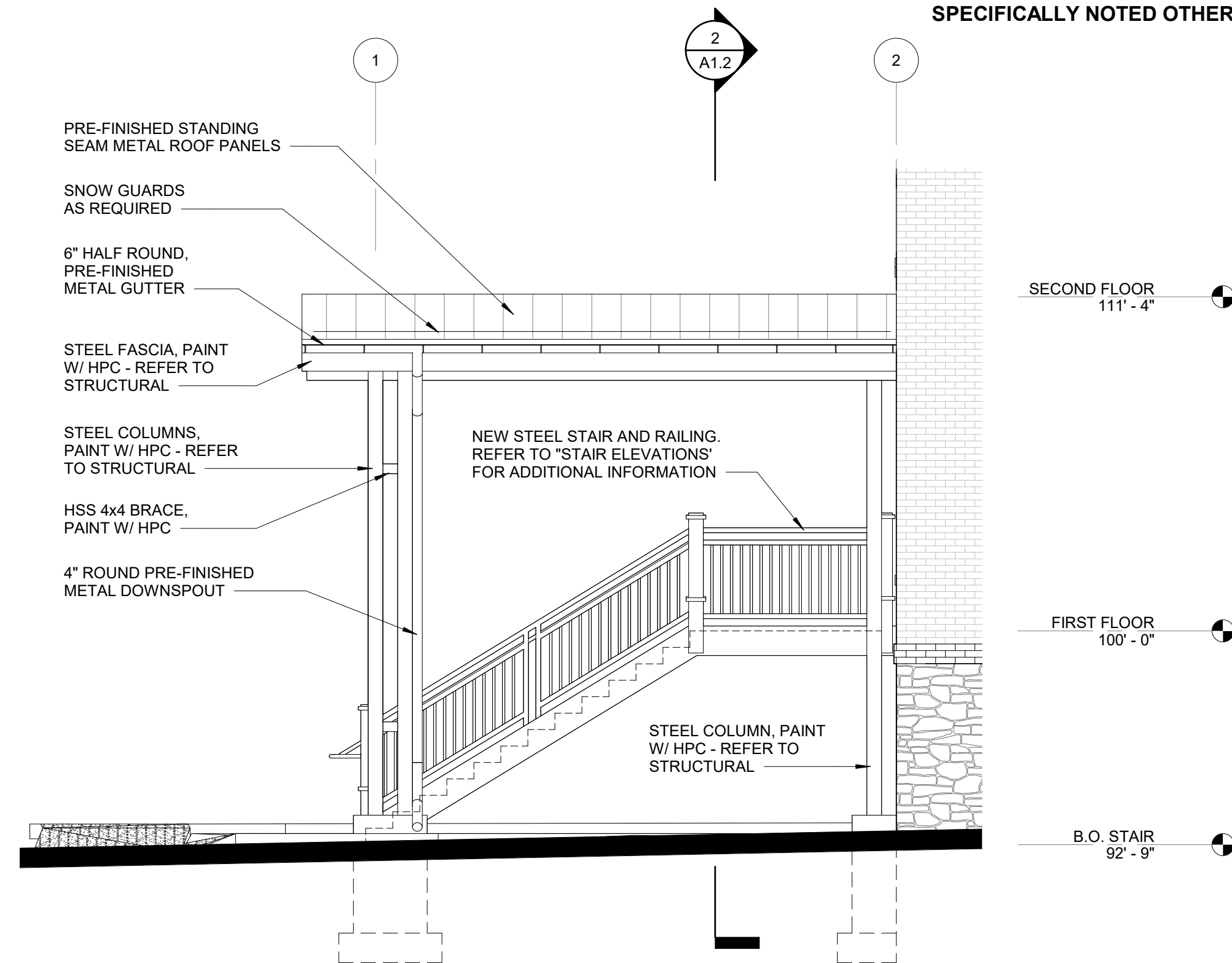
SHEET TITLE
ENTRY CANOPY
PLAN

SHEET
A1.0

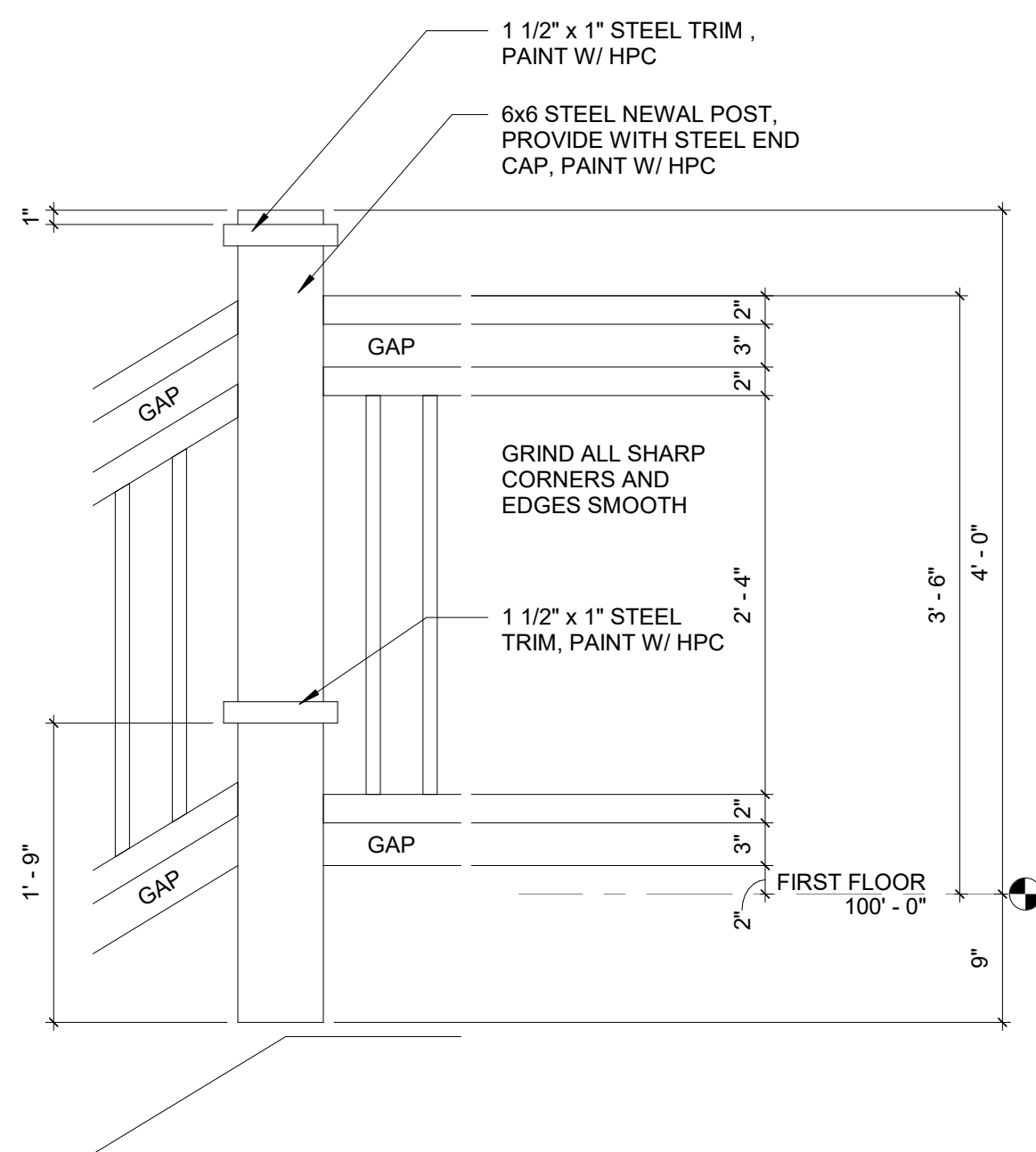
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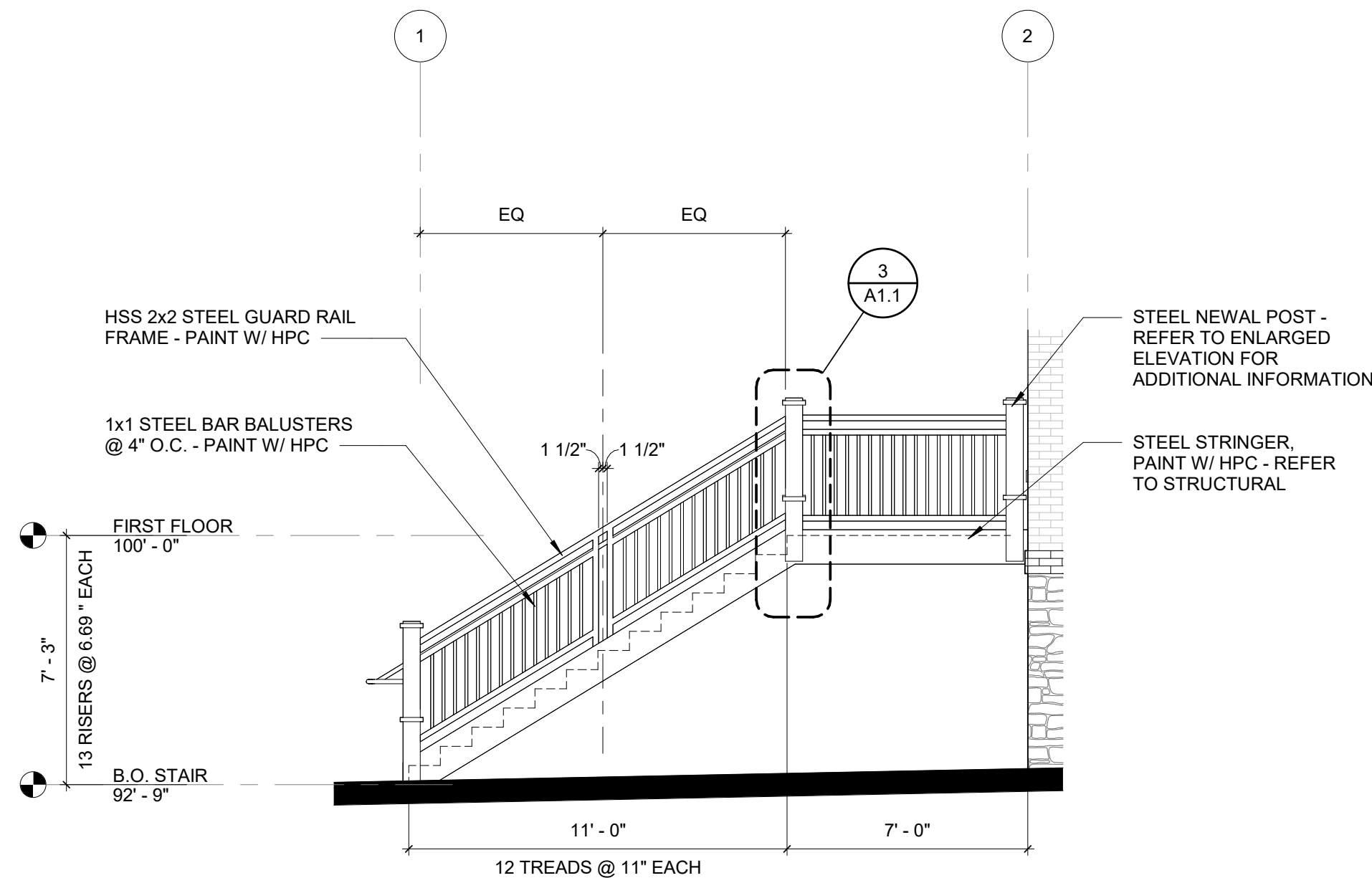
1
A1.1
01 NEW ENTRY CANOPY - EAST ELEVATION
1/4" = 1'-0"



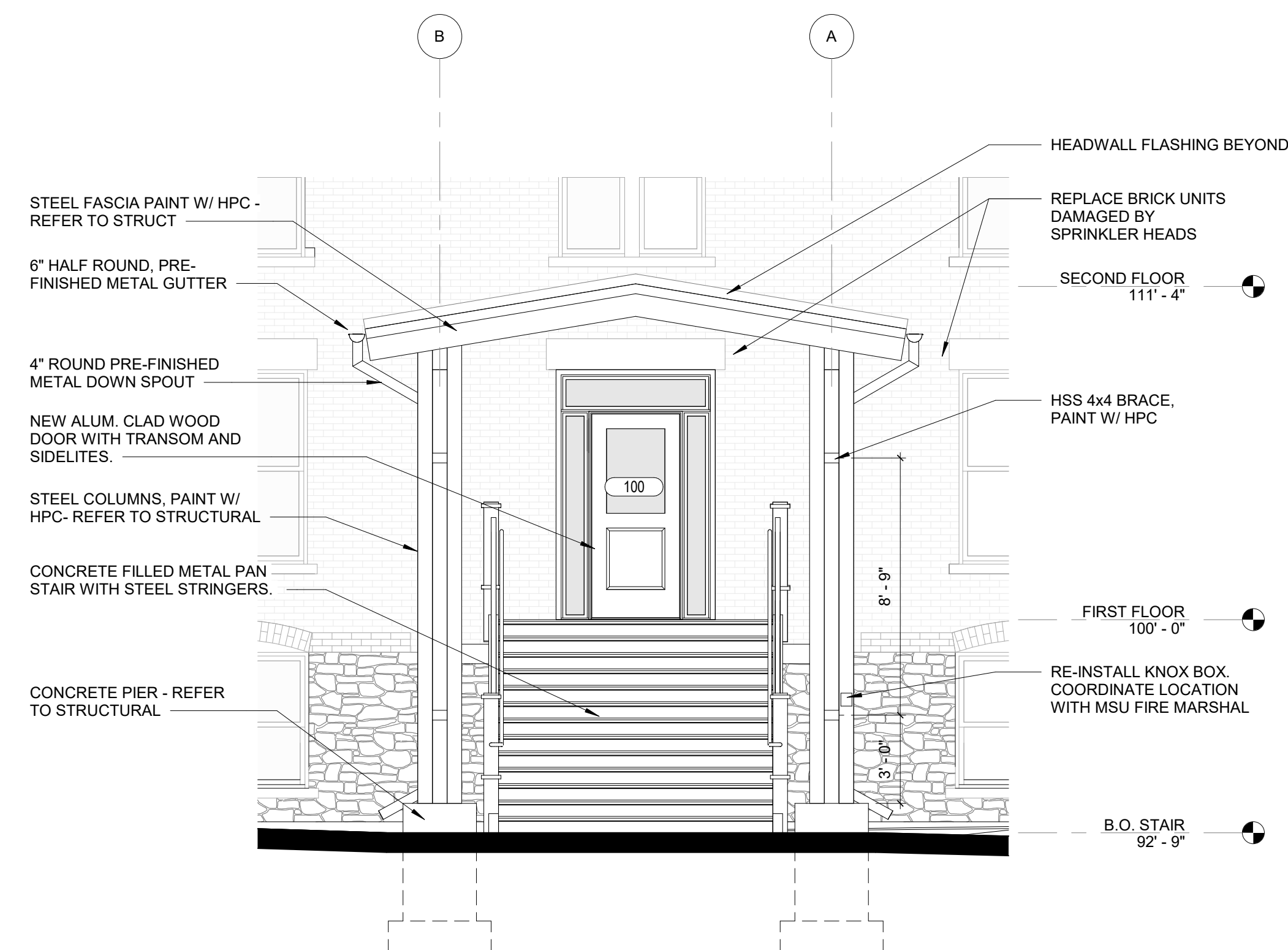
2
A1.1
01 NEW ENTRY CANOPY - WEST ELEVATION
1/4" = 1'-0"



3
A1.1
ENLARGED NEWAL POST DETAIL
1" = 1'-0"



4
A1.1
STAIR ELEVATION
1/4" = 1'-0"



5
A1.1
01 NEW ENTRY CANOPY - NORTH ELEVATION
1/4" = 1'-0"

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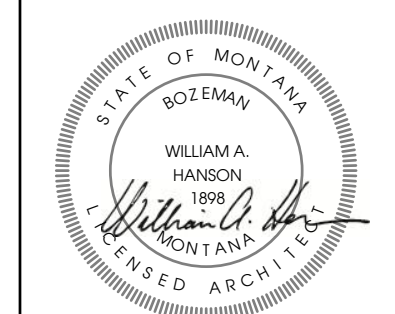
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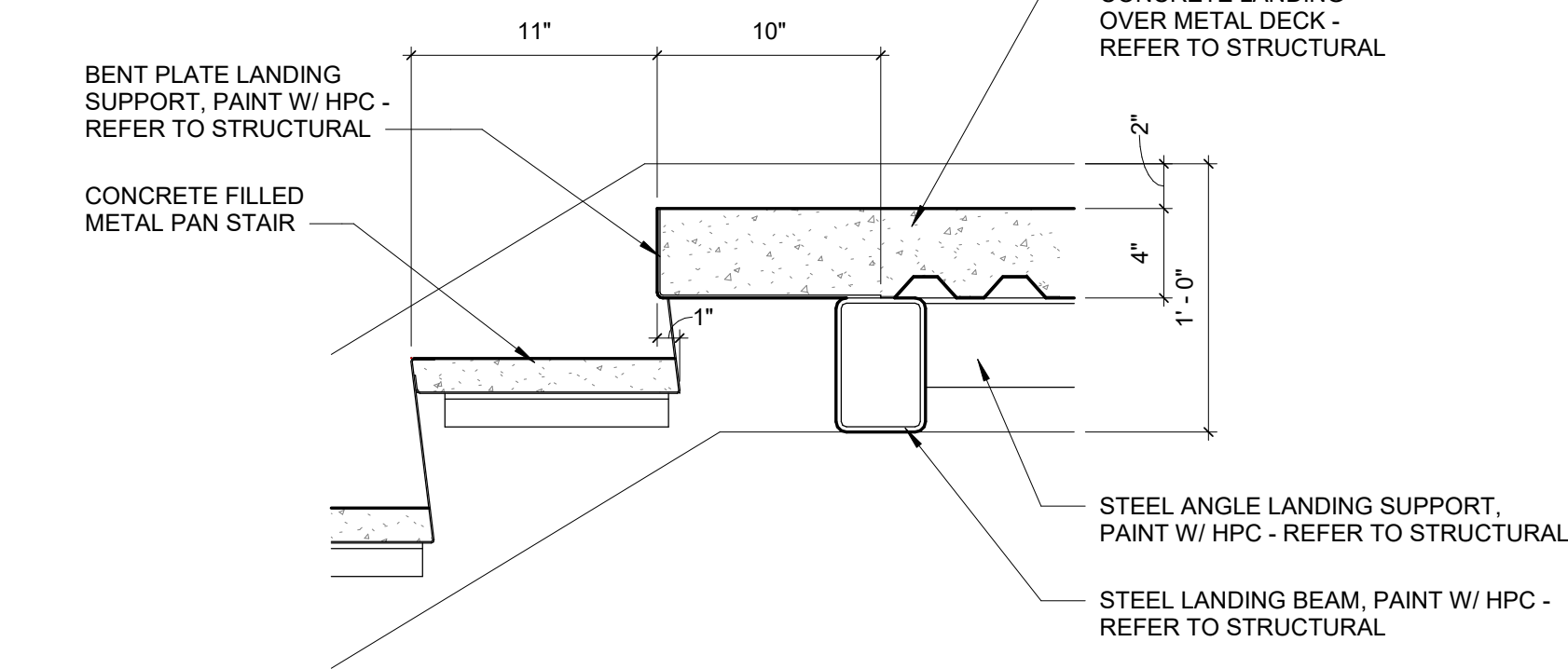
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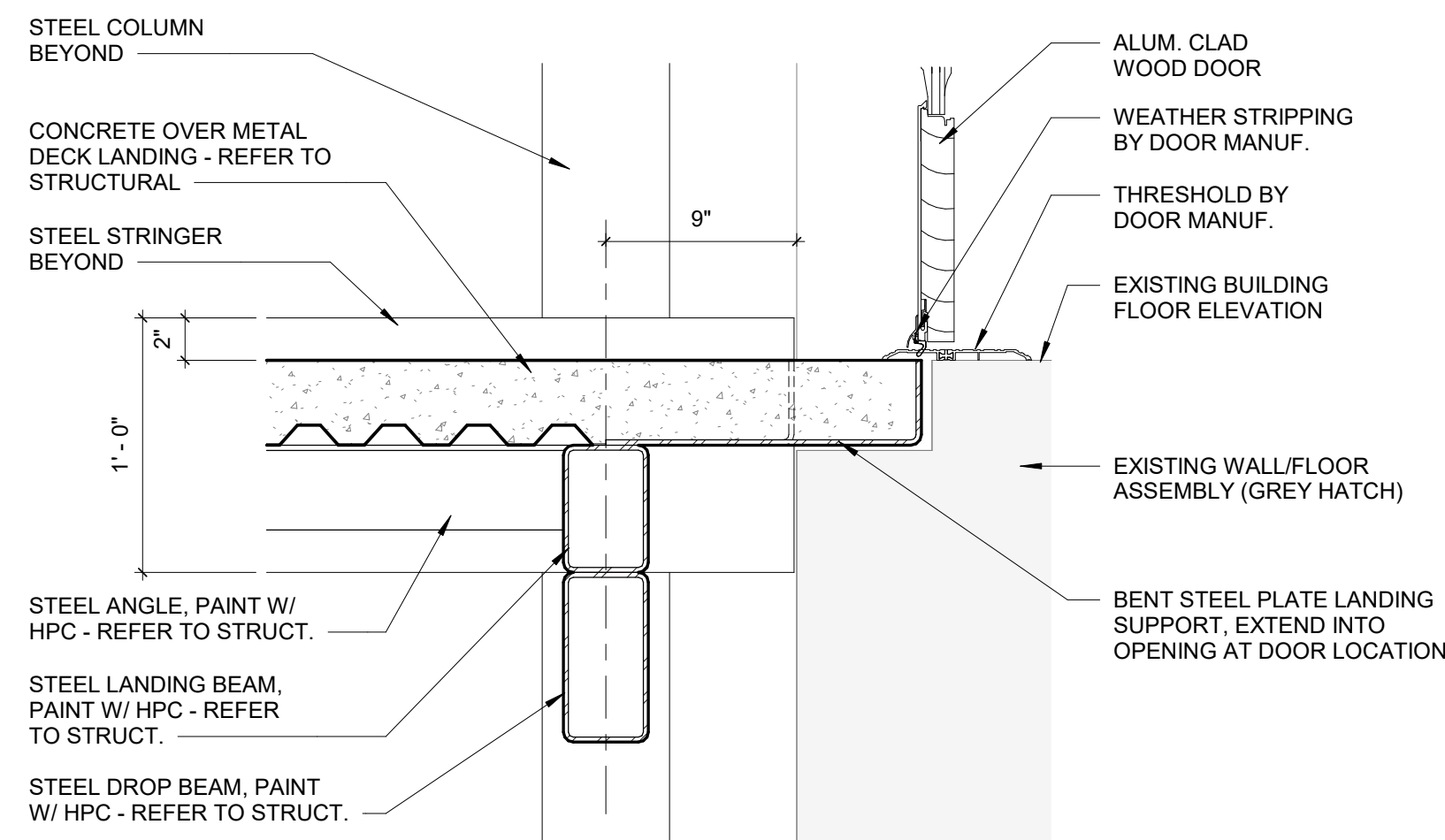
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SHEET TITLE
CANOPY
ELEVATIONS
SHEET
A1.1

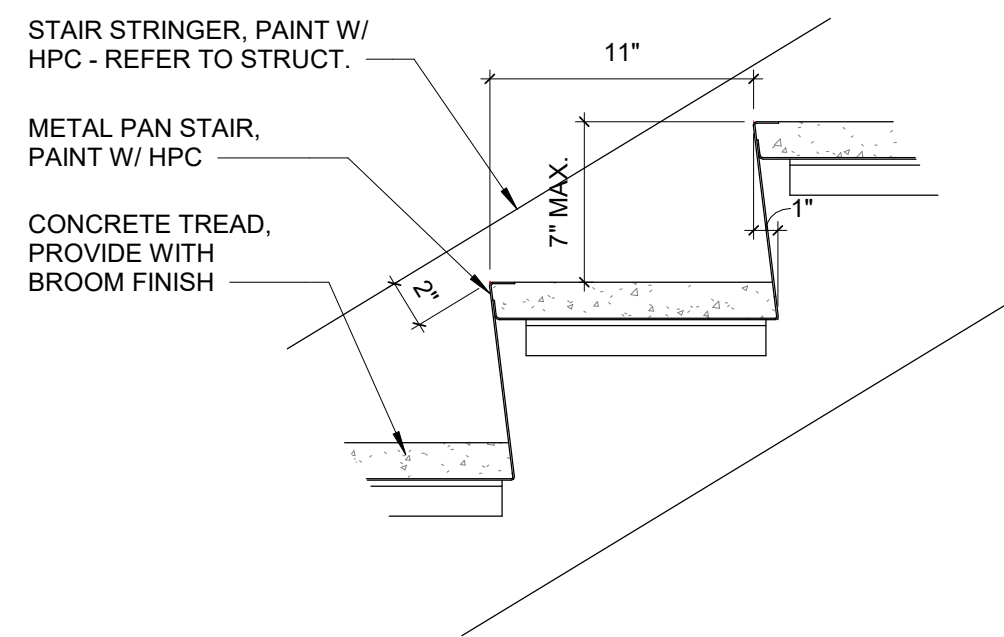
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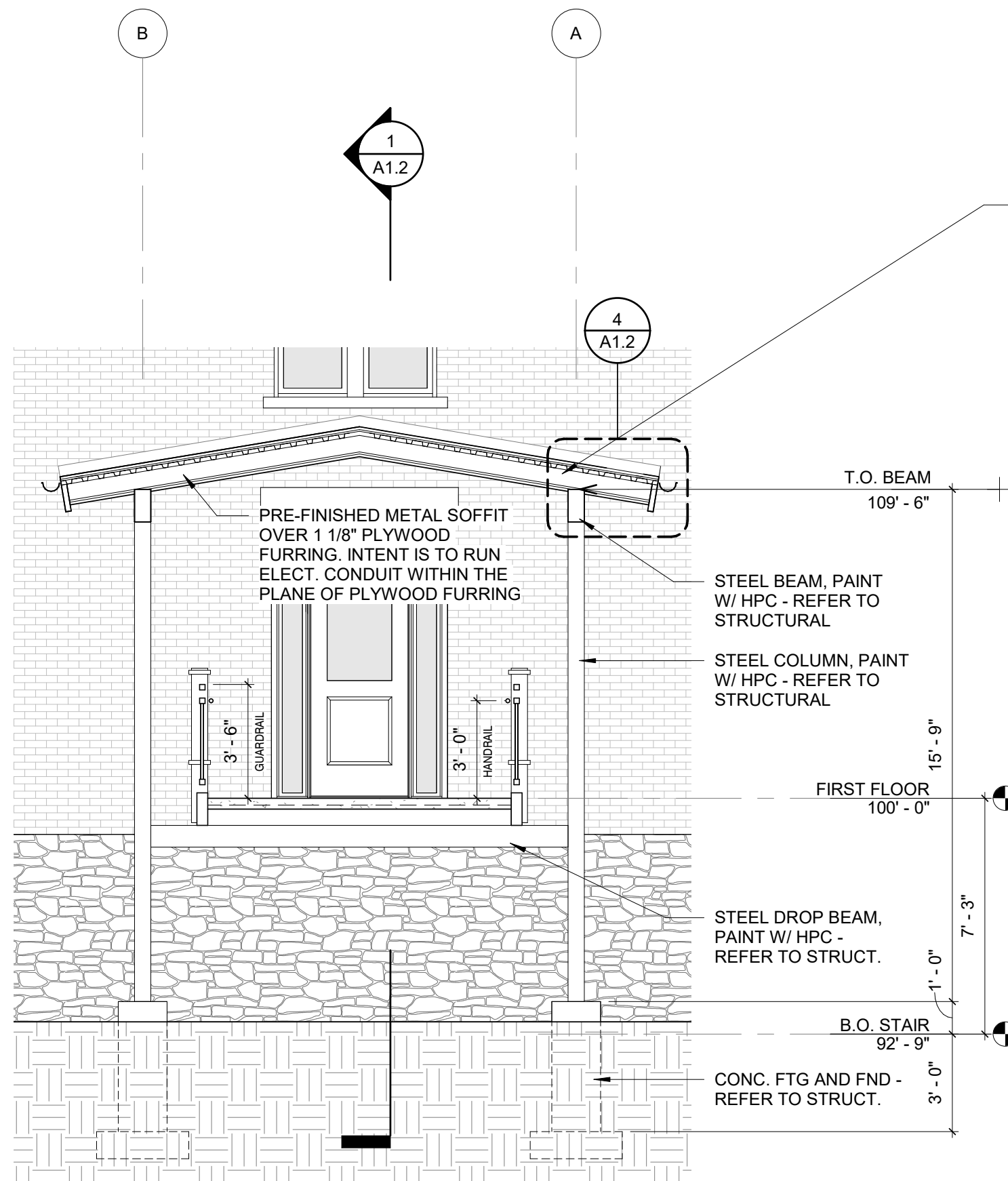
8 STAIR LANDING DETAIL No. 1
A1.2 1 1/2" = 1'-0"



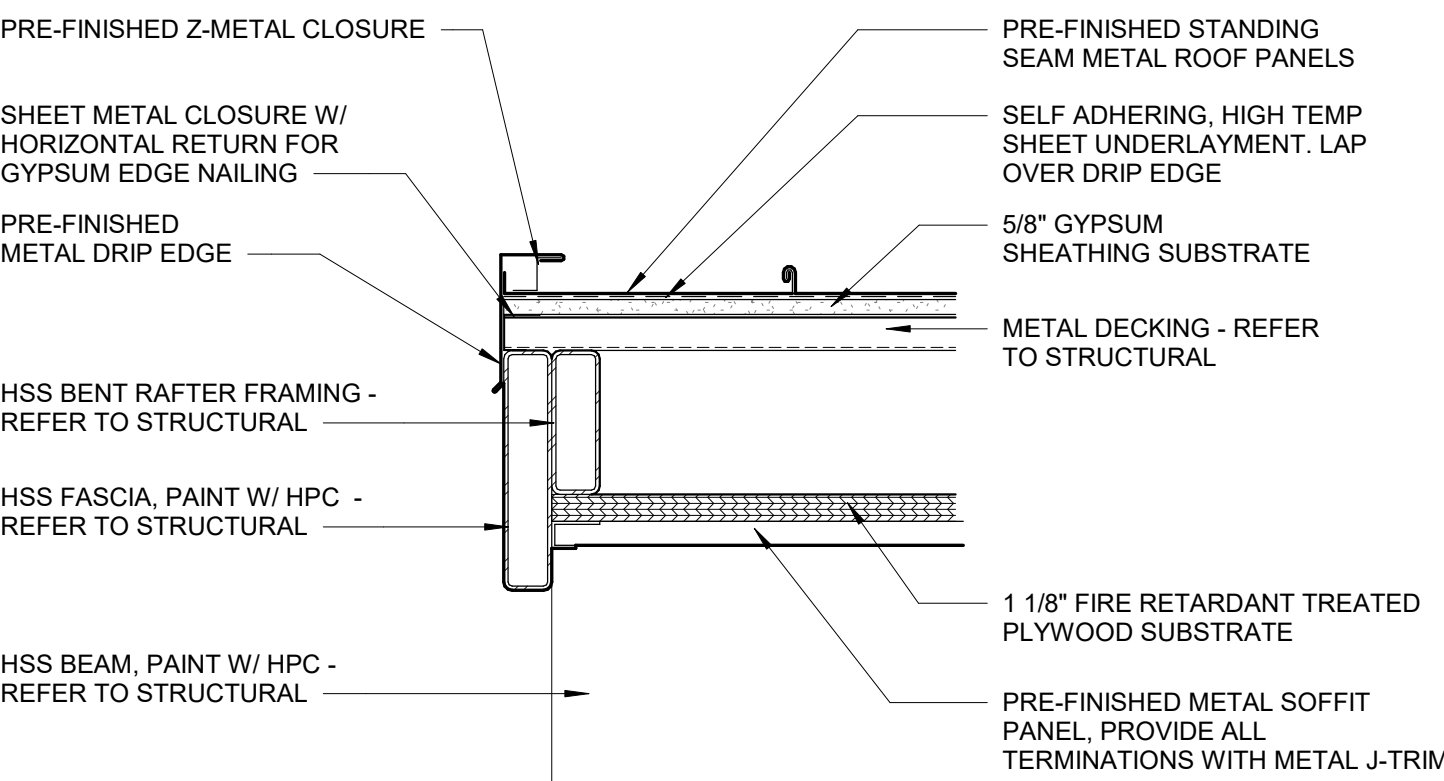
7 STAIR LANDING DETAIL No. 2
A1.2 1 1/2" = 1'-0"



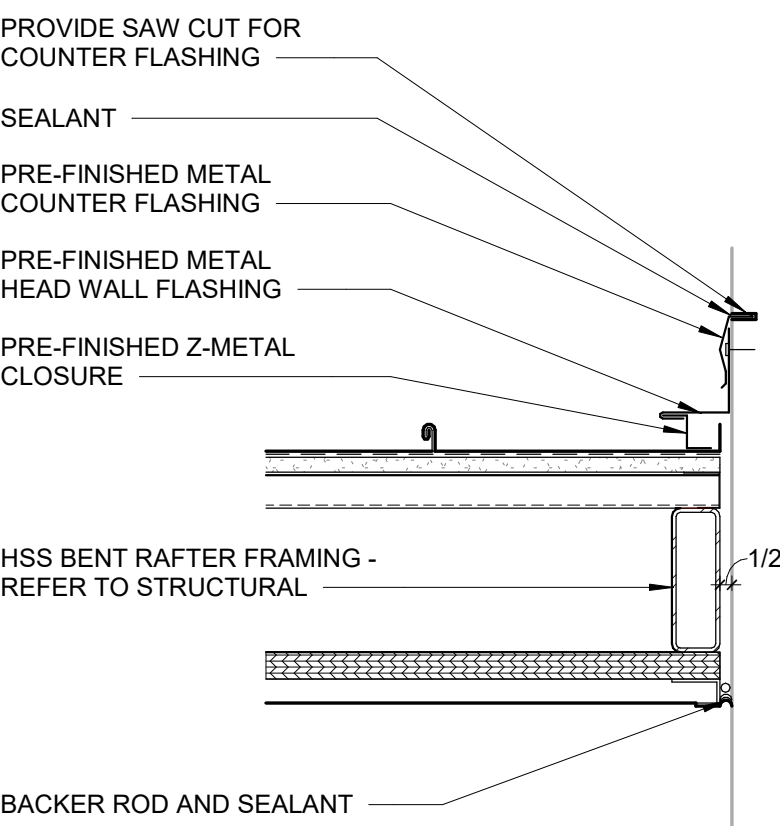
9 STAIR DETAIL
A1.2 1 1/2" = 1'-0"



2 E/W STAIR AND CANOPY SECTION
A1.2 1/4" = 1'-0"

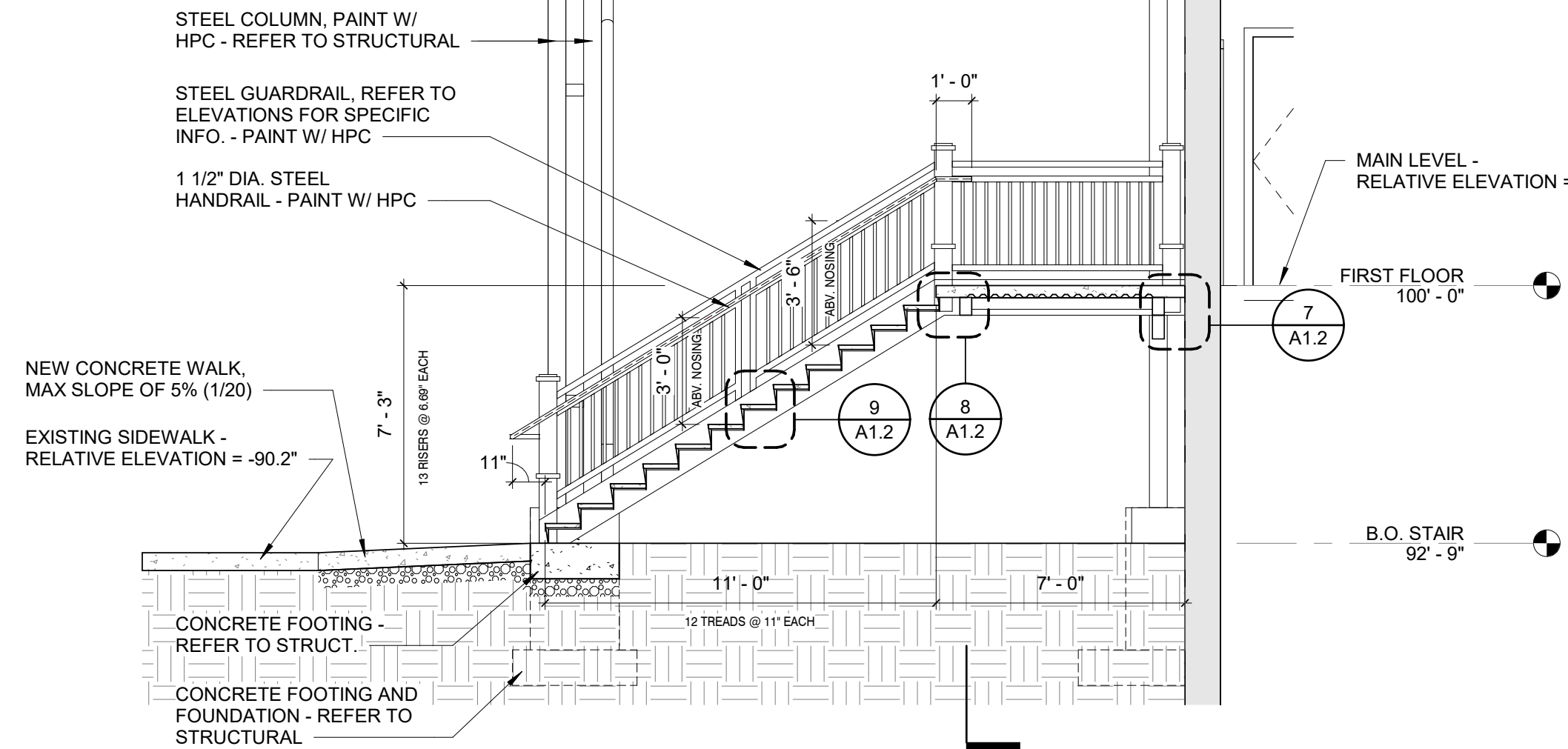


5 CANOPY RAKE DETAIL
A1.2 1 1/2" = 1'-0"

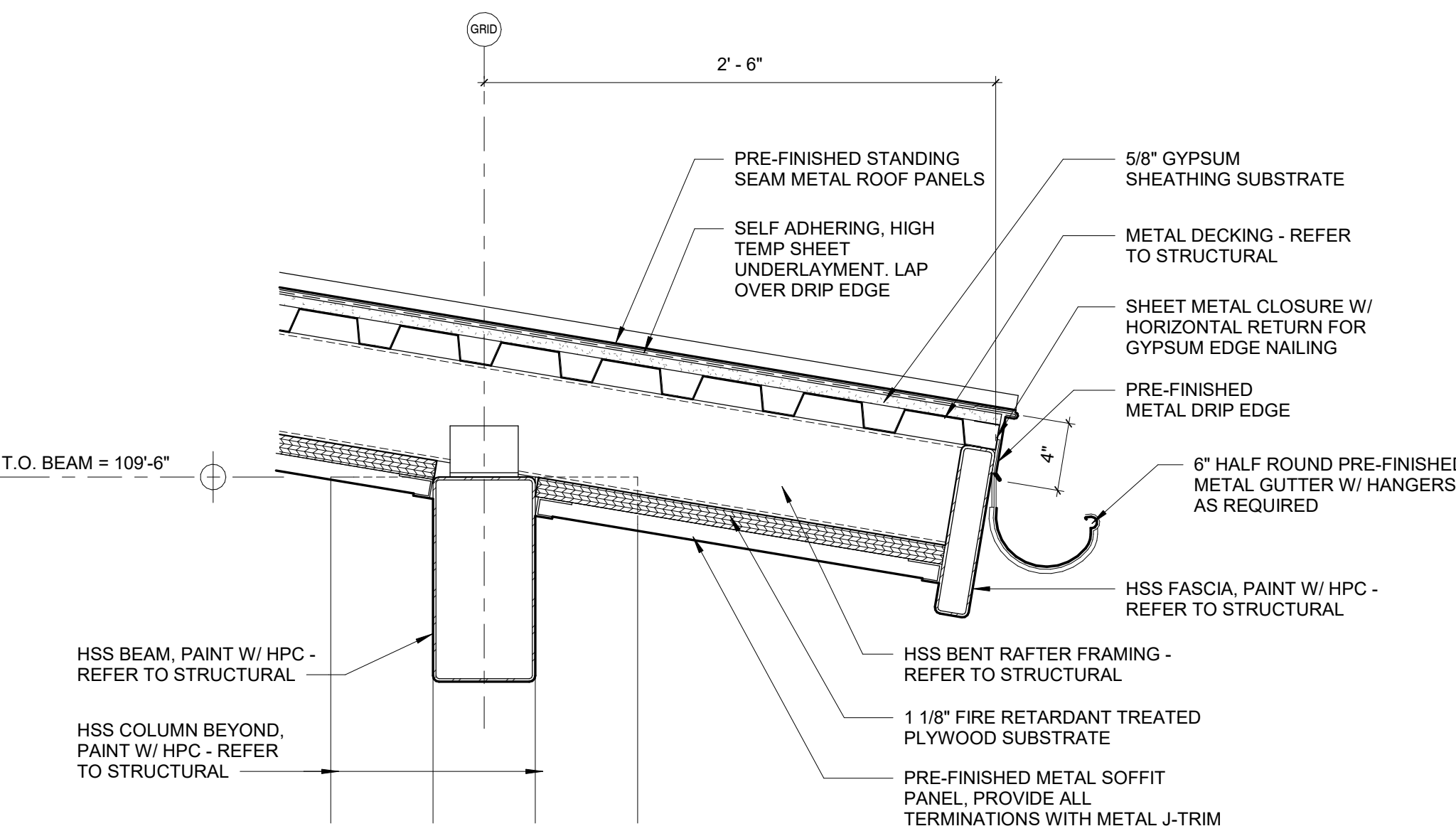


6 CANOPY HEAD WALL FLASHING DETAIL
A1.2 1 1/2" = 1'-0"

CANOPY ROOF ASSEMBLY
- PRE-FIN, STANDING SEAM MTL ROOF PANELS
- ICE & WATER SHIELD
- 5/8" GYPSUM SHEATHING
- METAL ROOF DECKING - REFER TO STRUCT.
- STEEL FRAMING - REFER TO STRUCT.



1 N/S STAIR AND CANOPY SECTION
A1.2 1/4" = 1'-0"



4 CANOPY EAVE DETAIL
A1.2 1 1/2" = 1'-0"

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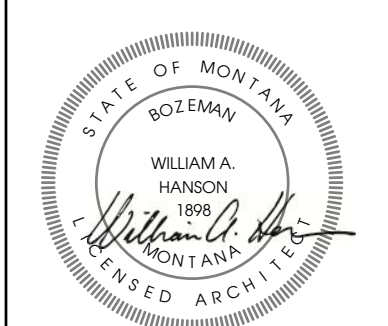


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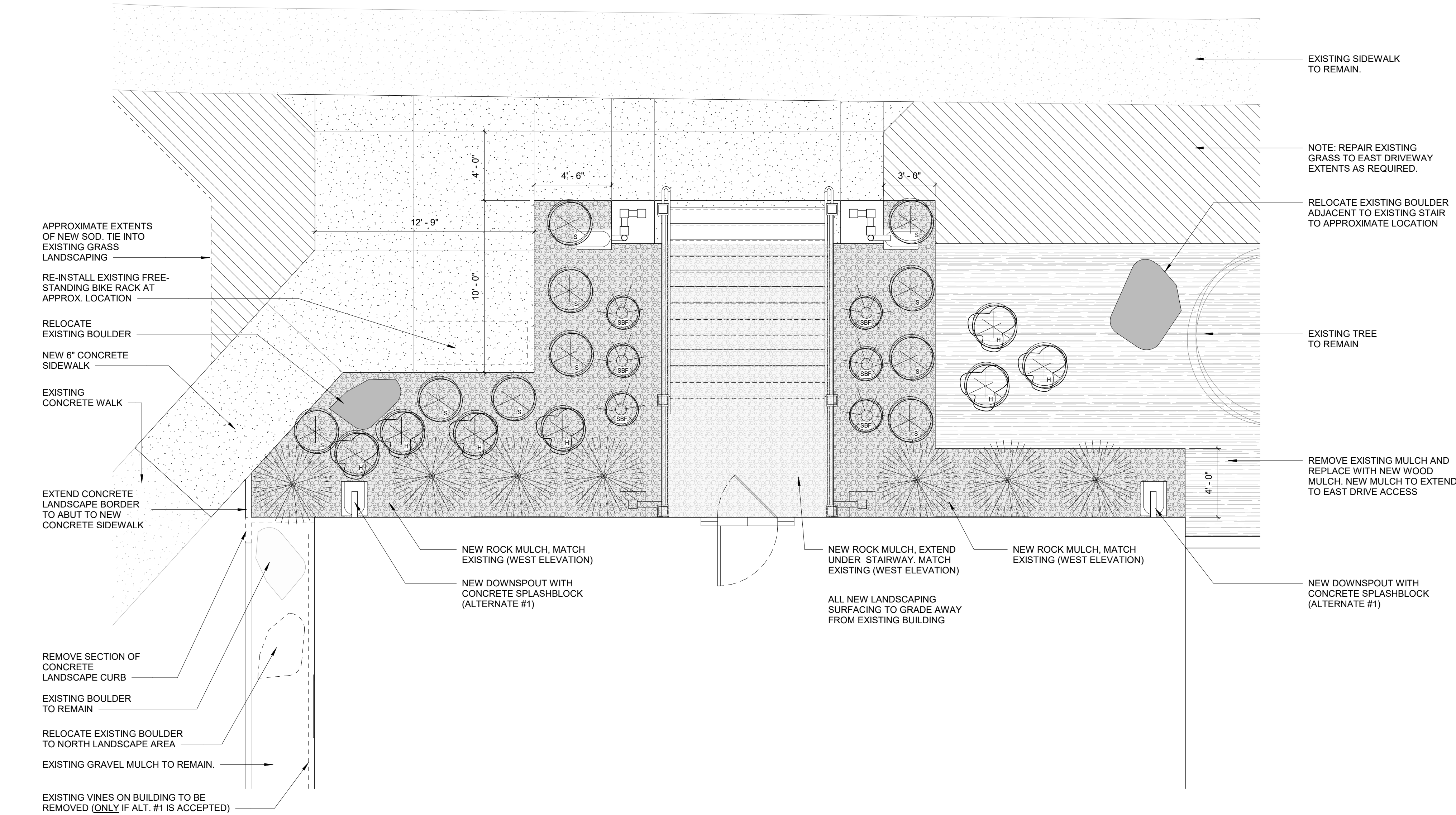
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SHEET TITLE
CANOPY DETAILS

SHEET
A1.2
DATE
03/10/25

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LANDSCAPING LEGEND		
SYMBOL	IMAGE	PLANT NAME/SIZE
		KARL FOERSTER FEATHER REED GRASS (1 GALLON)
		SISKIYOU BLUE FESCUE GRASS (1 GALLON)
		HELIOPSIS 'SUNSTRUCK' (1 GALLON)
		SALVIA 'MAY NIGHT' (1 GALLON)
	NEW ROCK MULCH, TO MATCH EXISTING (WEST PLANTING)	
	NEW WOOD MULCH, COLOR TBD IN COORDINATION WITH MSU	
	NEW/REPAIRED SOD GRASS	

NOTE: CONTRACTOR TO TIE INTO EXISTING IRRIGATION SYSTEM, AND PROVIDE
ADEQUATE IRRIGATION FOR ALL NEW PLANTINGS INDICATED ON LANDSCAPE
PLAN. ADJUST SPRINKLER HEADS/LOCATIONS AS NEEDED TO COVER ADDITIONAL
GRASS AND PLANTINGS.



1 LANDSCAPE PLAN
L1.0 1/4" = 1'-0"

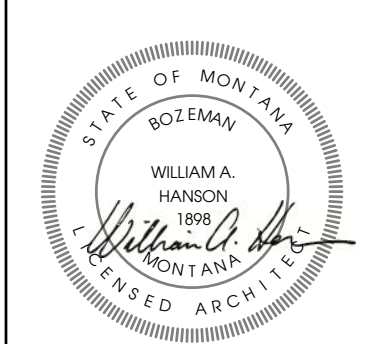


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SHEET TITLE
LANDSCAPE
PLAN

SHEET
L1.0

DATE
03/10/25

100% CONSTRUCTION DOCUMENTS



ALL WORK DESCRIBED ON THIS SHEET IS ALTERNATE #1 UNLESS SPECIFICALLY NOTED OTHERWISE

GENERAL NEW CONSTRUCTION KEY NOTES

- 1 REMOVE EXISTING CONCRETE APRON IN ITS ENTIRETY. CLEAN SURFACE OF STONE FOUNDATION OF ALL REMAINING CONCRETE DEBRIS. REPOINT ALL MASONRY JOINTS IN THIS AREA.
- 2 PROVIDE BACKER ROD AND SEALANT AROUND PIPE PENETRATION
- 3 VERIFY STL ANGLE EXISTS IF NOT, PROVIDE STEEL LINTEL WITH AT LEAST 4" OF BEARING
- 4 NEW PRE-FINISHED 6" HALF ROUND METAL GUTTER
- 5 NEW PRE-FINISHED 4" DIA METAL DOWN SPOUT. PROVIDE WITH PRE-CAST CONC. SPLASH BLOCK
- 6 INSTALL BACKER ROD AND SEALANT ALONG JOINT BETWEEN ORIGINAL BUILDING AND ADDITION
- 7 MAINTAIN GHOSTING OUTLINE OF PREVIOUS BUILDING ELEMENT
- 8 REMOVE EXISTING FIRE SPRINKLER HEAD AS REQUIRED. REPLACE DAMAGED BRICK UNITS. ASSUME (3) BRICKS
- 9 REMOVE PAINT SPLATTER FROM BRICK. SEE PHOTO 12/A2.1a

BRICK REPAIR KEY NOTES		AREA OUTLINE	REMARKS, SEE KEY
CODE	DESCRIPTION OF WORK		
B1-#	REPLACE BRICK UNITS DAMAGED BY EXISTING FASTENERS. REMOVE ANY ANCHORS THAT MAY STILL EXIST		1,3,4
B2-#	REPLACE SPALLED / DAMAGED / MISSING BRICK UNIT. (ASSUME STRETCHER COURSE U.N.O.)		1,3,4
B3-#	REPLACE SPALLED / DAMAGED / MISSING BRICK @ WINDOW ARCH		1,3,4
B4-# SF	REBUILD OUTER WYTHE OF WALL WITH MATCHING BRICK		2,5,8
B6-#	REMOVE WOOD BOARD AND ITS FASTENERS. REPLACE BRICK UNITS DAMAGE BY FASTENERS.		1,3,4
B8-# SF	REBUILD WITH EXISTING BRICK.		2,8
B8-ARC	REBUILD BRICK ARCH WITH EXISTING BRICK. RE-CAULK JOINT BETWEEN THE INTRADOS AND WINDOW HEAD		8
B8-BC	REBUILD BELT COURSE WITH EXISTING BRICK.		8
B9-# SF	REMOVE EXISTING CONCRETE FROM STONE MASONRY		2

REPOINTING KEY NOTES		AREA OUTLINE	REMARKS, SEE KEY
CODE	DESCRIPTION OF WORK		
REP-# SF	REPOINT MORTAR JOINTS		2,6
REP-ARC	REPOINT ALL MORTAR JOINTS ALONG BRICK ARCH INCLUDING JOINTS ALONG THE INTRADOS AND SKYWARD JOINTS		6,7
REP-BC#	REPOINT MORTAR JOINTS ALONG BELT COURSE INCLUDING ALL SKYWARD JOINTS		6,7

SIDING, SOFFIT AND FASCIA REPAIR KEY NOTES		AREA OUTLINE	REMARKS, SEE KEY
CODE	DESCRIPTION OF WORK		
P-S	RE-PAINT EXISTING SIDING		
P-SO	RE-PAINT EXISTING SOFFIT		
P-F	RE-PAINT EXISTING FASCIA		
P-T	RE-PAINT EXISTING TRIM WORK		

- 1 # INDICATES NUMBER OF BRICK UNITS IN NEED OF REPLACEMENT.
- 2 # INDICATES SQUARE FOOTAGE OF INTENDED WORK
- 3 CONTRACTOR OPTION TO TURN BRICK AROUND IF BACKSIDE IS IN GOOD CONDITION.
- 4 ASSUME ALL JOINTS SURROUNDING REPLACED BRICK UNITS WILL BE REPOINTED
- 5 SALVAGE AS MANY REUSABLE BRICK UNITS AS POSSIBLE TO BE INCORPORATED INTO THE RE-BUILT AREA
- 6 REFER TO TYPICAL REPOINTING DETAIL ON A2.5
- 7 SKYWARD JOINTS ARE TO RECEIVE POLYURETHANE SEALANT, REFER TO SKYWARD JOINT REPOINTING DETAIL ON A2.5.
- 8 THE CONTRACTOR IS RESPONSIBLE FOR ADEQUATELY SUPPORTING MASONRY ABOVE ANY PORTIONS THAT ARE TO BE REBUILT.

1 WEST ELEVATION
A2.1 1/4" = 1'-0"

100% CONSTRUCTION DOCUMENTS



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TAYLOR HALL MASONRY AND ENTRY IMPROVEMENTS

MONTANA STATE UNIVERSITY

THINKONE

DRAWN BY: **Author**
REVIEWED BY: **Checker**

REV.	DESCRIPTION	DATE



PPA#22-0522
A/E#00-0-00
CONSULTANT #2308

SHEET TITLE
EXTERIOR ELEVATION - W

SHEET
A2.1

DATE
03/10/25

ALL WORK DESCRIBED ON THIS SHEET IS ALTERNATE #1 UNLESS SPECIFICALLY NOTED OTHERWISE



1 WEST ELEVATION PHOTO



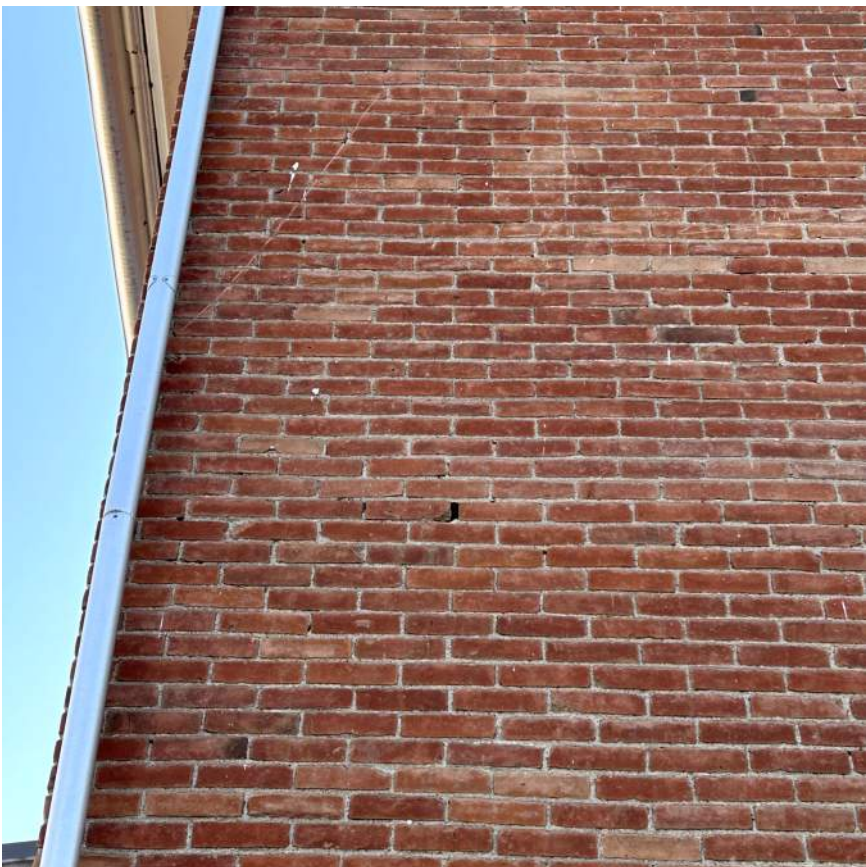
14 WEST DETAIL PHOTO 14



11 WEST DETAIL PHOTO 11



8 WEST DETAIL PHOTO 8



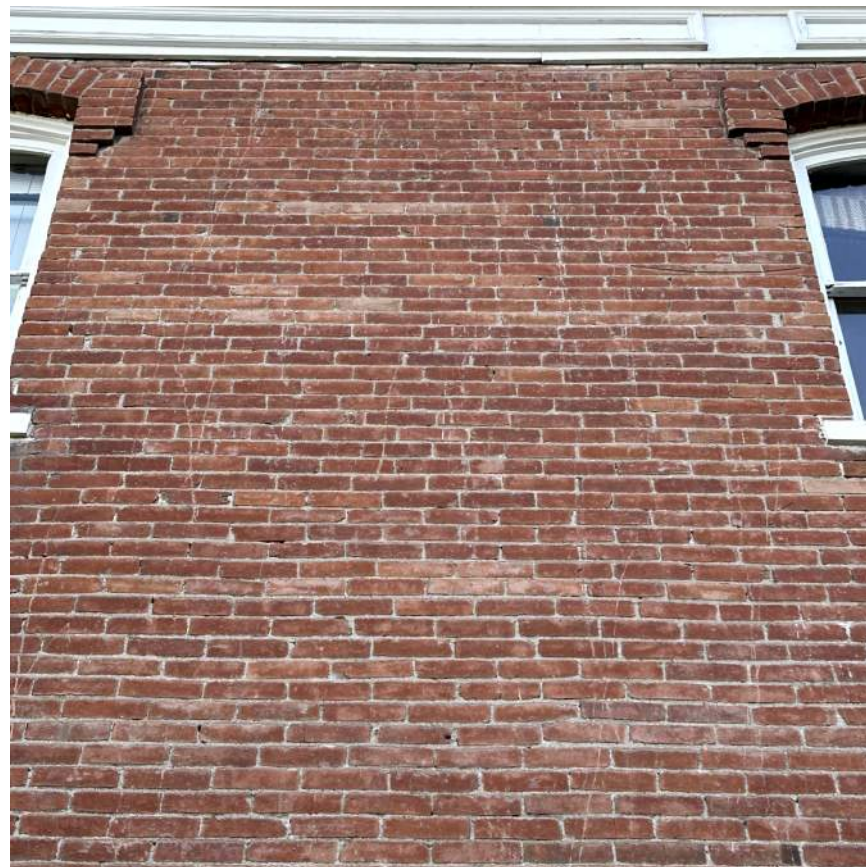
15 WEST DETAIL PHOTO 15



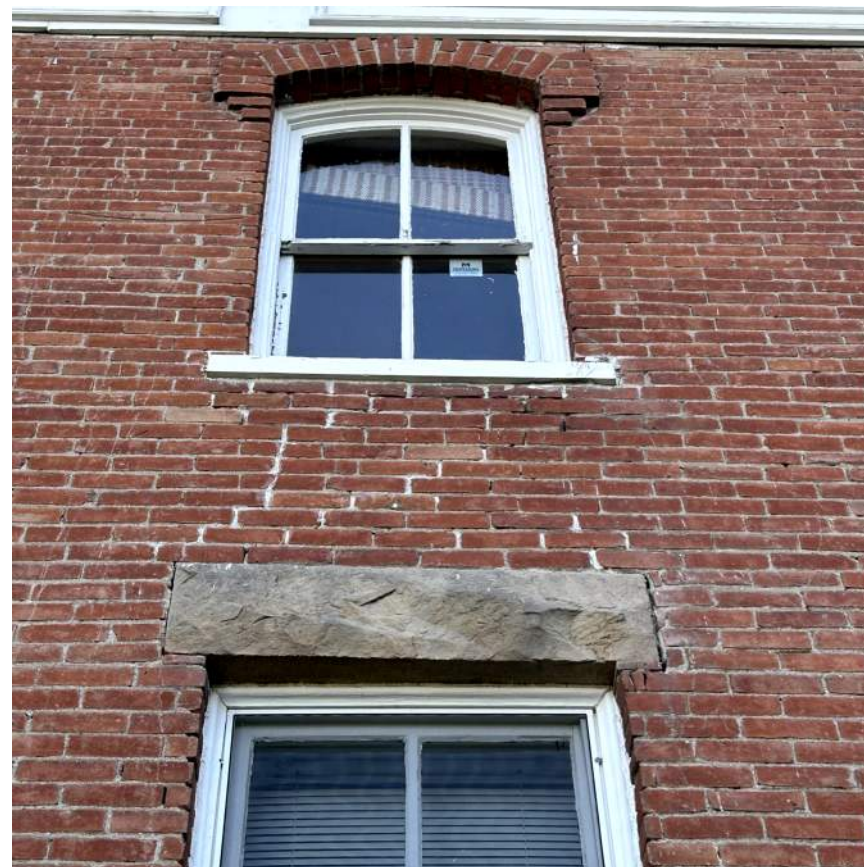
12 WEST DETAIL PHOTO 12



9 WEST DETAIL PHOTO 9



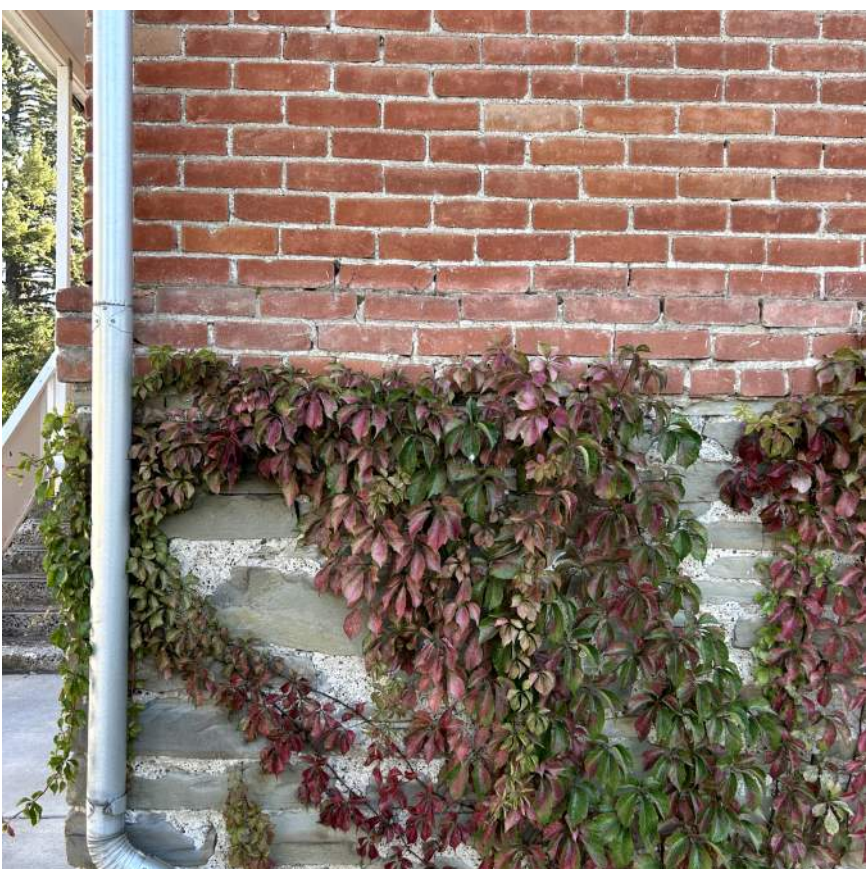
6 WEST DETAIL PHOTO 6



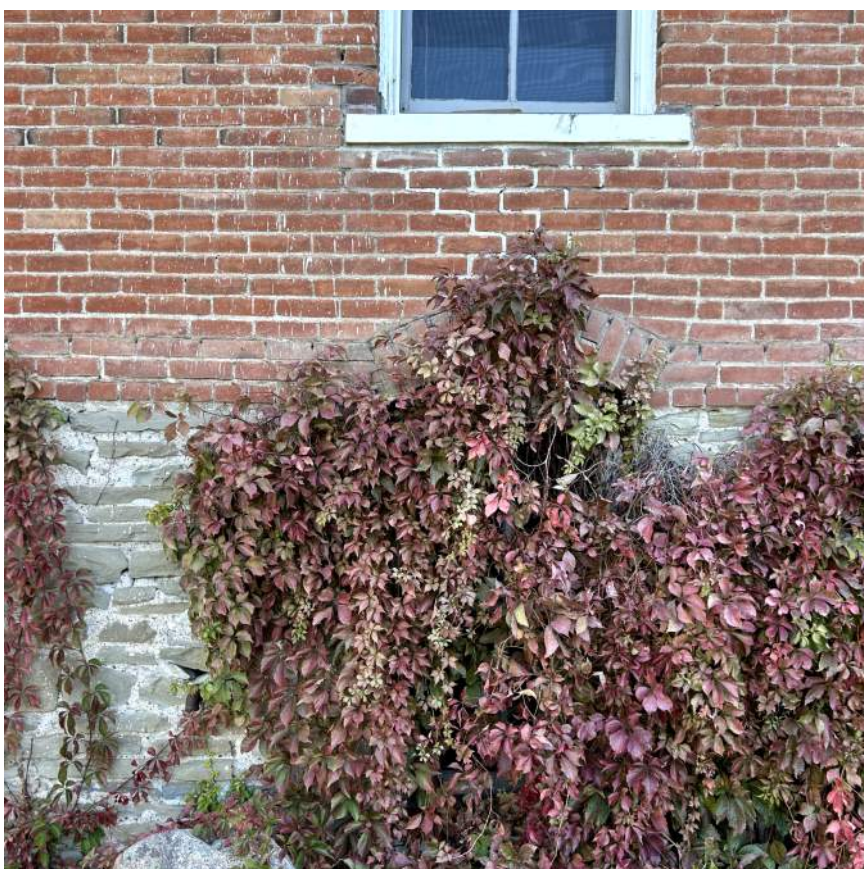
4 WEST DETAIL PHOTO 4



2 WEST DETAIL PHOTO 2



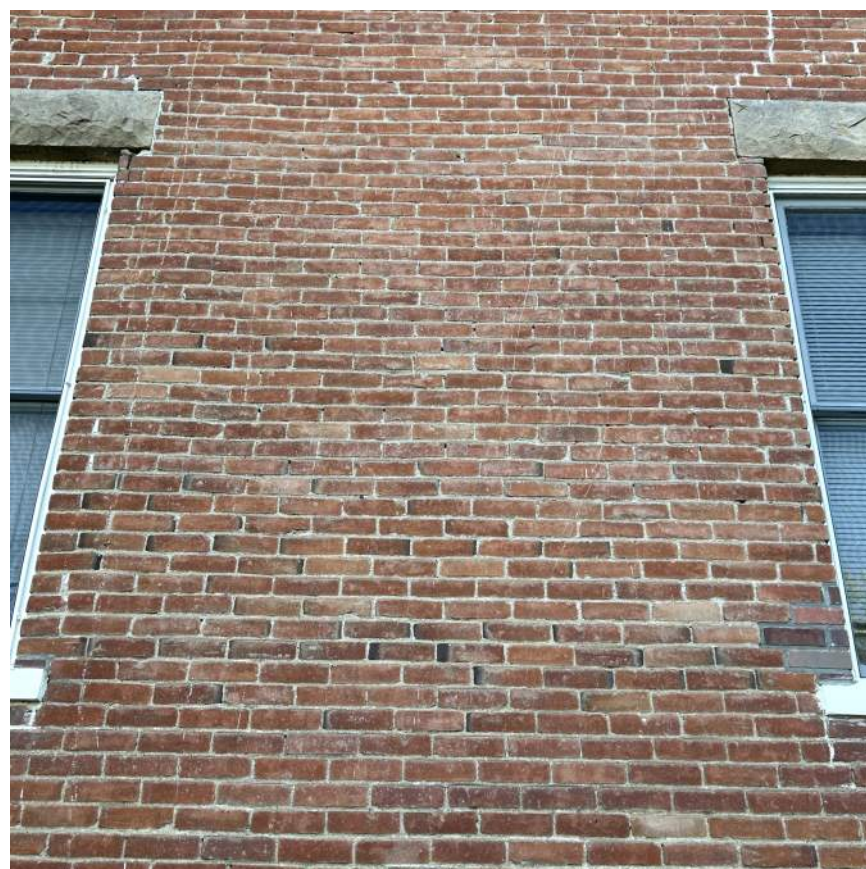
16 WEST DETAIL PHOTO 16



13 WEST DETAIL PHOTO 13



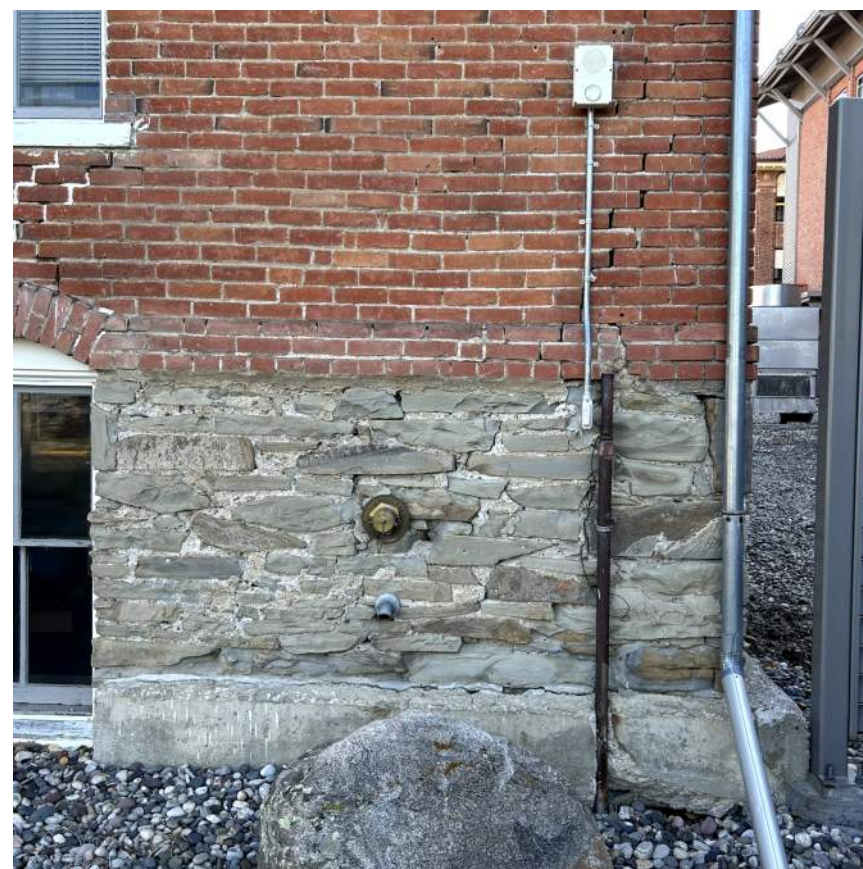
10 WEST DETAIL PHOTO 10



7 WEST DETAIL PHOTO 7



5 WEST DETAIL PHOTO 5



3 WEST DETAIL PHOTO 3

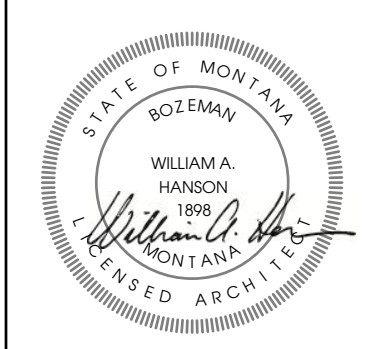


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TAYLOR HALL MASONRY AND
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DRAWN BY: Author		
REVIEWED BY: Checker		
REV.	DESCRIPTION	DATE



PPA#22-0522
A/E#00-0-00
CONSULTANT #2308

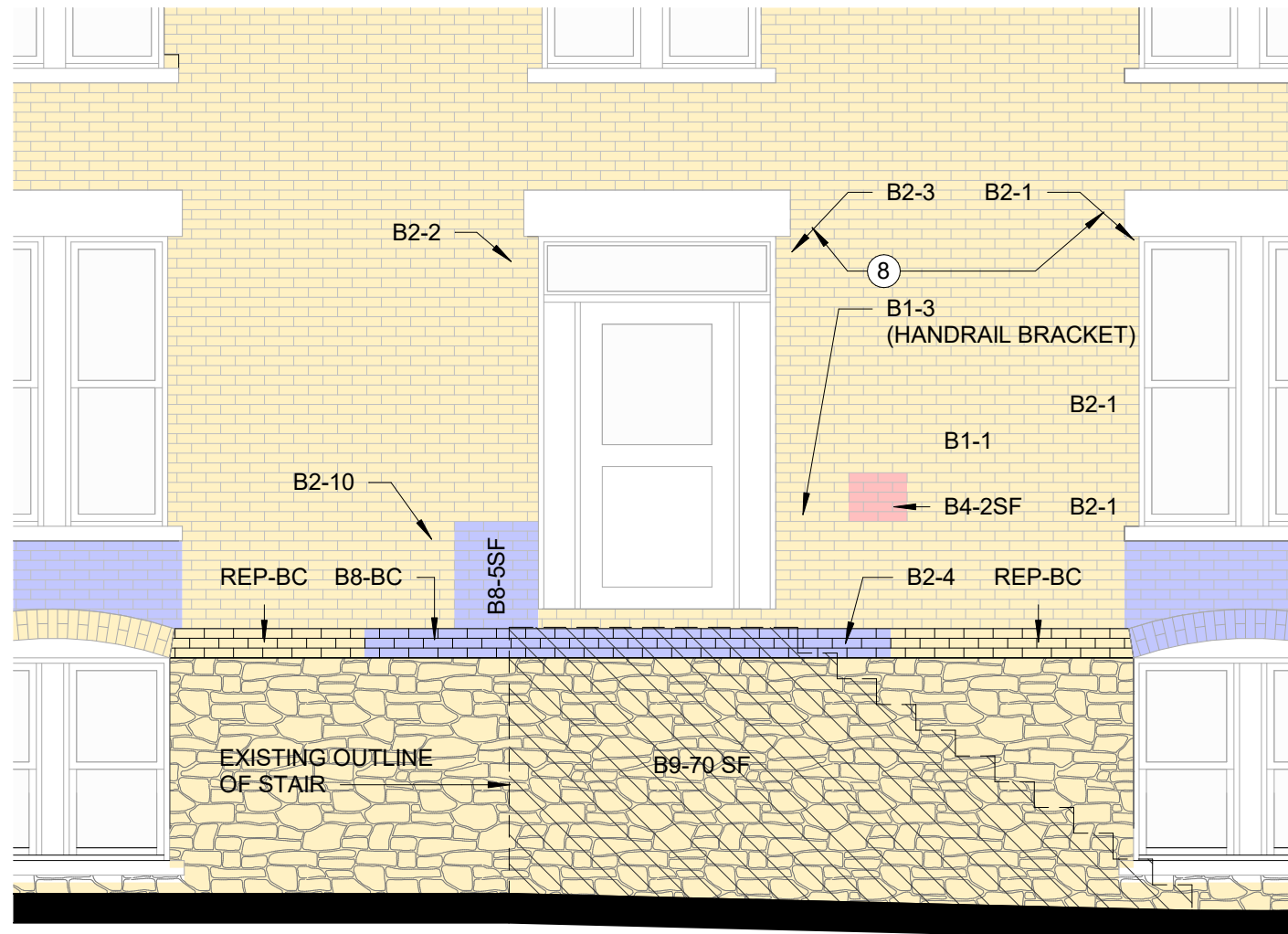
SHEET TITLE
EXISTING
PHOTOS - WEST
SHEET
A2.1a

DATE
03/10/25

100% CONSTRUCTION DOCUMENTS



1 NORTH ELEVATION
A2.2 1/4" = 1'-0"



2 NORTH ELEVATION (WITHOUT NEW ENTRY)
A2.2 1/4" = 1'-0"

GENERAL NEW CONSTRUCTION KEY NOTES

- 1 REMOVE EXISTING CONCRETE APRON IN ITS ENTIRETY. CLEAN SURFACE OF STONE FOUNDATION OF ALL REMAINING CONCRETE DEBRIS. REPOINT ALL MASONRY JOINTS IN THIS AREA.
- 2 PROVIDE BACKER ROD AND SEALANT AROUND PIPE PENETRATION
- 3 VERIFY STL ANGLE EXISTS IF NOT, PROVIDE STEEL LINTEL WITH AT LEAST 4" OF BEARING
- 4 NEW PRE-FINISHED 6" HALF ROUND METAL GUTTER
- 5 NEW PRE-FINISHED 4" DIA METAL DOWN SPOUT. PROVIDE WITH PRE-CAST CONC. SPLASH BLOCK
- 6 INSTALL BACKER ROD AND SEALANT ALONG JOINT BETWEEN ORIGINAL BUILDING AND ADDITION
- 7 MAINTAIN GHOSTING OUTLINE OF PREVIOUS BUILDING ELEMENT
- 8 REMOVE EXISTING FIRE SPRINKLER HEAD AS REQUIRED. REPLACE DAMAGED BRICK UNITS. ASSUME (3) BRICKS
- 9 REMOVE PAINT SPLATTER FROM BRICK. SEE PHOTO 12/A2.1a

BRICK REPAIR KEY NOTES		AREA OUTLINE	REMARKS SEE KEY
CODE	DESCRIPTION OF WORK		
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B2-#	REPLACE SPALLED / DAMAGED / MISSING BRICK UNIT. (ASSUME STRETCHER COURSE U.N.O.)		1,3,4
B3-#	REPLACE SPALLED / DAMAGED / MISSING BRICK @ WINDOW ARCH		1,3,4
B4-# SF	REBUILD OUTER WYTHE OF WALL WITH MATCHING BRICK		2,5,8
B6-#	REMOVE WOOD BOARD AND ITS FASTENERS. REPLACE BRICK UNITS DAMAGE BY FASTENERS.		1,3,4
B8-# SF	REBUILD WITH EXISTING BRICK.		2,8
B8-ARC	REBUILD BRICK ARCH WITH EXISTING BRICK. RE-CAULK JOINT BETWEEN THE INTRADOS AND WINDOW HEAD		8
B8-BC	REBUILD BELT COURSE WITH EXISTING BRICK.		8
B9-# SF	REMOVE EXISTING CONCRETE FROM STONE MASONRY		2

REPOINTING KEY NOTES		AREA OUTLINE	REMARKS SEE KEY
CODE	DESCRIPTION OF WORK		
REP-# SF	REPOINT MORTAR JOINTS		2,6
REP-ARC	REPOINT ALL MORTAR JOINTS ALONG BRICK ARCH INCLUDING JOINTS ALONG THE INTRADOS AND SKYWARD JOINTS		6,7
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SIDING, SOFFIT AND FASCIA REPAIR KEY NOTES		AREA OUTLINE	REMARKS SEE KEY
CODE	DESCRIPTION OF WORK		
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P-SO	RE-PAINT EXISTING SOFFIT		
P-F	RE-PAINT EXISTING FASCIA		
P-T	RE-PAINT EXISTING TRIM WORK		

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- 4 ASSUME ALL JOINTS SURROUNDING REPLACED BRICK UNITS WILL BE REPOINTED
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- 8 THE CONTRACTOR IS RESPONSIBLE FOR ADEQUATELY SUPPORTING MASONRY ABOVE ANY PORTIONS THAT ARE TO BE REBUILT.

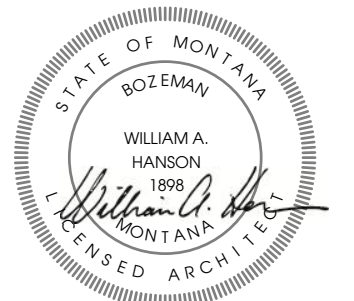


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TAYLOR HALL MASONRY AND
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MONTANA STATE UNIVERSITY



DRAWN BY: Author	REVIEWED BY: Checker
REV.	DESCRIPTION
	DATE



PPA#22-0522
A/E#00-0-00
CONSULTANT #2308

SHEET TITLE
EXTERIOR
ELEVATION - N
SHEET
A2.2

DATE
03/10/25

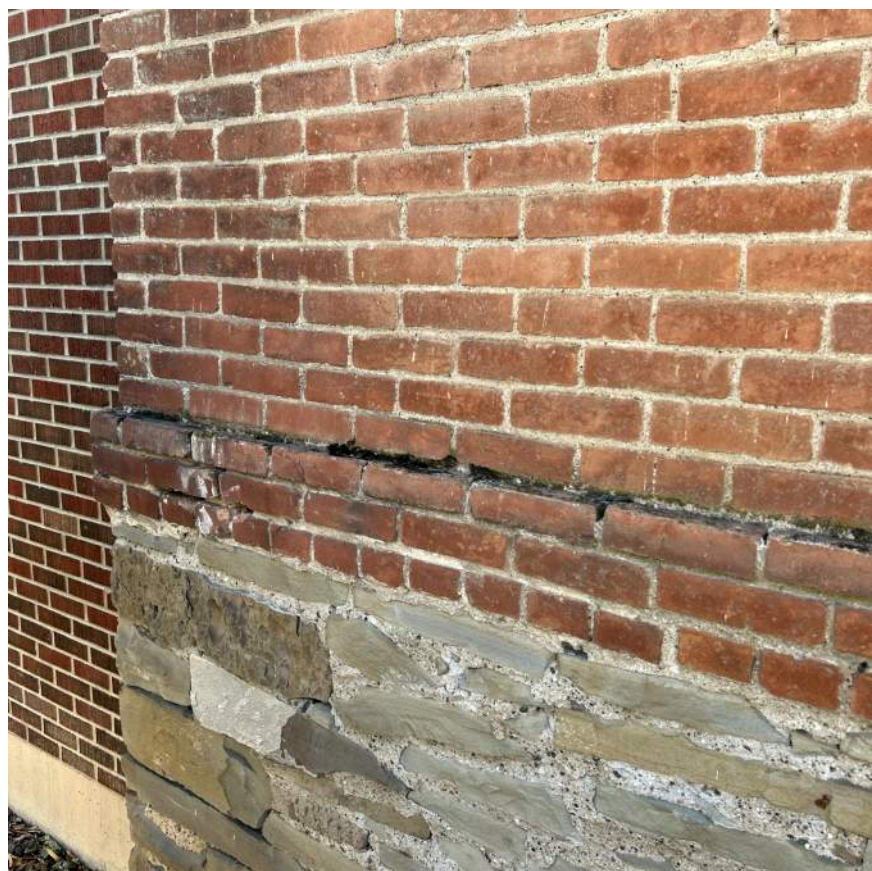
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8 NORTH DETAIL PHOTO 8



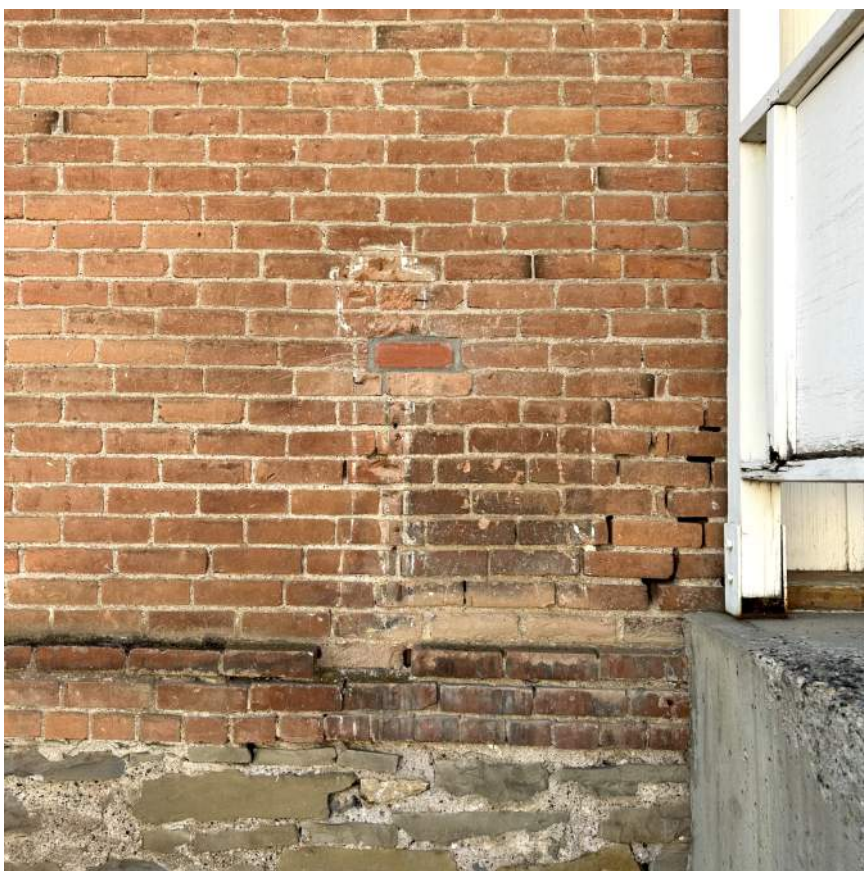
1 NORTH ELEVATION PHOTO



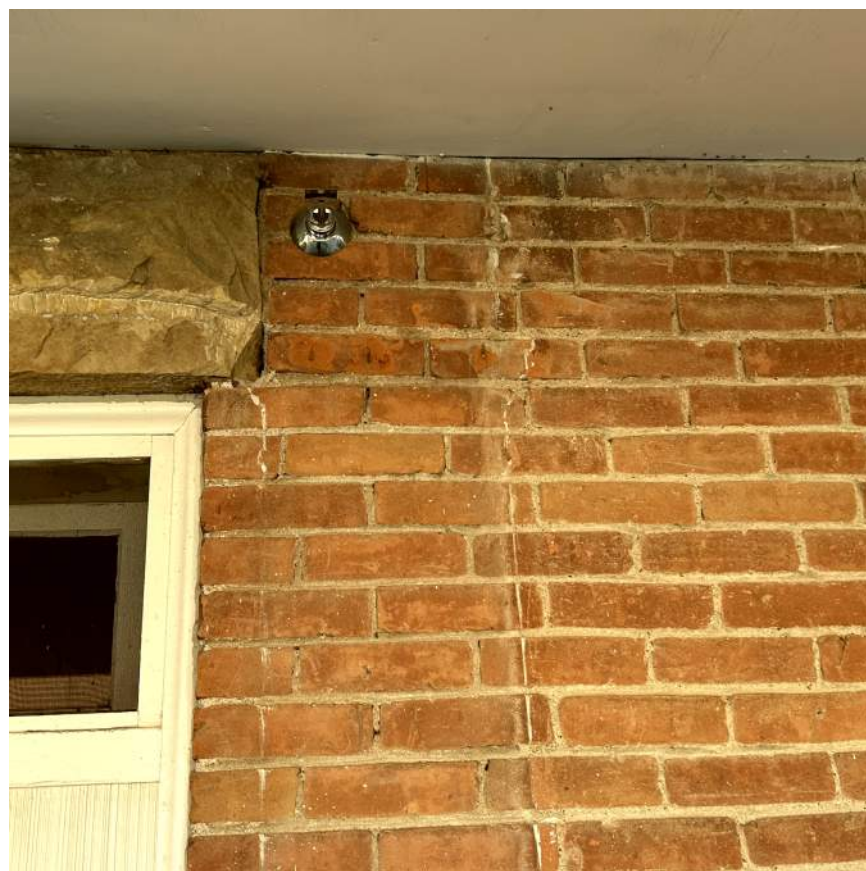
13 NORTH DETAIL PHOTO 13



11 NORTH DETAIL PHOTO 11



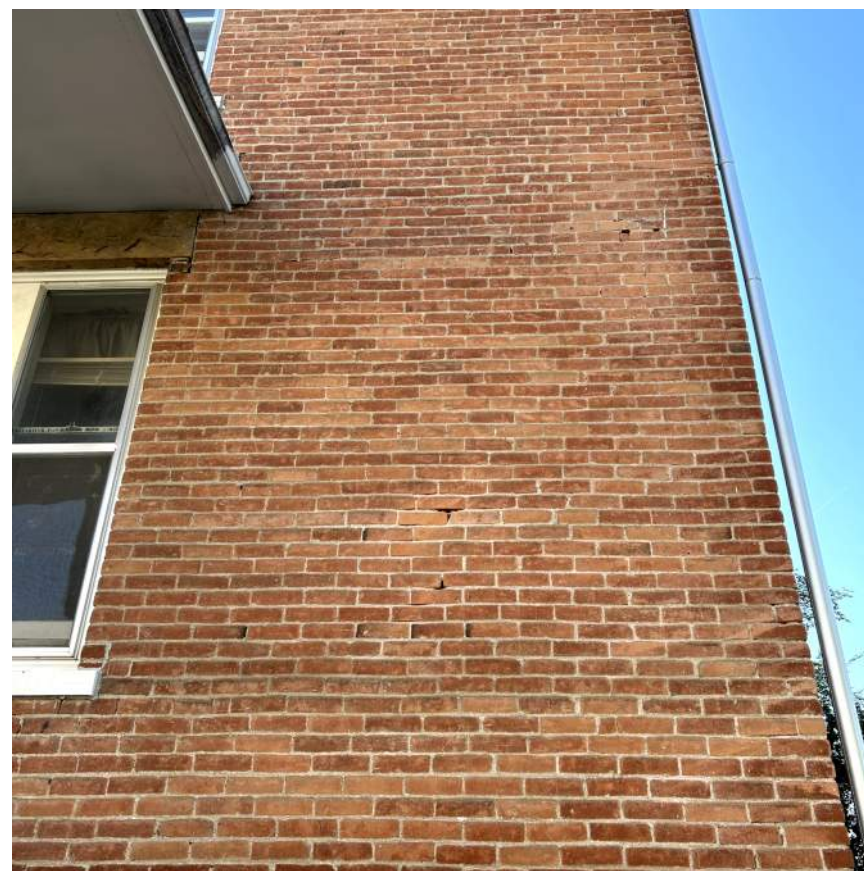
9 NORTH DETAIL PHOTO 9



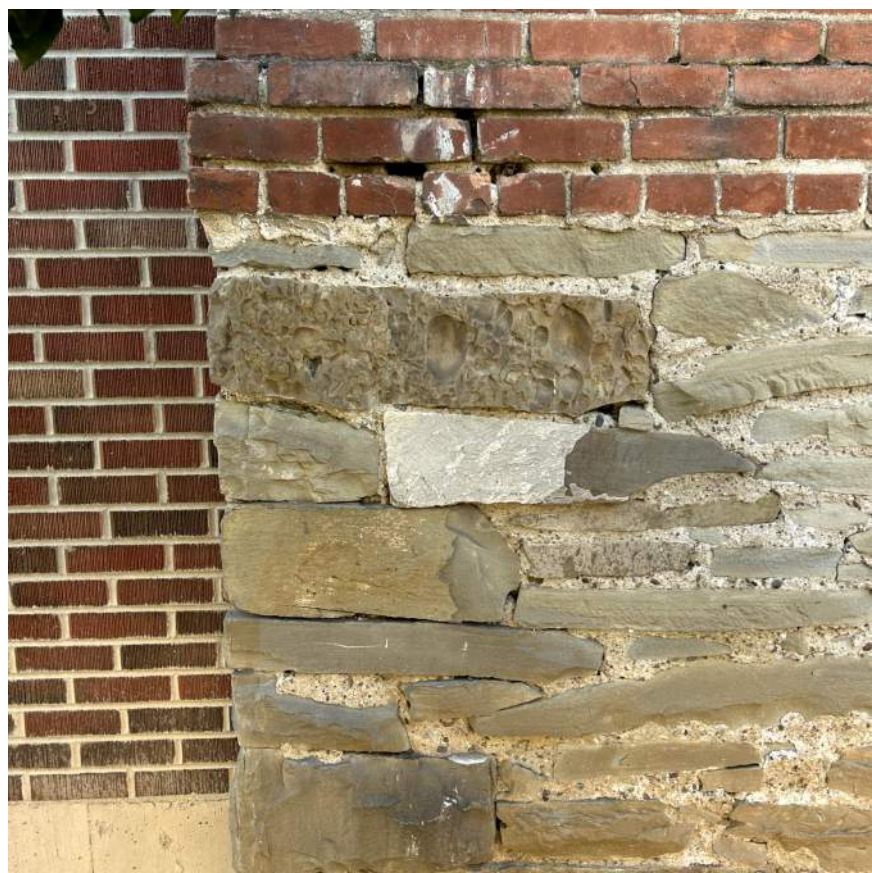
6 NORTH DETAIL PHOTO 6



4 NORTH DETAIL PHOTO 4



2 NORTH DETAIL PHOTO 2



14 NORTH DETAIL PHOTO 14



12 NORTH DETAIL PHOTO 12



10 NORTH DETAIL PHOTO 10



7 NORTH DETAIL PHOTO 7



5 NORTH DETAIL PHOTO 5



3 NORTH DETAIL PHOTO 3

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REVIEWED BY: Checker		
REV.	DESCRIPTION	DATE



PPA#22-0522
A/E#00-0-00
CONSULTANT #2308

SHEET TITLE
EXISTING PHOTOS - NORTH
SHEET
A2.2a

DATE
03/10/25



GENERAL NEW CONSTRUCTION KEY NOTES

- 1 REMOVE EXISTING CONCRETE APRON IN ITS ENTIRETY. CLEAN SURFACE OF STONE FOUNDATION OF ALL REMAINING CONCRETE DEBRIS. REPOINT ALL MASONRY JOINTS IN THIS AREA.
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- 3 VERIFY STL ANGLE EXISTS IF NOT, PROVIDE STEEL LINTEL WITH AT LEAST 4" OF BEARING
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- 9 REMOVE PAINT SPLATTER FROM BRICK. SEE PHOTO 12/A2.1a

BRICK REPAIR KEY NOTES		AREA OUTLINE	REMARKS, SEE KEY
CODE	DESCRIPTION OF WORK		
B1-#	REPLACE BRICK UNITS DAMAGED BY EXISTING FASTENERS. REMOVE ANY ANCHORS THAT MAY STILL EXIST		1,3,4
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B3-#	REPLACE SPALLED / DAMAGED / MISSING BRICK @ WINDOW ARCH		1,3,4
B4-# SF	REBUILD OUTER WYTHE OF WALL WITH MATCHING BRICK		2,5,8
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B8-# SF	REBUILD WITH EXISTING BRICK.		2,8
B8-ARC	REBUILD BRICK ARCH WITH EXISTING BRICK. RE-CAULK JOINT BETWEEN THE INTRADOS AND WINDOW HEAD		8
B8-BC	REBUILD BELT COURSE WITH EXISTING BRICK.		8
B9-# SF	REMOVE EXISTING CONCRETE FROM STONE MASONRY		2

REPOINTING KEY NOTES		AREA OUTLINE	REMARKS, SEE KEY
CODE	DESCRIPTION OF WORK		
REP-# SF	REPOINT MORTAR JOINTS		2,6
REP-ARC	REPOINT ALL MORTAR JOINTS ALONG BRICK ARCH INCLUDING JOINTS ALONG THE INTRADOS AND SKYWARD JOINTS		6,7
REP-BC#	REPOINT MORTAR JOINTS ALONG BELT COURSE INCLUDING ALL SKYWARD JOINTS		6,7

SIDING, SOFFIT AND FASCIA REPAIR KEY NOTES		AREA OUTLINE	REMARKS, SEE KEY
CODE	DESCRIPTION OF WORK		
P-S	RE-PAINT EXISTING SIDING		
P-SO	RE-PAINT EXISTING SOFFIT		
P-F	RE-PAINT EXISTING FASCIA		
P-T	RE-PAINT EXISTING TRIM WORK		

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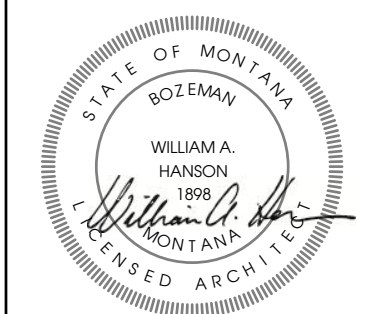


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REVIEWED BY: Checker		
REV.	DESCRIPTION	DATE



PPA#22-0522
A/E#00-0-00
CONSULTANT #2308

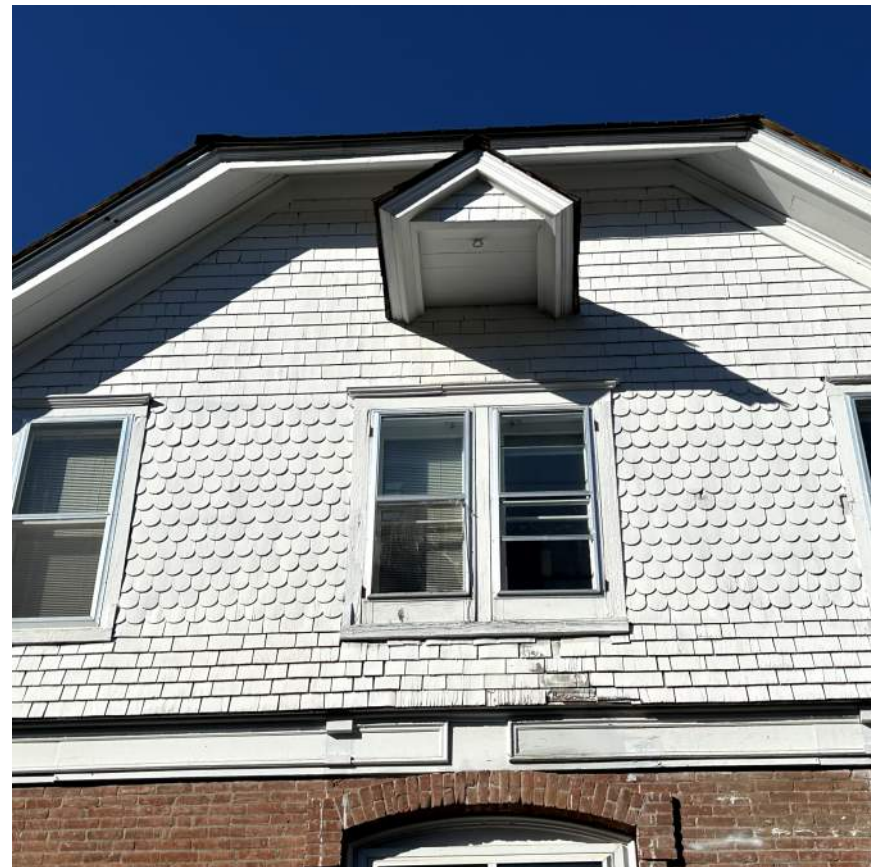
SHEET TITLE
EXTERIOR
ELEVATION - E
SHEET
A2.3

DATE
03/10/25

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12 EAST DETAIL PHOTO 12
A2.3a 1 1/2" = 1'-0"



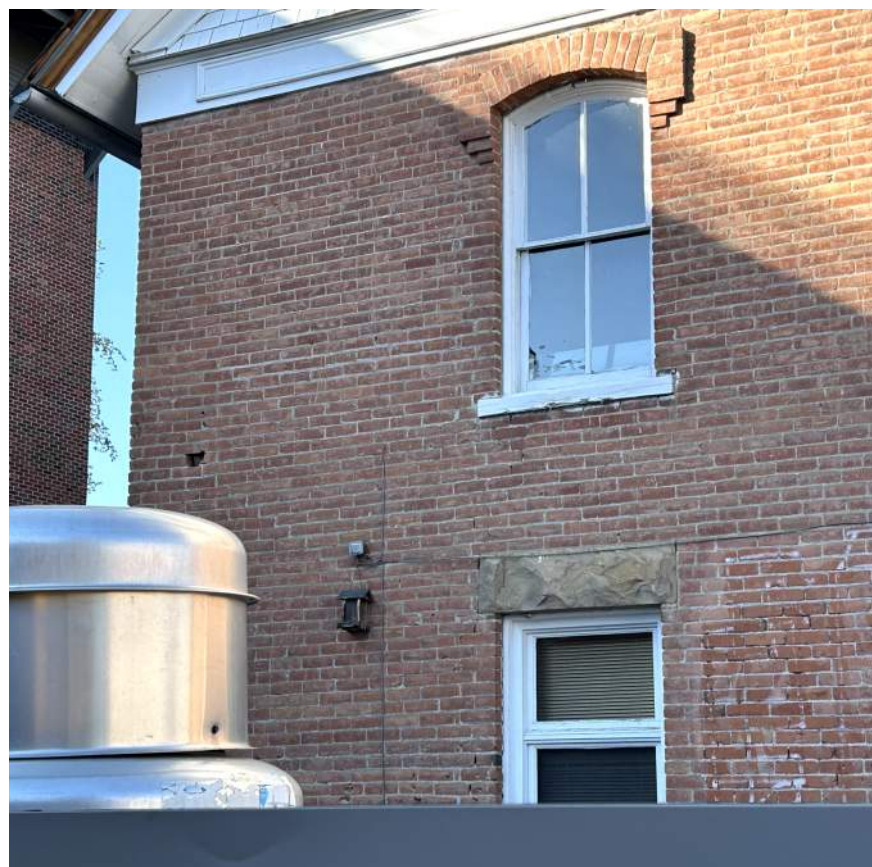
9 EAST DETAIL PHOTO 9
A2.3a 1 1/2" = 1'-0"



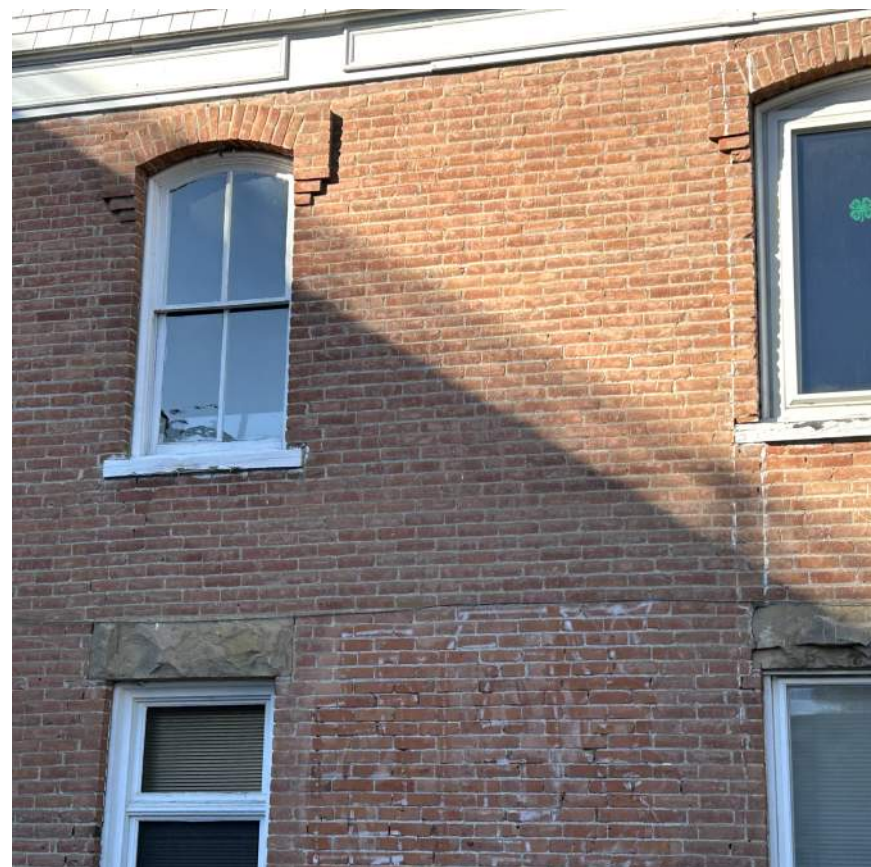
6 EAST DETAIL PHOTO 6
A2.3a 1 1/2" = 1'-0"



1 EAST ELEVATION PHOTO
A2.3a



13 EAST DETAIL PHOTO 13
A2.3a 1 1/2" = 1'-0"



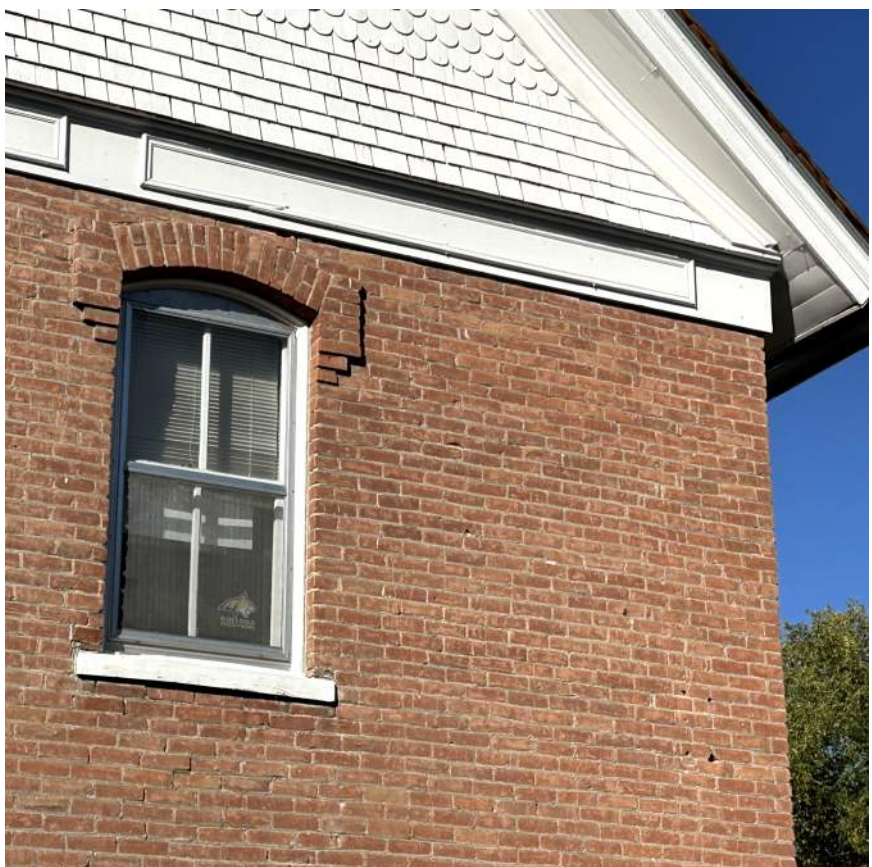
10 EAST DETAIL PHOTO 10
A2.3a 1 1/2" = 1'-0"



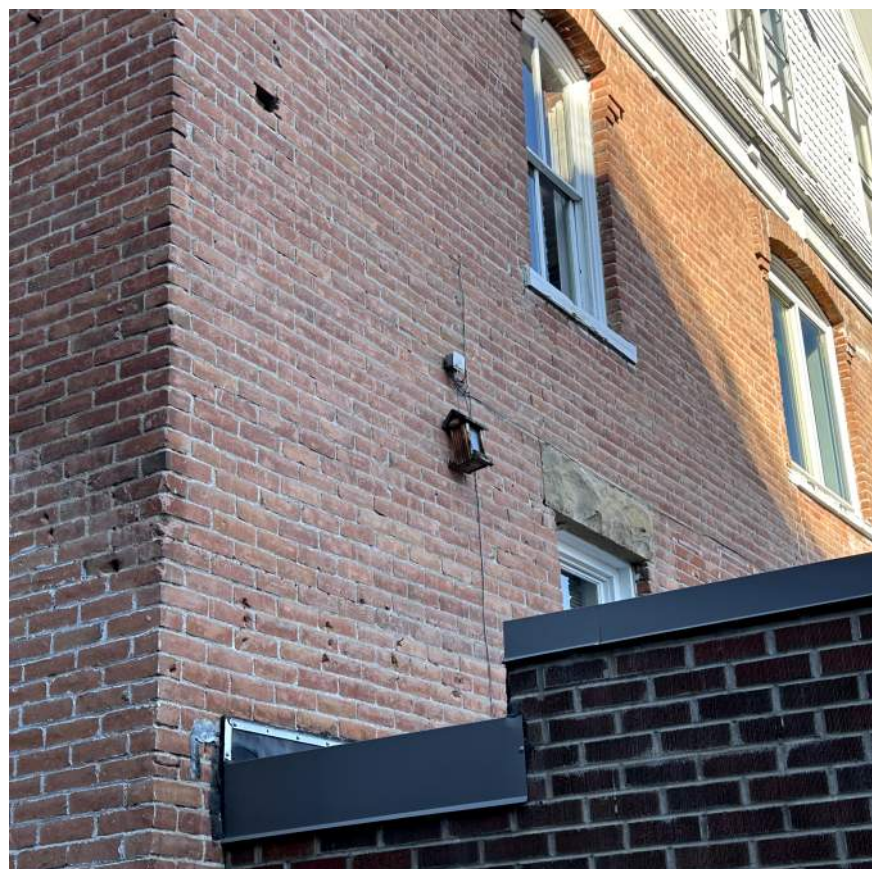
7 EAST DETAIL PHOTO 7
A2.3a 1 1/2" = 1'-0"



4 EAST DETAIL PHOTO 4
A2.3a 1 1/2" = 1'-0"



2 EAST DETAIL PHOTO 2
A2.3a 1 1/2" = 1'-0"



14 EAST DETAIL PHOTO 14
A2.3a 1 1/2" = 1'-0"



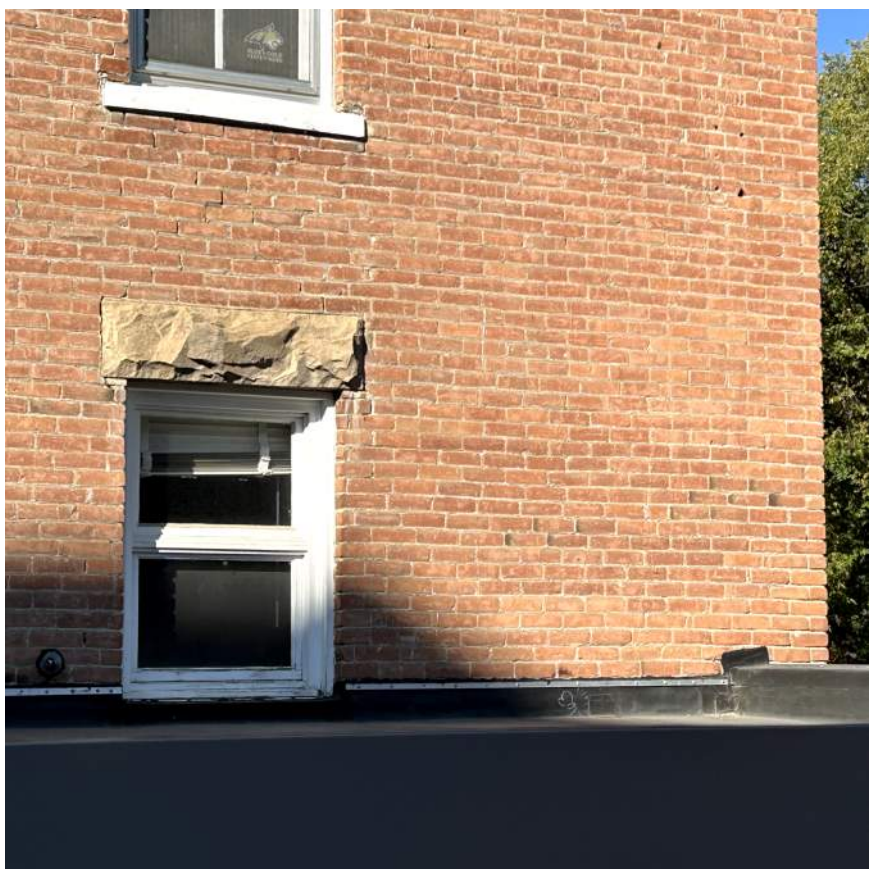
11 EAST DETAIL PHOTO 11
A2.3a 1 1/2" = 1'-0"



8 EAST DETAIL PHOTO 8
A2.3a 1 1/2" = 1'-0"



5 EAST DETAIL PHOTO 5
A2.3a 1 1/2" = 1'-0"



3 EAST DETAIL PHOTO 3
A2.3a 1 1/2" = 1'-0"

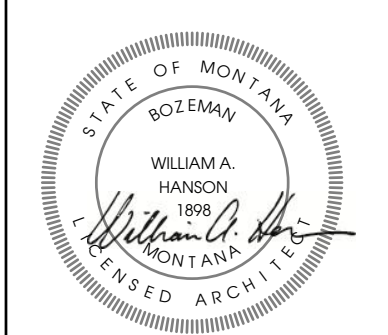


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REV.	DESCRIPTION	DATE



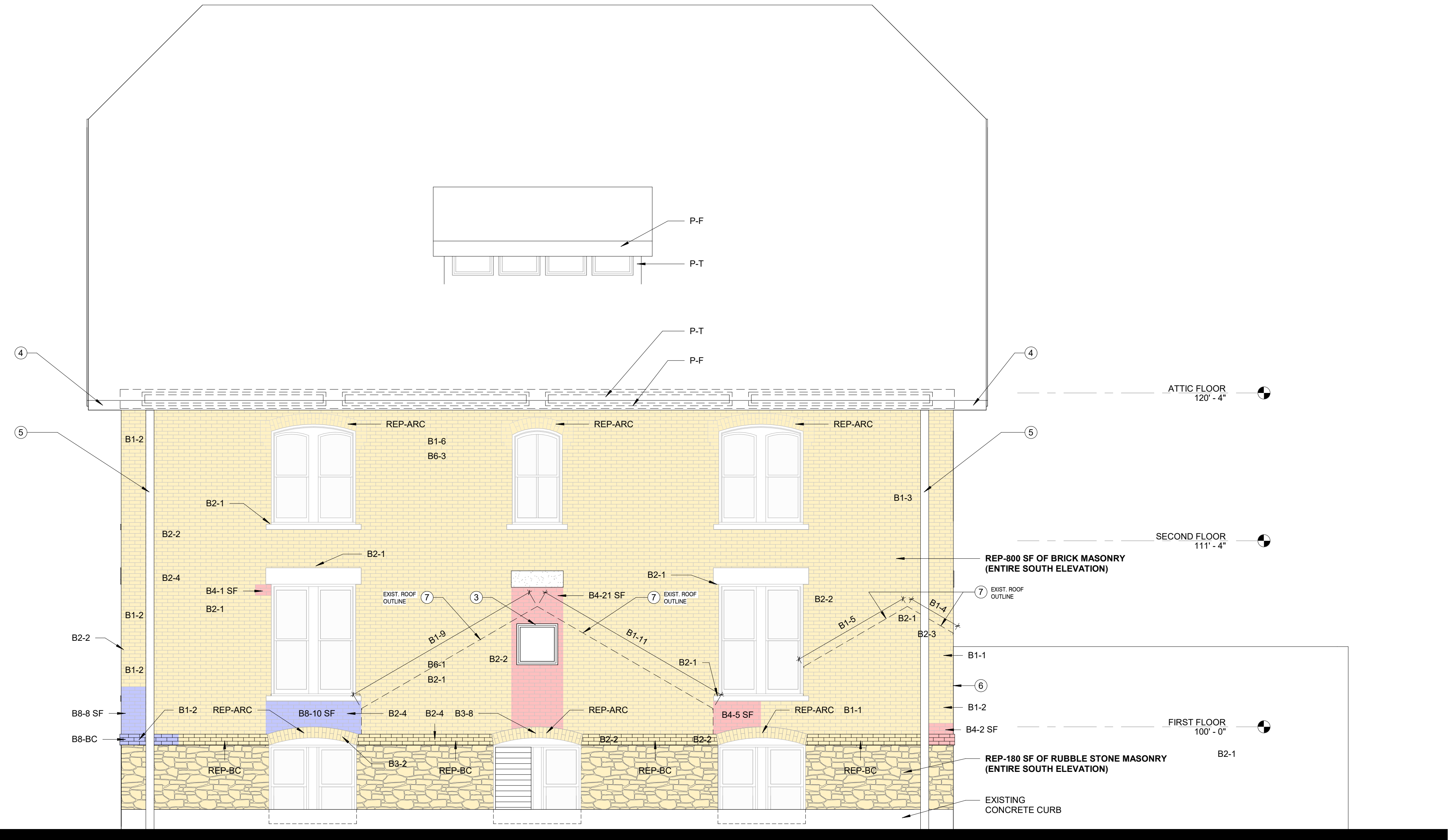
PPA#22-0522
A/E#00-0-00
CONSULTANT #2308

SHEET TITLE
EXISTING
PHOTOS - EAST
SHEET
A2.3a

DATE
03/10/25

100% CONSTRUCTION DOCUMENTS

ALL WORK DESCRIBED ON THIS SHEET IS ALTERNATE #1 UNLESS SPECIFICALLY NOTED OTHERWISE



1 SOUTH ELEVATION
A2.4 1/4" = 1'-0"

GENERAL NEW CONSTRUCTION KEY NOTES

- 1 REMOVE EXISTING CONCRETE APRON IN ITS ENTIRETY. CLEAN SURFACE OF STONE FOUNDATION OF ALL REMAINING CONCRETE DEBRIS. REPOINT ALL MASONRY JOINTS IN THIS AREA.
- 2 PROVIDE BACKER ROD AND SEALANT AROUND PIPE PENETRATION
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BRICK REPAIR KEY NOTES		AREA OUTLINE	REMARKS, SEE KEY
CODE	DESCRIPTION OF WORK		
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REPOINTING KEY NOTES		AREA OUTLINE	REMARKS, SEE KEY
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SIDING, SOFFIT AND FASCIA REPAIR KEY NOTES		AREA OUTLINE	REMARKS, SEE KEY
CODE	DESCRIPTION OF WORK		
P-S	RE-PAINT EXISTING SIDING		
P-SO	RE-PAINT EXISTING SOFFIT		
P-F	RE-PAINT EXISTING FASCIA		
P-T	RE-PAINT EXISTING TRIM WORK		

- 1 # INDICATES NUMBER OF BRICK UNITS IN NEED OF REPLACEMENT.
- 2 # INDICATES SQUARE FOOTAGE OF INTENDED WORK
- 3 CONTRACTOR OPTION TO TURN BRICK AROUND IF BACKSIDE IS IN GOOD CONDITION.
- 4 ASSUME ALL JOINTS SURROUNDING REPLACED BRICK UNITS WILL BE REPOINTED
- 5 SALVAGE AS MANY REUSABLE BRICK UNITS AS POSSIBLE TO BE INCORPORATED INTO THE RE-BUILT AREA
- 6 REFER TO TYPICAL REPOINTING DETAIL ON A2.5
- 7 SKYWARD JOINTS ARE TO RECEIVE POLYURETHANE SEALANT. REFER TO SKYWARD JOINT REPOINTING DETAIL ON A2.5.
- 8 THE CONTRACTOR IS RESPONSIBLE FOR ADEQUATELY SUPPORTING MASONRY ABOVE ANY PORTIONS THAT ARE TO BE REBUILT.



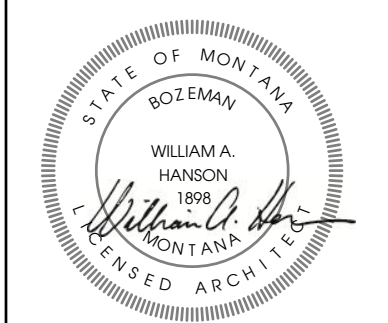
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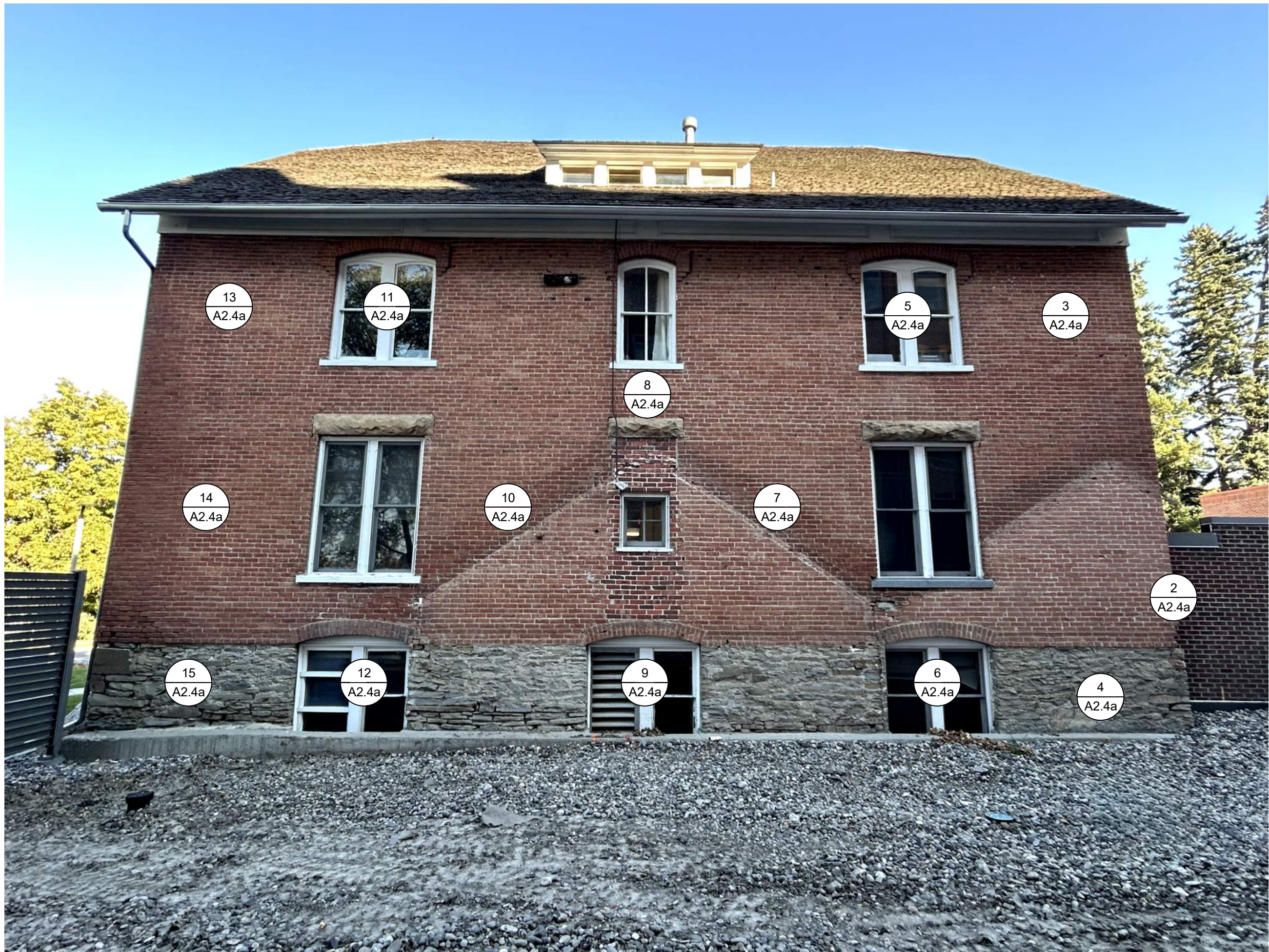


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CONSULTANT #2308

SHEET TITLE
EXTERIOR ELEVATION - S
SHEET
A2.4

DATE
03/10/25

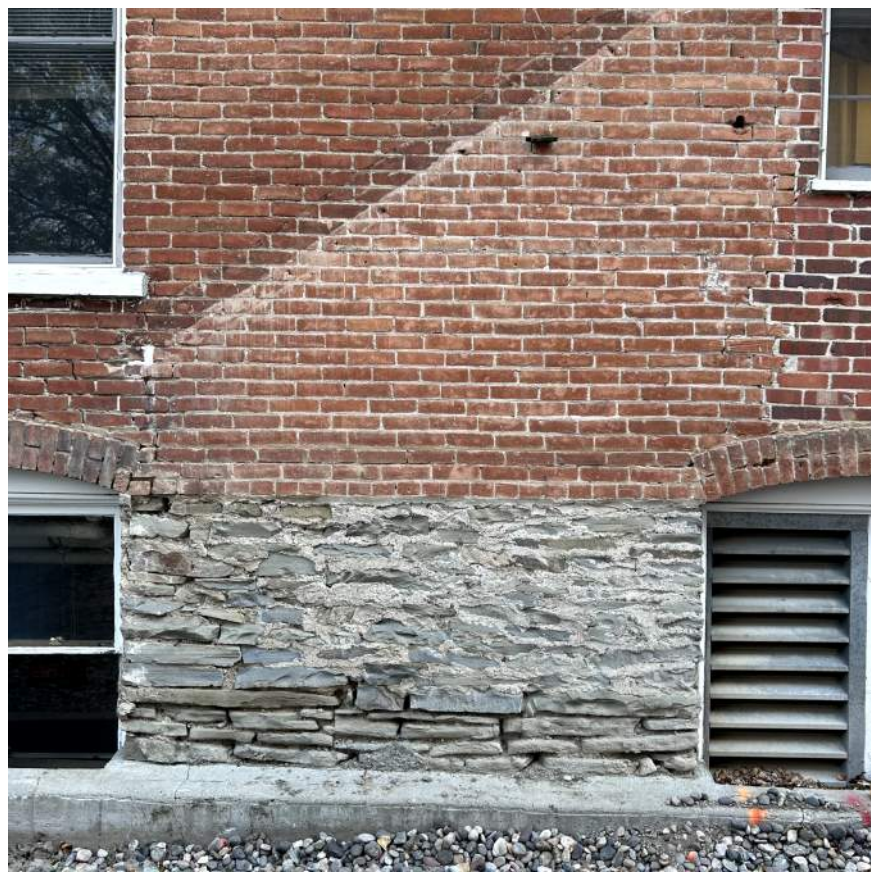
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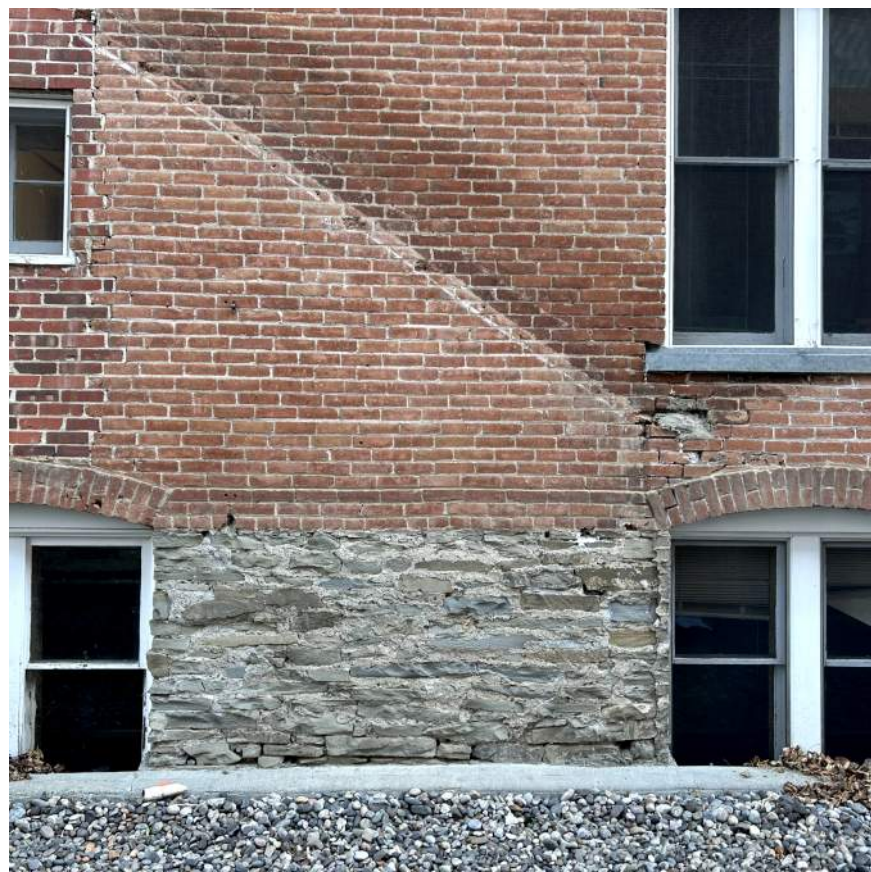
1 SOUTH ELEVATION PHOTO
A2.4a 1 1/2" = 1'-0"



13 SOUTH DETAIL PHOTO 13
A2.4a 1 1/2" = 1'-0"



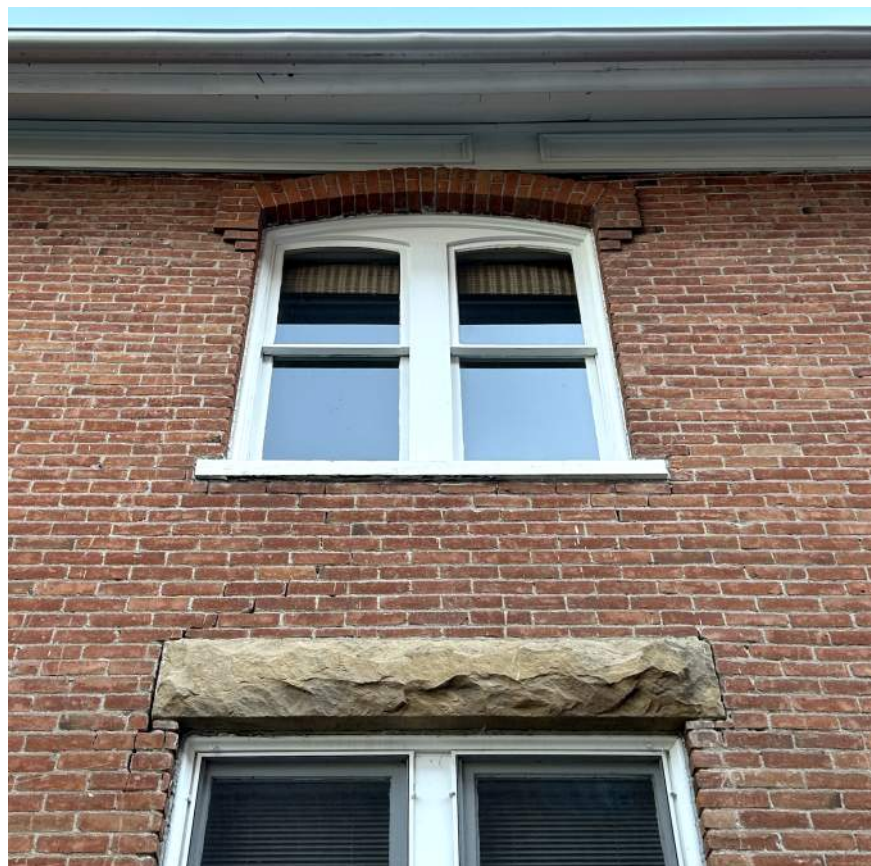
10 SOUTH DETAIL PHOTO 10
A2.4a 1 1/2" = 1'-0"



7 SOUTH DETAIL PHOTO 7
A2.4a 1 1/2" = 1'-0"



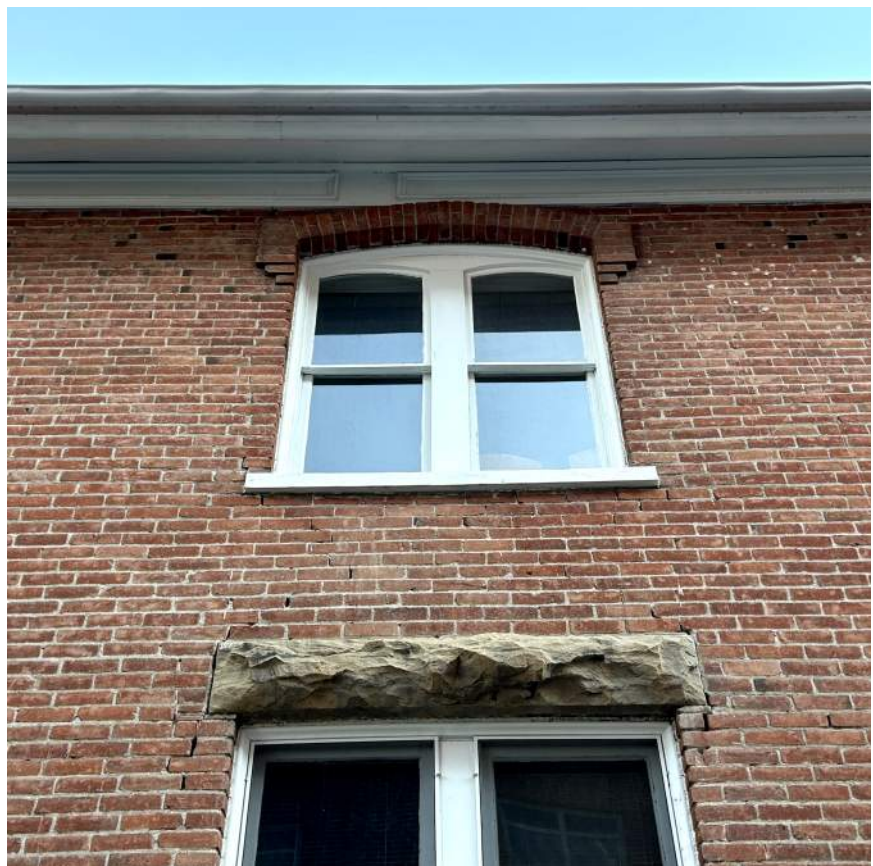
14 SOUTH DETAIL PHOTO 14
A2.4a 1 1/2" = 1'-0"



11 SOUTH DETAIL PHOTO 11
A2.4a 1 1/2" = 1'-0"



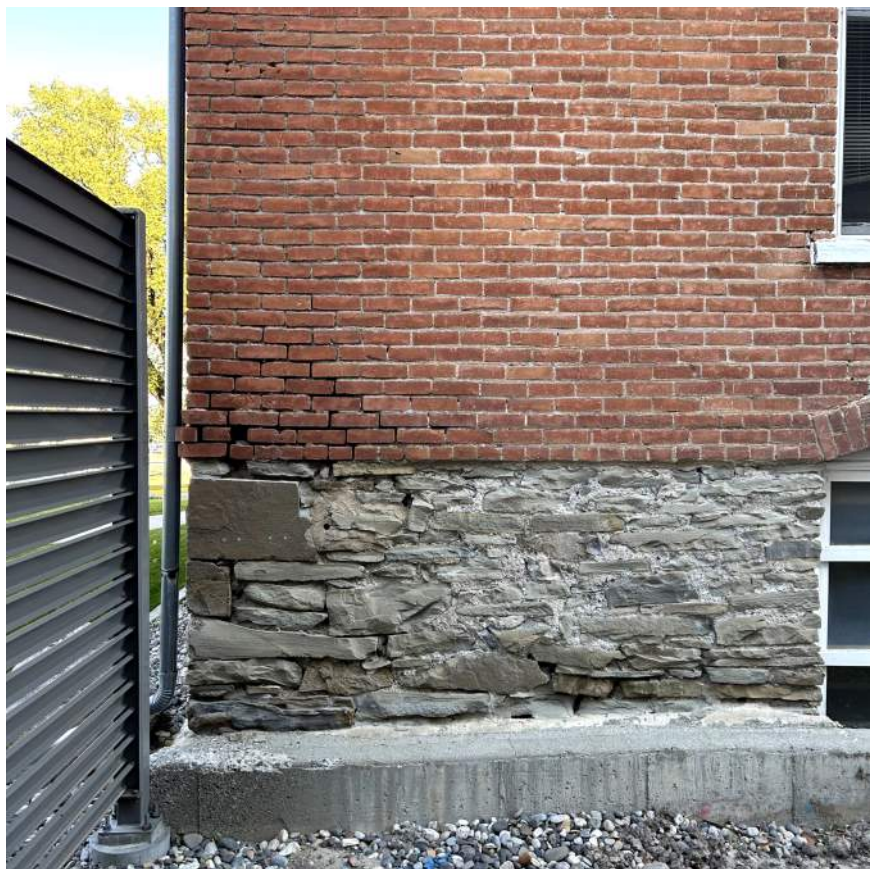
8 SOUTH DETAIL PHOTO 8
A2.4a 1 1/2" = 1'-0"



5 SOUTH DETAIL PHOTO 5
A2.4a 1 1/2" = 1'-0"



3 SOUTH DETAIL PHOTO 3
A2.4a 1 1/2" = 1'-0"



15 SOUTH DETAIL PHOTO 15
A2.4a 1 1/2" = 1'-0"



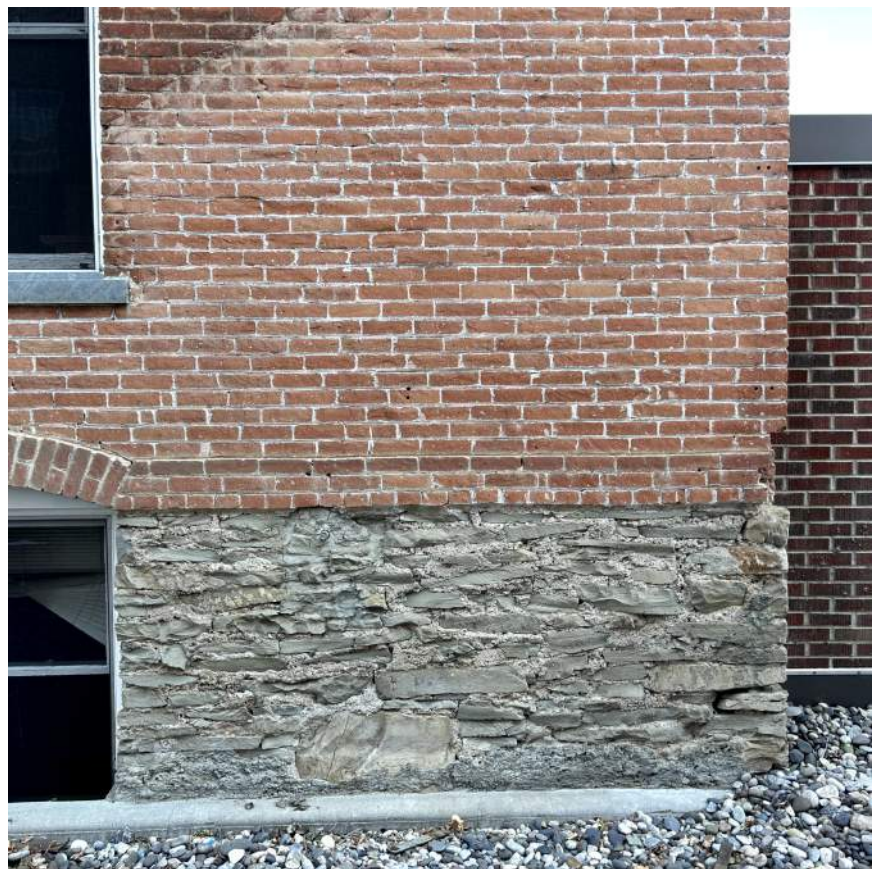
12 SOUTH DETAIL PHOTO 12
A2.4a 1 1/2" = 1'-0"



9 SOUTH DETAIL PHOTO 9
A2.4a 1 1/2" = 1'-0"



6 SOUTH DETAIL PHOTO 6
A2.4a 1 1/2" = 1'-0"



4 SOUTH DETAIL PHOTO 4
A2.4a 1 1/2" = 1'-0"



2 SOUTH DETAIL PHOTO 2
A2.4a 1 1/2" = 1'-0"

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SHEET TITLE
EXISTING PHOTOS - SOUTH
SHEET
A2.4a

DATE
03/10/25

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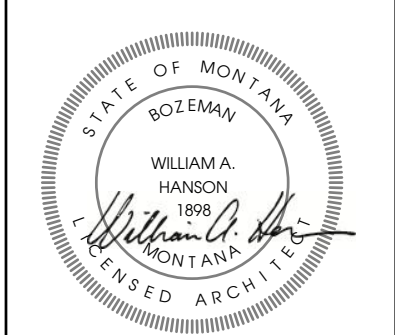
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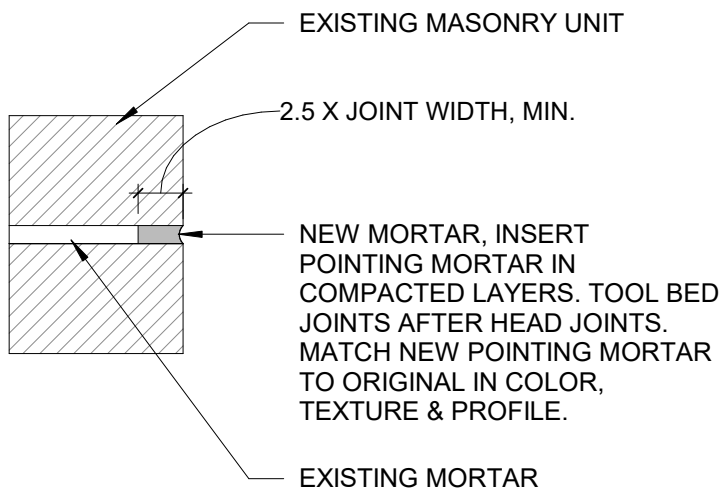


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SHEET TITLE
DETAILS

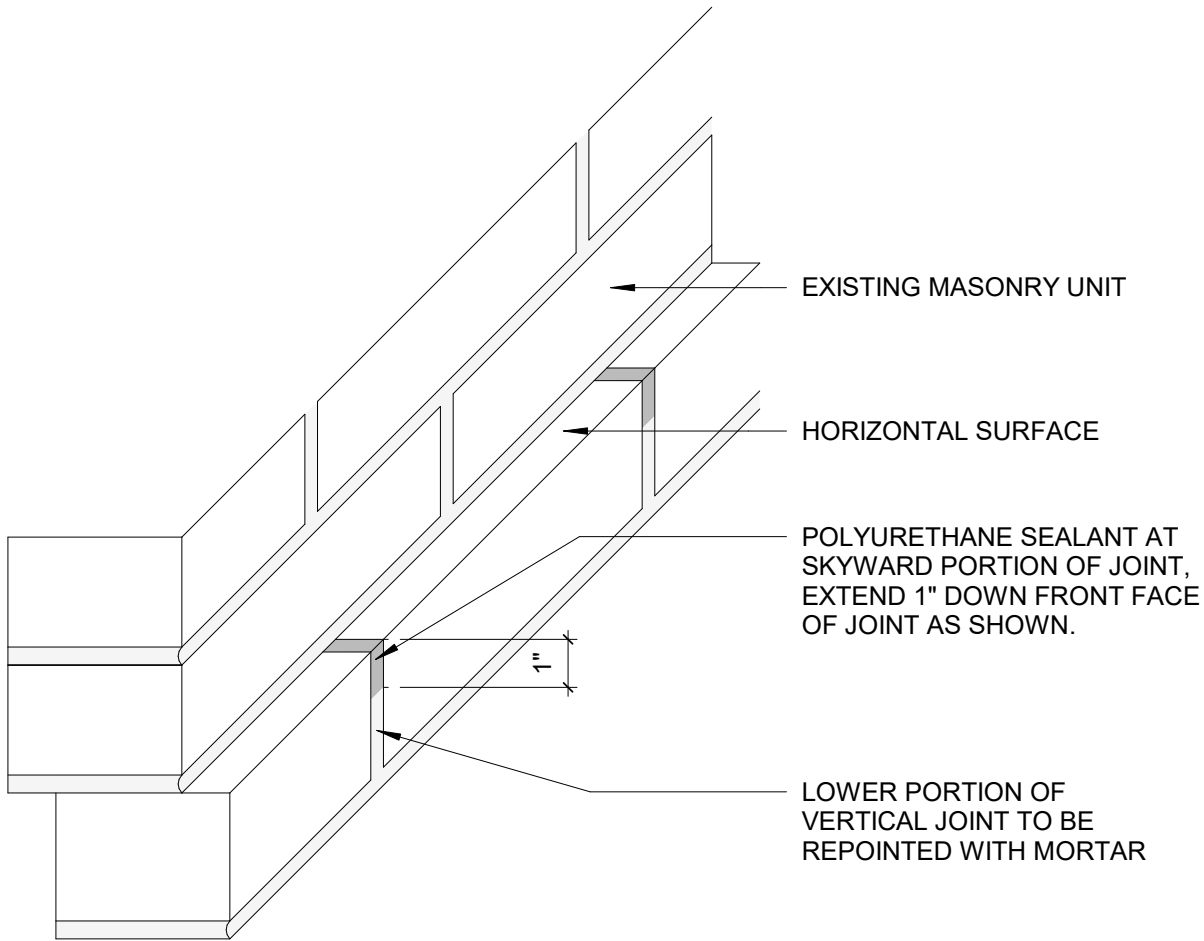
SHEET
A2.5

DATE
03/10/25



- GENERAL NOTES:
1. REPOINTING INCLUDES REMOVAL OF SEALANT
2. REMOVE ALL JOINT MORTAR, SEALANT AND FILLER MATERIAL
3. THE REMOVAL PROCESS SHALL NOT DAMAGE EXISTING MASONRY. OVER CUTTING AND WIDENING OF JOINTS WILL NOT BE ACCEPTED.
4. REMOVAL DEVICES SHALL BE HELD IN STRICT ALIGNMENT WITH JOINT & NOT AT AN ANGLE.
5. CONTRACTOR SHALL PROVIDE SAMPLE MOCK-UP FOR ARCHITECTS ACCEPTANCE BEFORE PROCEEDING WITH WORK.

2 TYPICAL REPOINTING DETAIL
A2.5 3" = 1'-0"



- GENERAL NOTES:
1. REPOINTING INCLUDES REMOVAL OF SEALANT
2. REMOVE ALL JOINT MORTAR, SEALANT AND FILLER MATERIAL
3. THE REMOVAL PROCESS SHALL NOT DAMAGE EXISTING MASONRY. OVER CUTTING AND WIDENING OF JOINTS WILL NOT BE ACCEPTED.
4. REMOVAL DEVICES SHALL BE HELD IN STRICT ALIGNMENT WITH JOINT & NOT AT AN ANGLE.
5. CONTRACTOR SHALL PROVIDE SAMPLE MOCK-UP FOR ARCHITECTS ACCEPTANCE BEFORE PROCEEDING WITH WORK.

1 SKYWARD JOINT DETAIL
A2.5 3" = 1'-0"



3 NORTH ELEVATION - WINDOW REPLACEMENT
A5.0 1/8" = 1'-0"



1 WEST ELEVATION - WINDOW REPLACEMENT
A5.0 1/8" = 1'-0"



4 SOUTH ELEVATION - WINDOW REPLACEMENT
A5.0 1/8" = 1'-0"



2 EAST ELEVATION - WINDOW REPLACEMENT
A5.0 1/8" = 1'-0"

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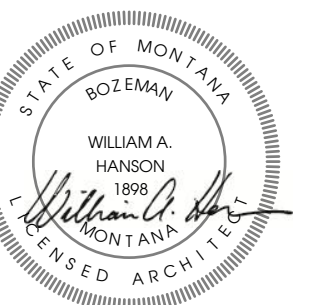


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SHEET TITLE
WINDOW
REPLACEMENT
ELEVATIONS

A5.0

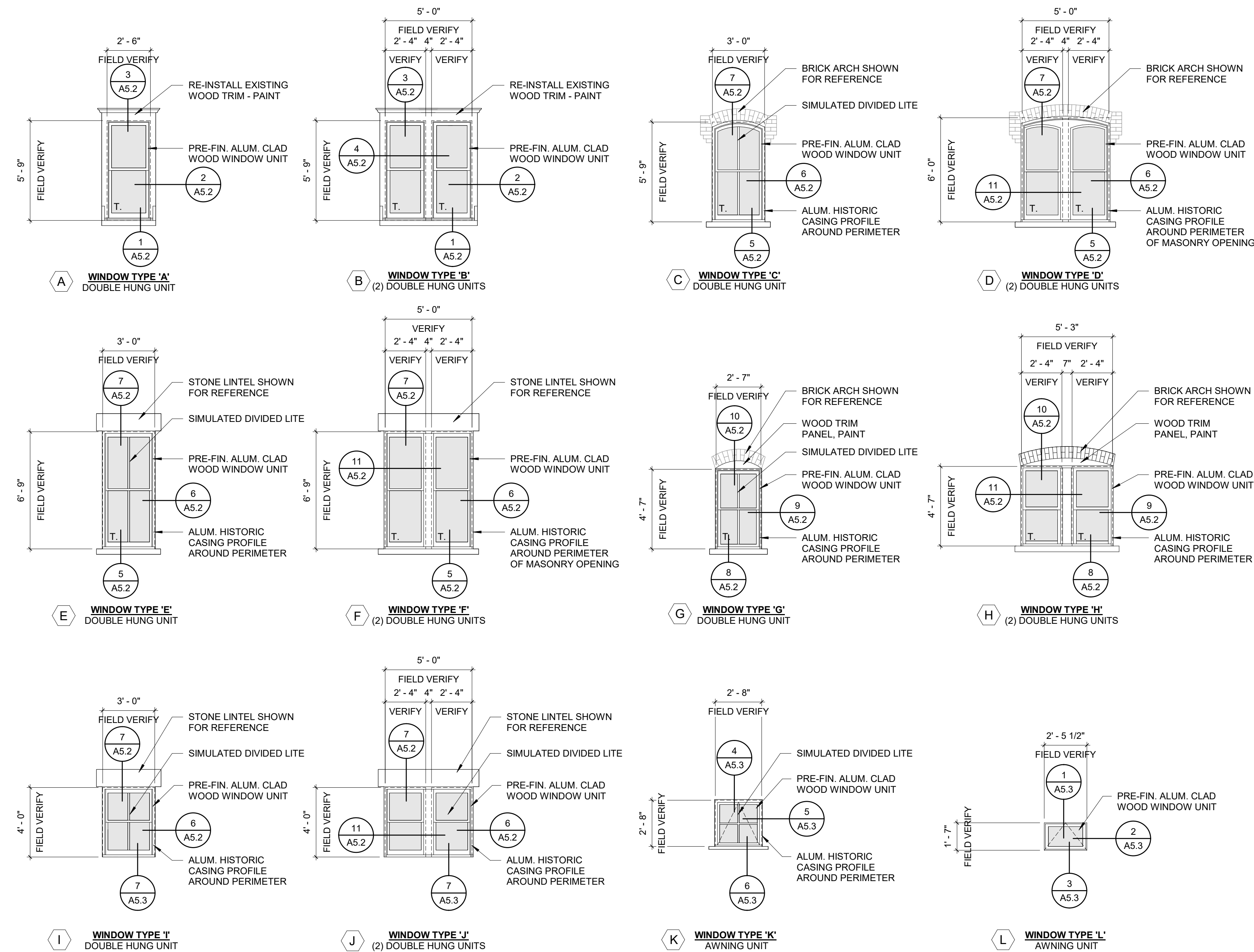
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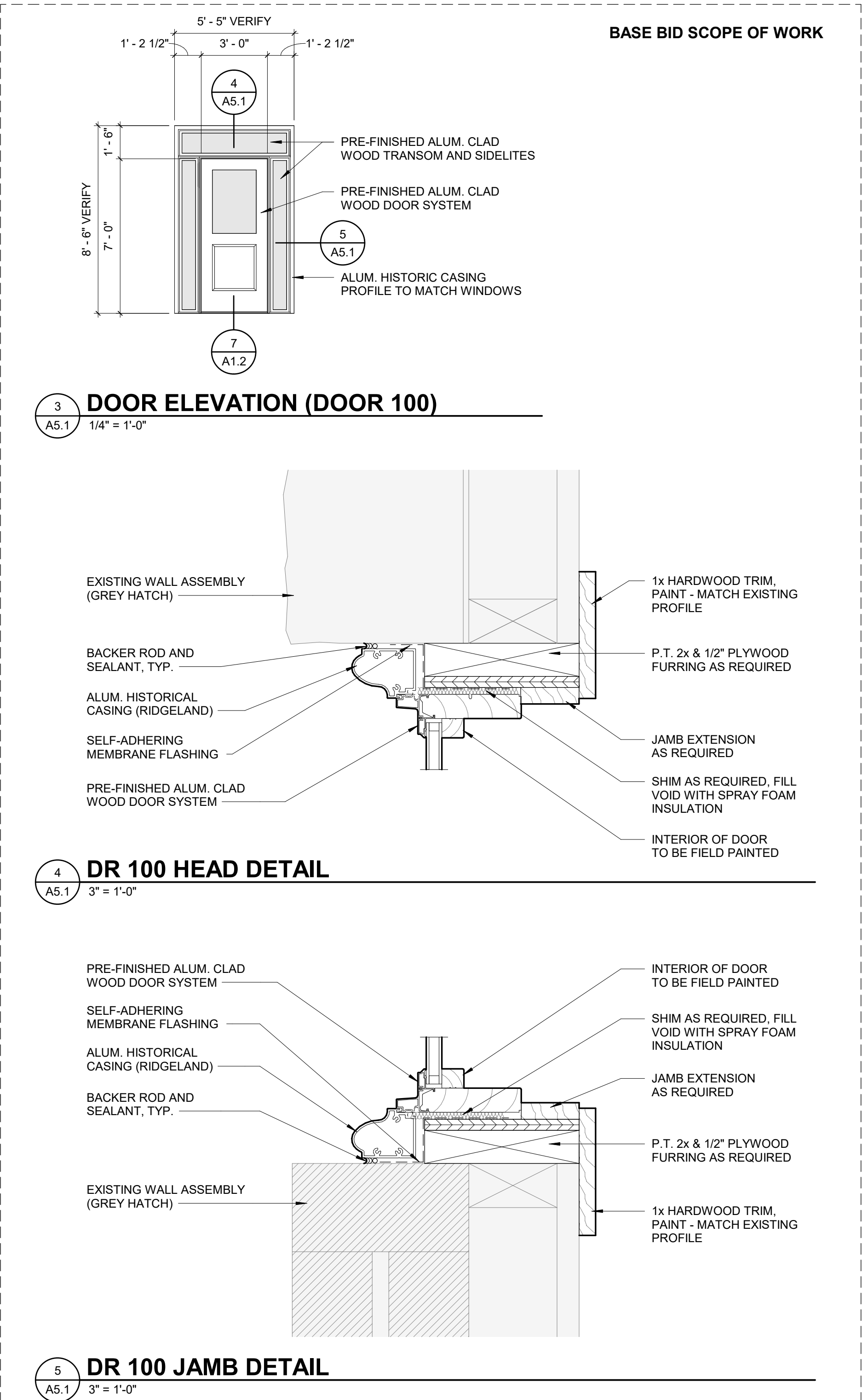
WINDOW SCHEDULE				
TYPE	SIZE	OPERATION	MATERIAL	REMARKS
A	RE: ELEV.	DOUBLE HUNG	EXT - PRE-FIN ALUM. INT - PRIMED, FIELD PAINT	
B	RE: ELEV.	(2X) DOULE HUNG	EXT - PRE-FIN ALUM. INT - PRIMED, FIELD PAINT	
C	RE: ELEV.	DOUBLE HUNG	EXT - PRE-FIN ALUM. INT - PRIMED, FIELD PAINT	EXTERIOR IS ARCH TOP, INTERIOR IS SQUARE TOP
D	RE: ELEV.	(2X) DOUBLE HUNG	EXT - PRE-FIN ALUM. INT - PRIMED, FIELD PAINT	EXTERIOR IS ARCH TOP, INTERIOR IS SQUARE TOP
E	RE: ELEV.	DOUBLE HUNG	EXT - PRE-FIN ALUM. INT - PRIMED, FIELD PAINT	
F	RE: ELEV.	(2X) DOUBLE HUNG	EXT - PRE-FIN ALUM. INT - PRIMED, FIELD PAINT	
G	RE: ELEV.	DOUBLE HUNG	EXT - PRE-FIN ALUM. INT - PRIMED, FIELD PAINT	
H	RE: ELEV.	(2X) DOUBLE HUNG	EXT - PRE-FIN ALUM. INT - PRIMED, FIELD PAINT	
I	RE: ELEV.	DOUBLE HUNG	EXT - PRE-FIN ALUM. INT - PRIMED, FIELD PAINT	
J	RE: ELEV.	(2X) DOUBLE HUNG	EXT - PRE-FIN ALUM. INT - PRIMED, FIELD PAINT	
K	RE: ELEV.	AWNING	EXT - PRE-FIN ALUM. INT - PRIMED, FIELD PAINT	
L	RE: ELEV.	AWNING	EXT - PRE-FIN ALUM. INT - PRIMED, FIELD PAINT	

NOTE: BASIS-OF-DESIGN FOR NEW WINDOWS IS
**MARVIN: ULTIMATE DOUBLE HUNG G2 WITH
HISTORIC CASING OPTION**

NOTE: ALL DIMENSIONS TO BE FIELD VERIFIED.



1 WINDOW TYPES
A5.1 1/4" = 1'-0"



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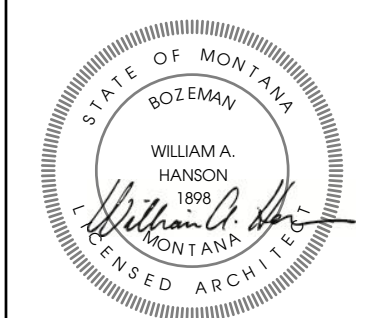
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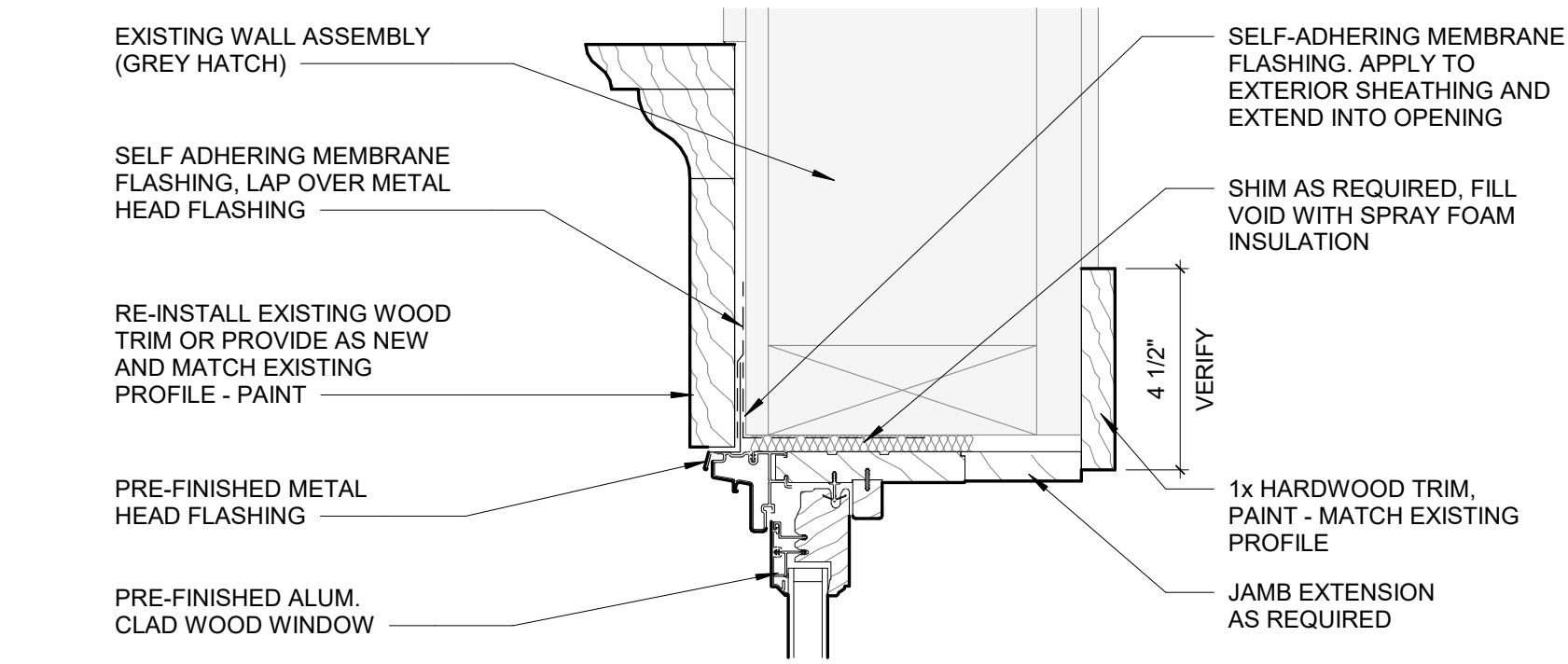


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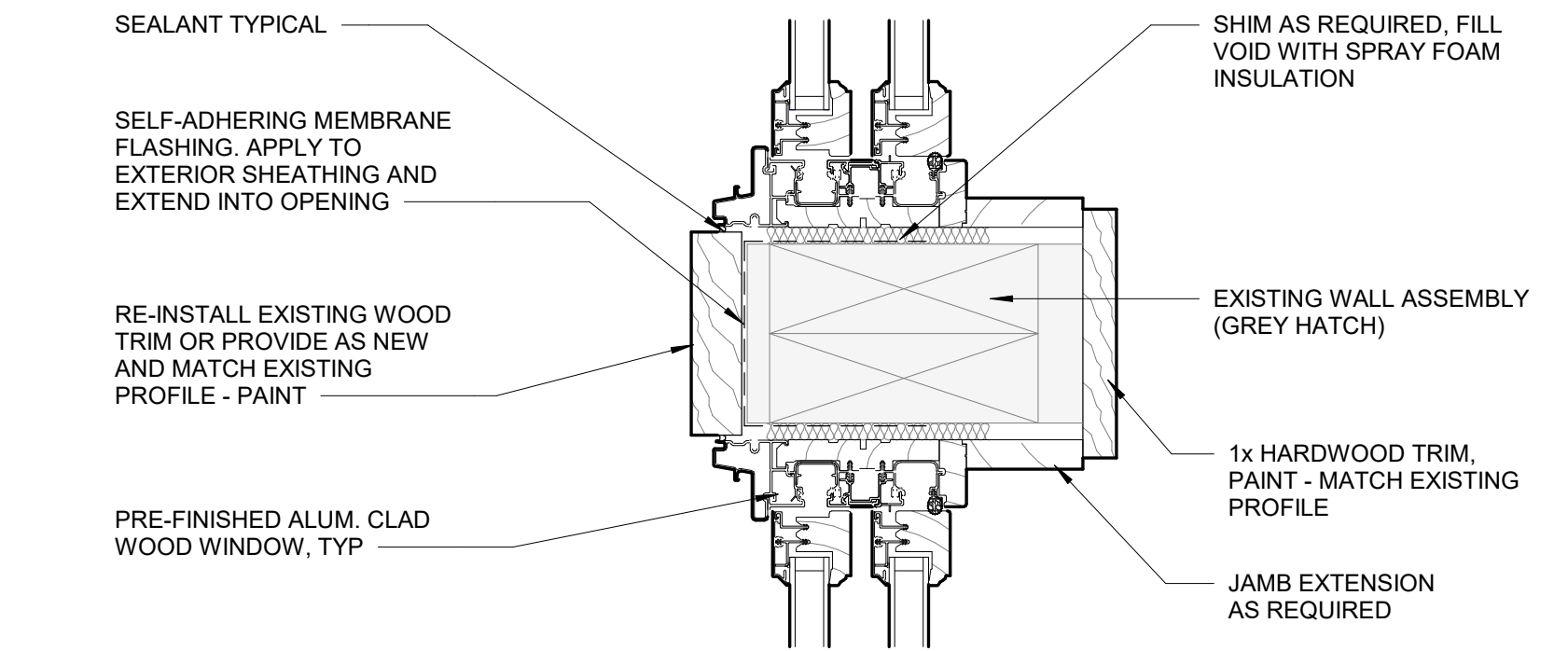
SHEET TITLE
**WINDOW &
DOOR SCHEDULE**

SHEET
A5.1

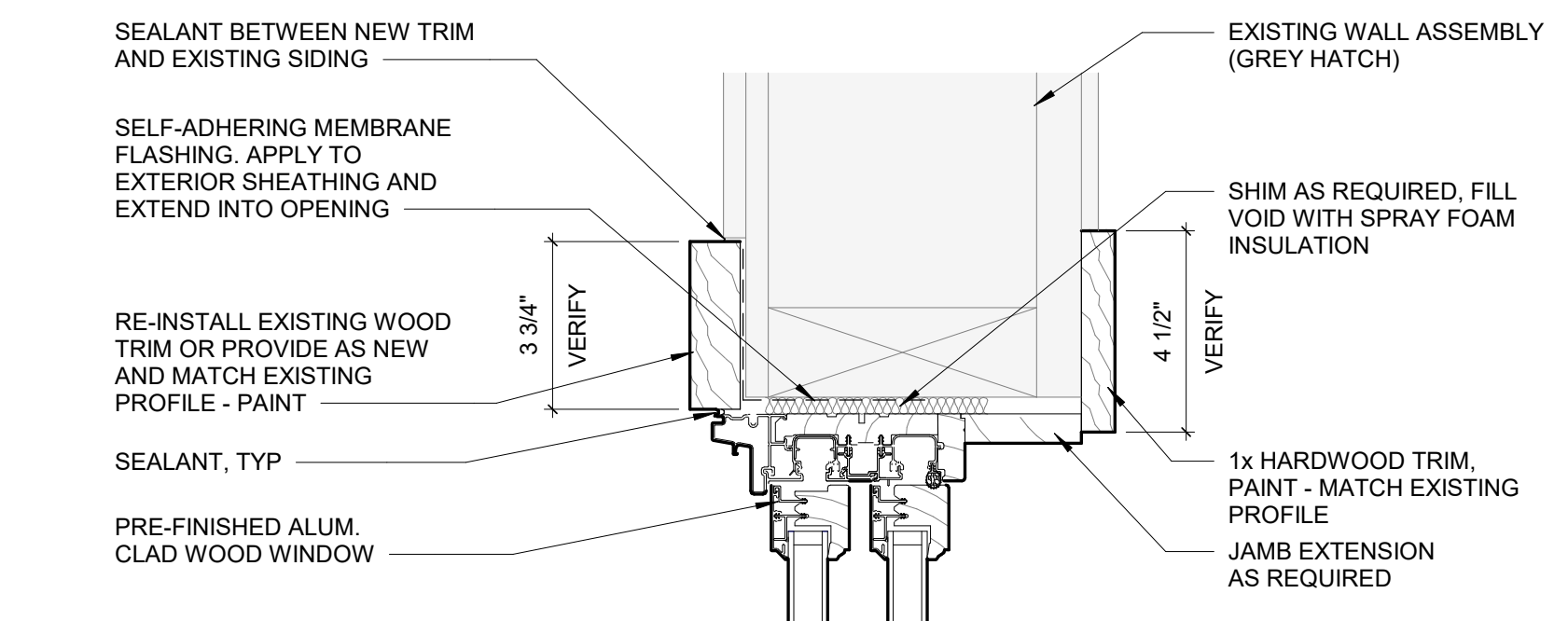
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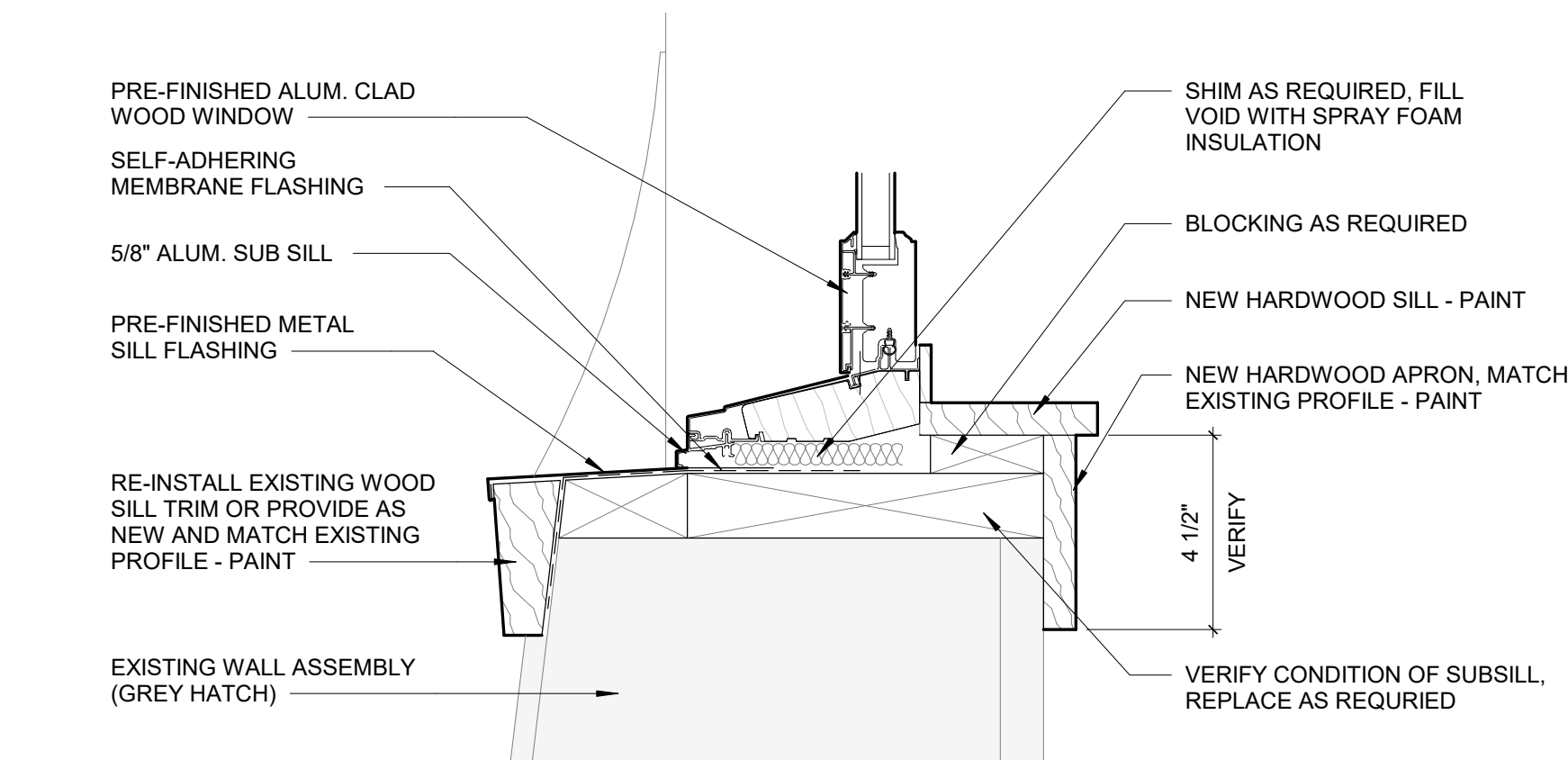
3
A5.2
WND 'A' & 'B' HEAD DETAIL
3" = 1'-0"



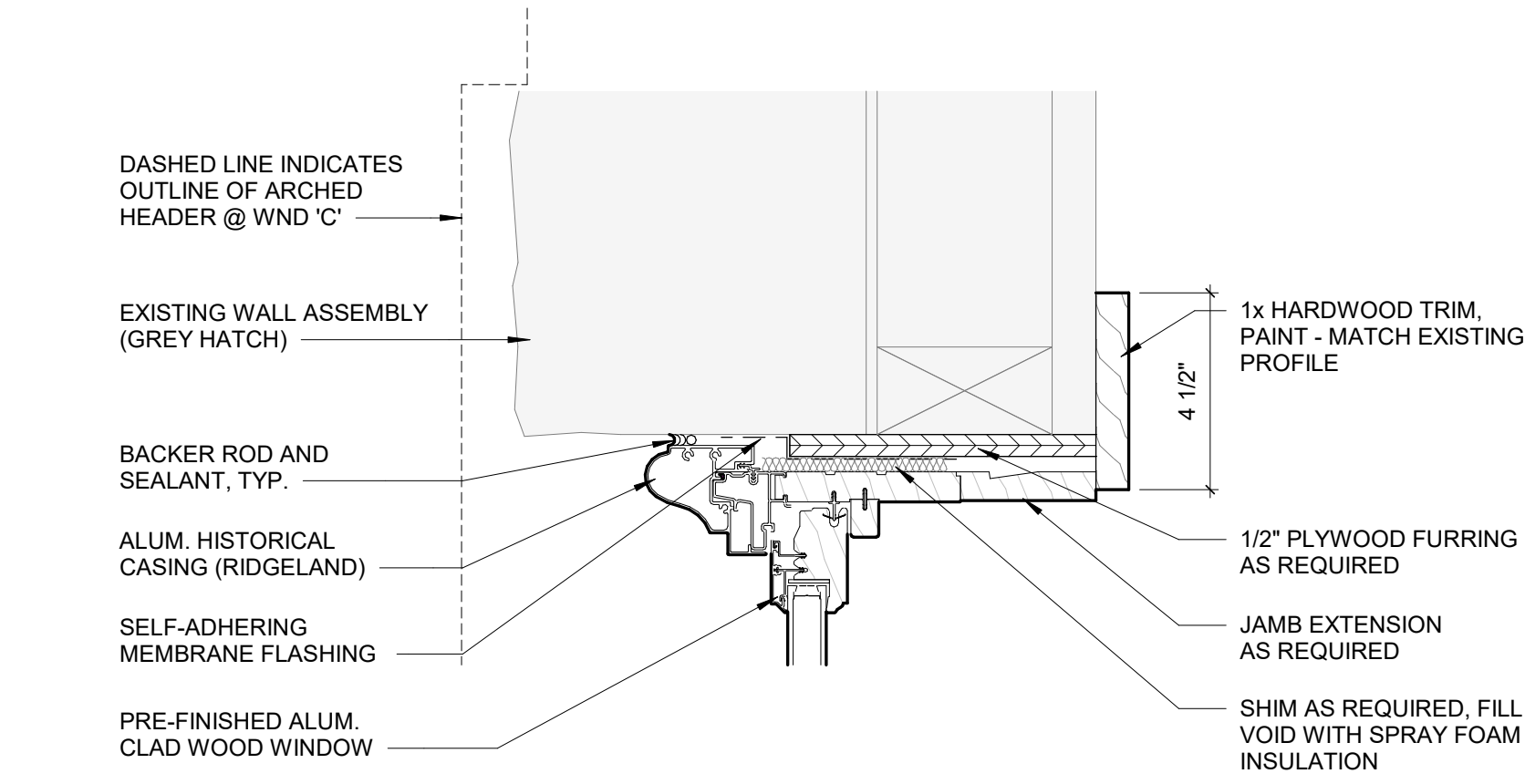
4
A5.2
WND 'B' MULL DETAIL
3" = 1'-0"



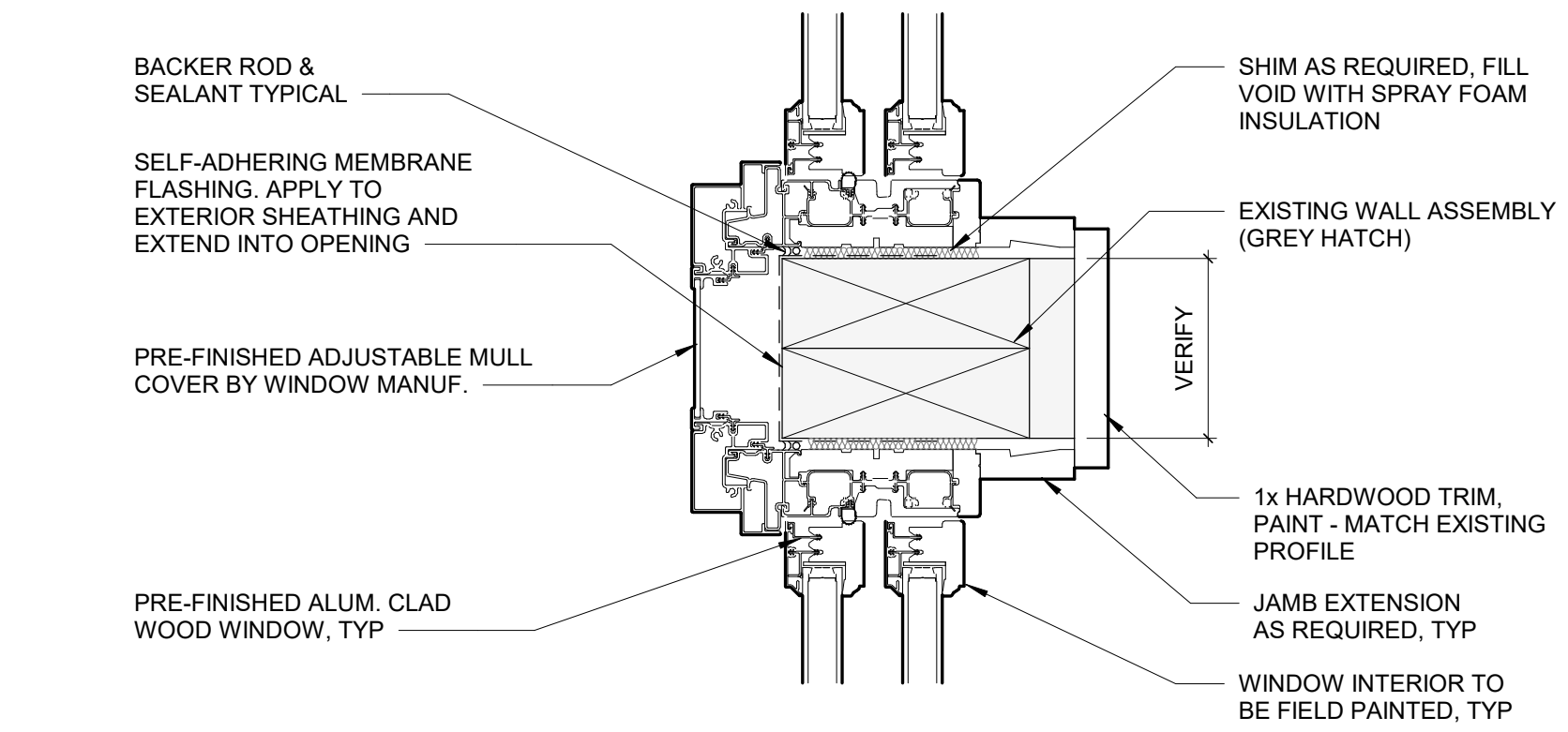
2
A5.2
WND 'A' & 'B' JAMB DETAIL
3" = 1'-0"



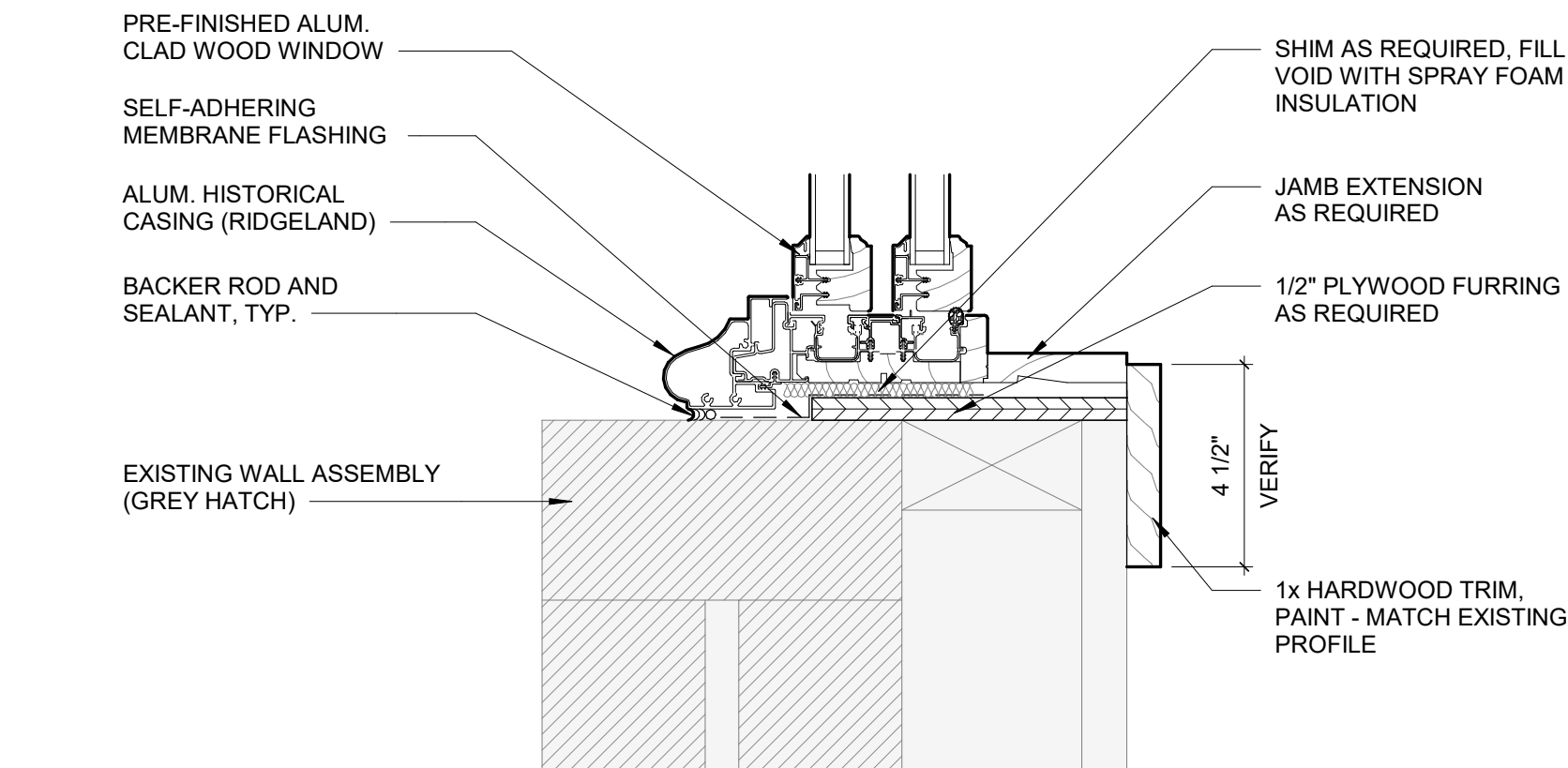
1
A5.2
WND 'A' & 'B' SILL DETAIL
3" = 1'-0"



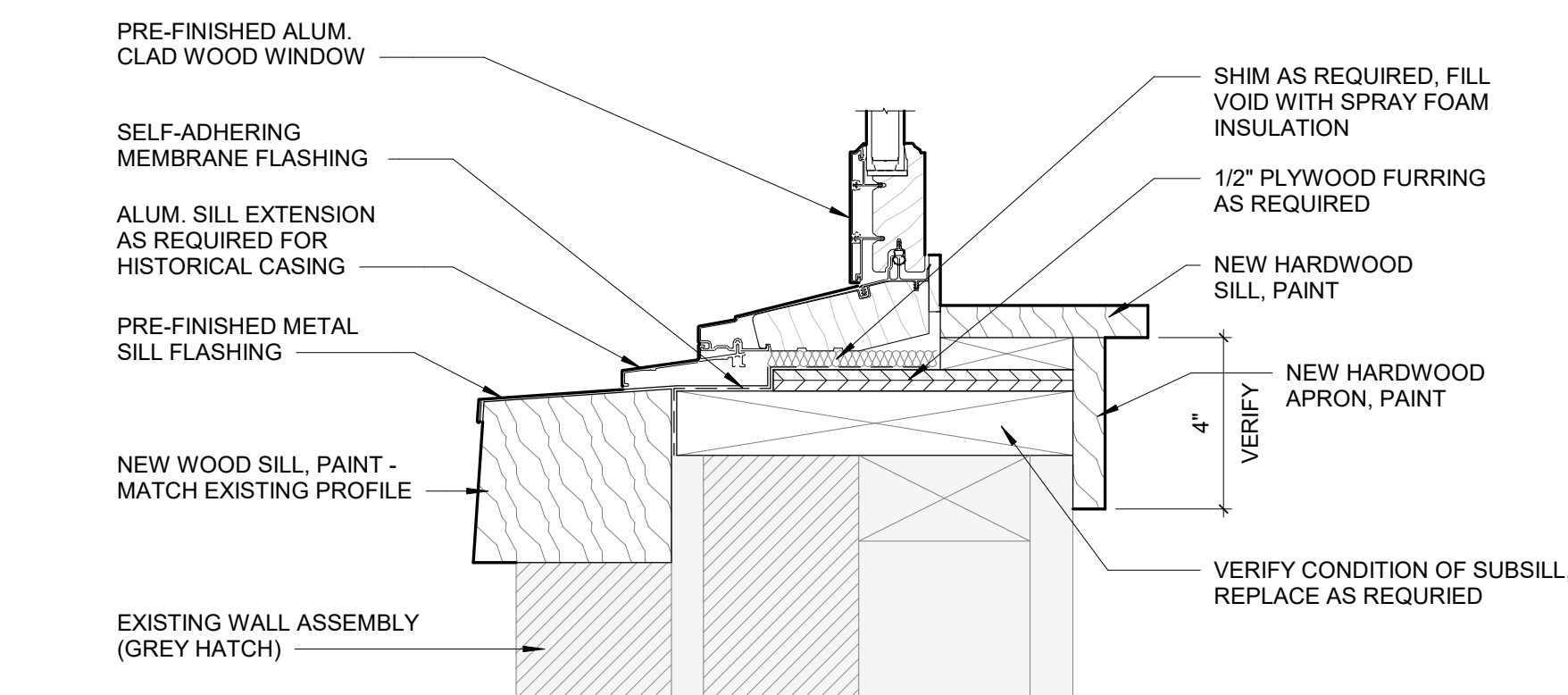
7
A5.2
WND 'C', 'D', 'E' & 'F' HEAD DETAIL
3" = 1'-0"



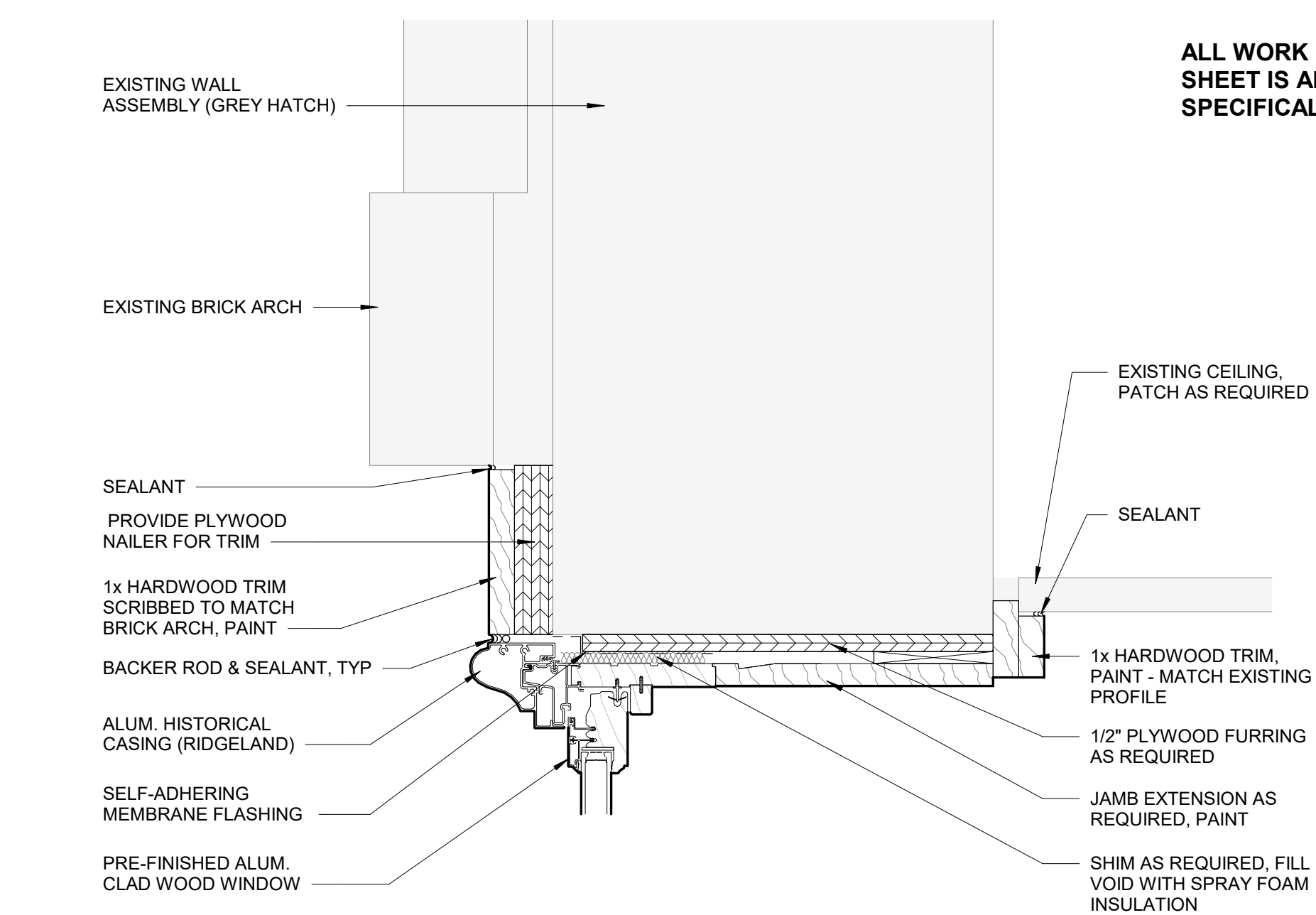
11
A5.2
WND 'D', 'F', 'H' & 'J' MULL DETAIL
3" = 1'-0"



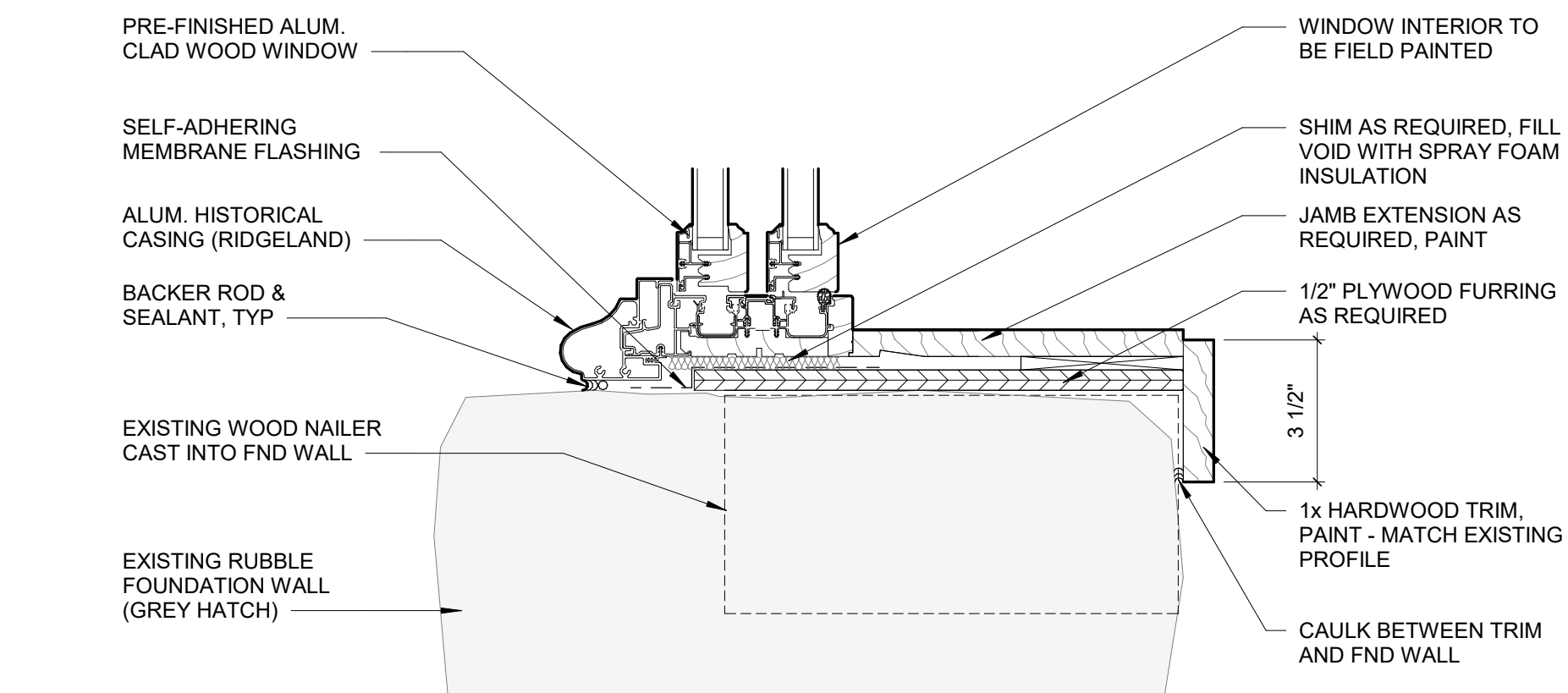
6
A5.2
WND 'C', 'D', 'E' & 'F' JAMB DETAIL
3" = 1'-0"



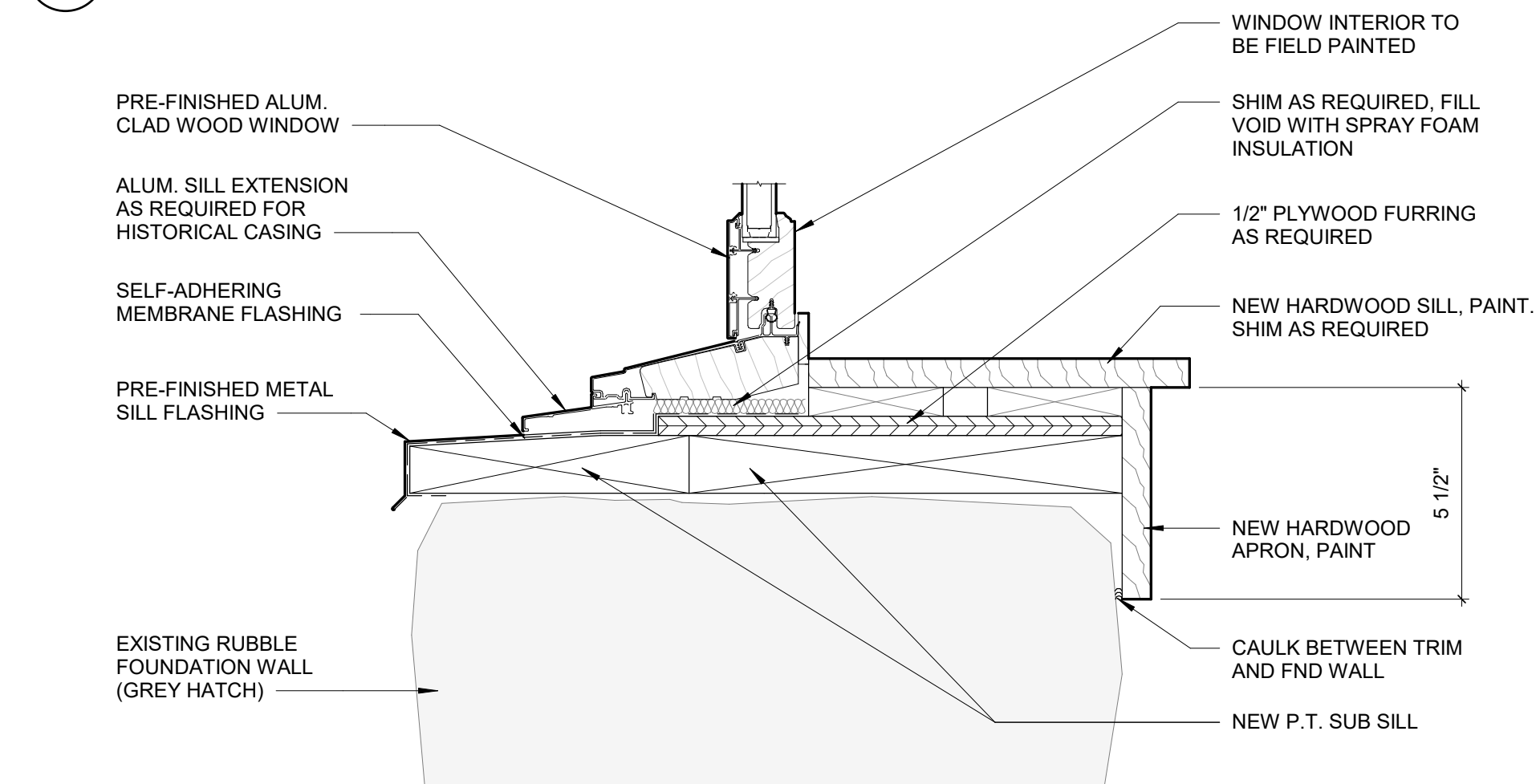
5
A5.2
WND 'C', 'D', 'E' & 'F' SILL DETAIL
3" = 1'-0"



10
A5.2
WND 'G' & 'H' HEAD DETAIL
3" = 1'-0"



9
A5.2
WND 'G' & 'H' JAMB DETAIL
3" = 1'-0"



8
A5.2
WND 'G' & 'H' SILL DETAIL
3" = 1'-0"

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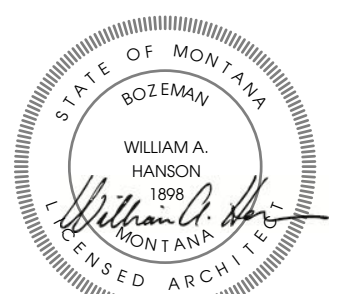


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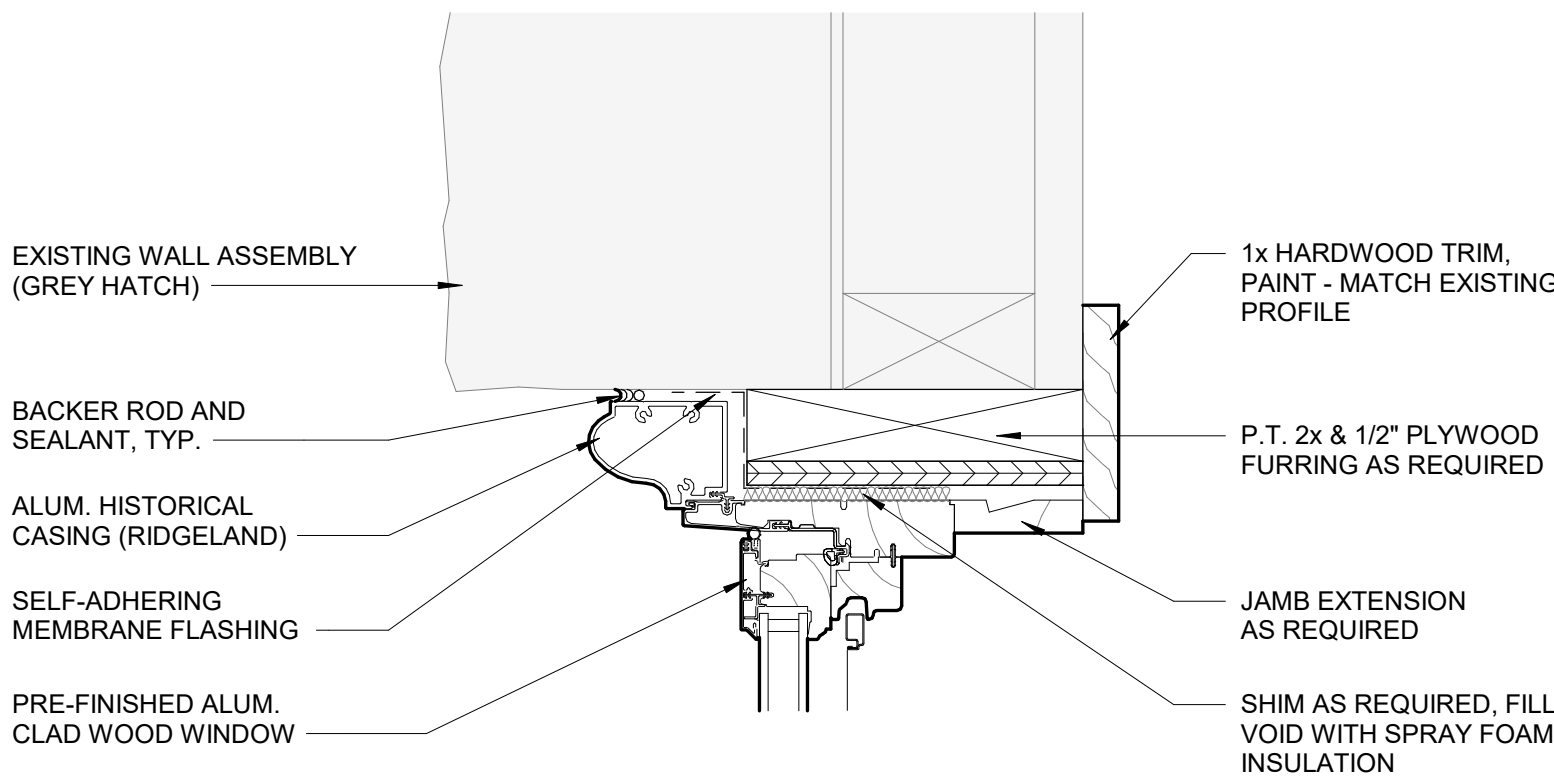
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SHEET TITLE
WINDOW DETAILS

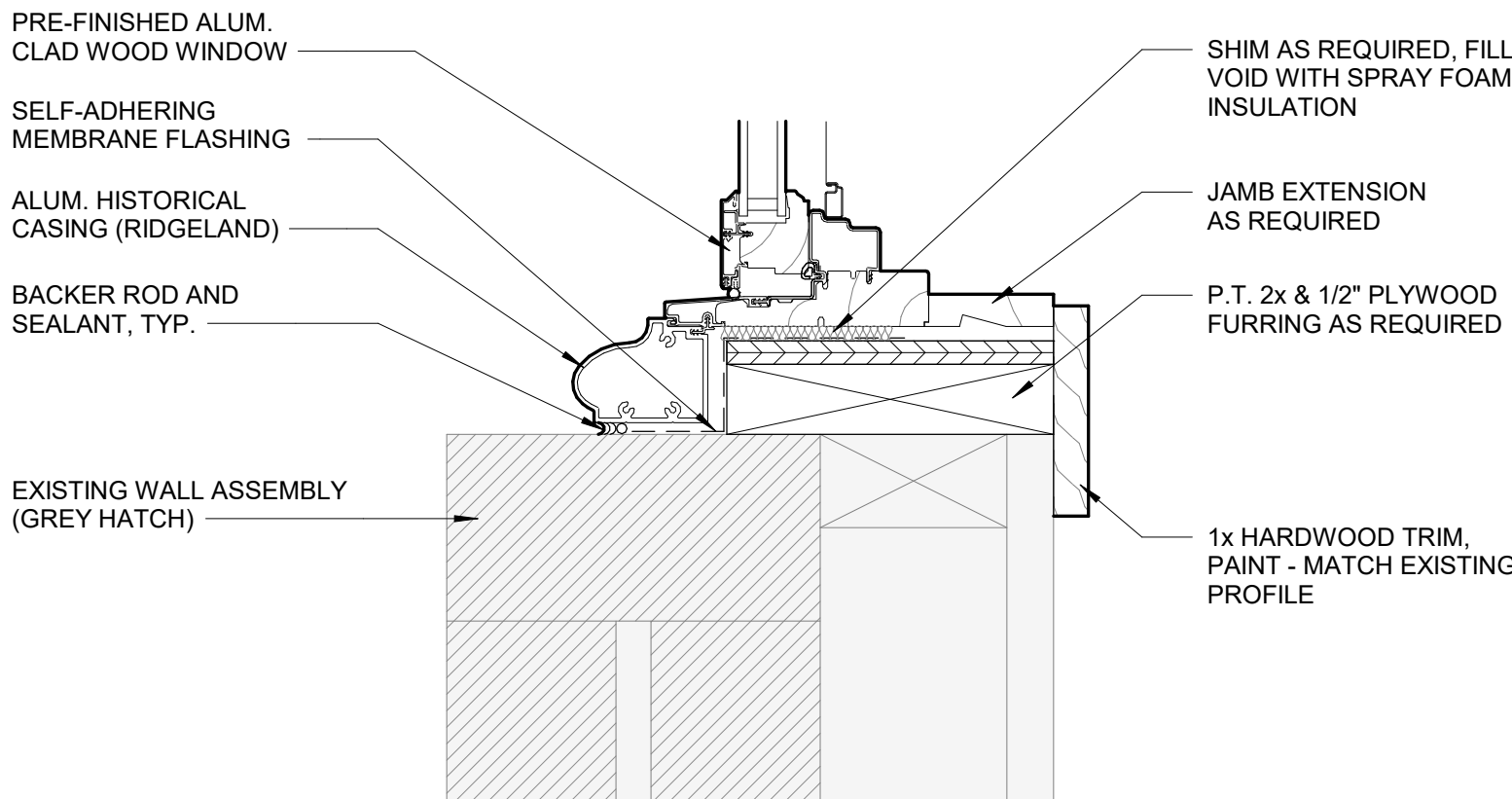
SHEET
A5.2

DATE
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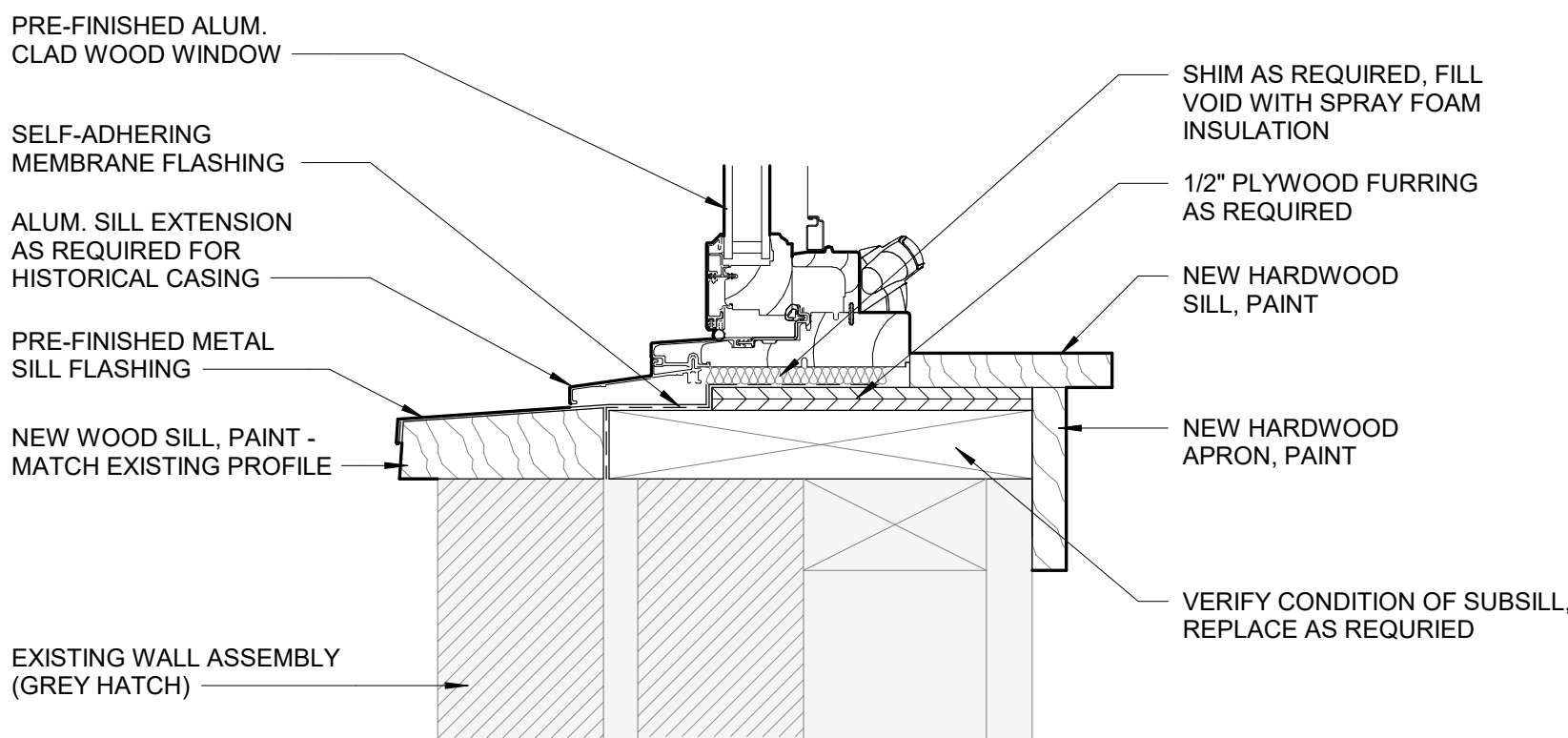
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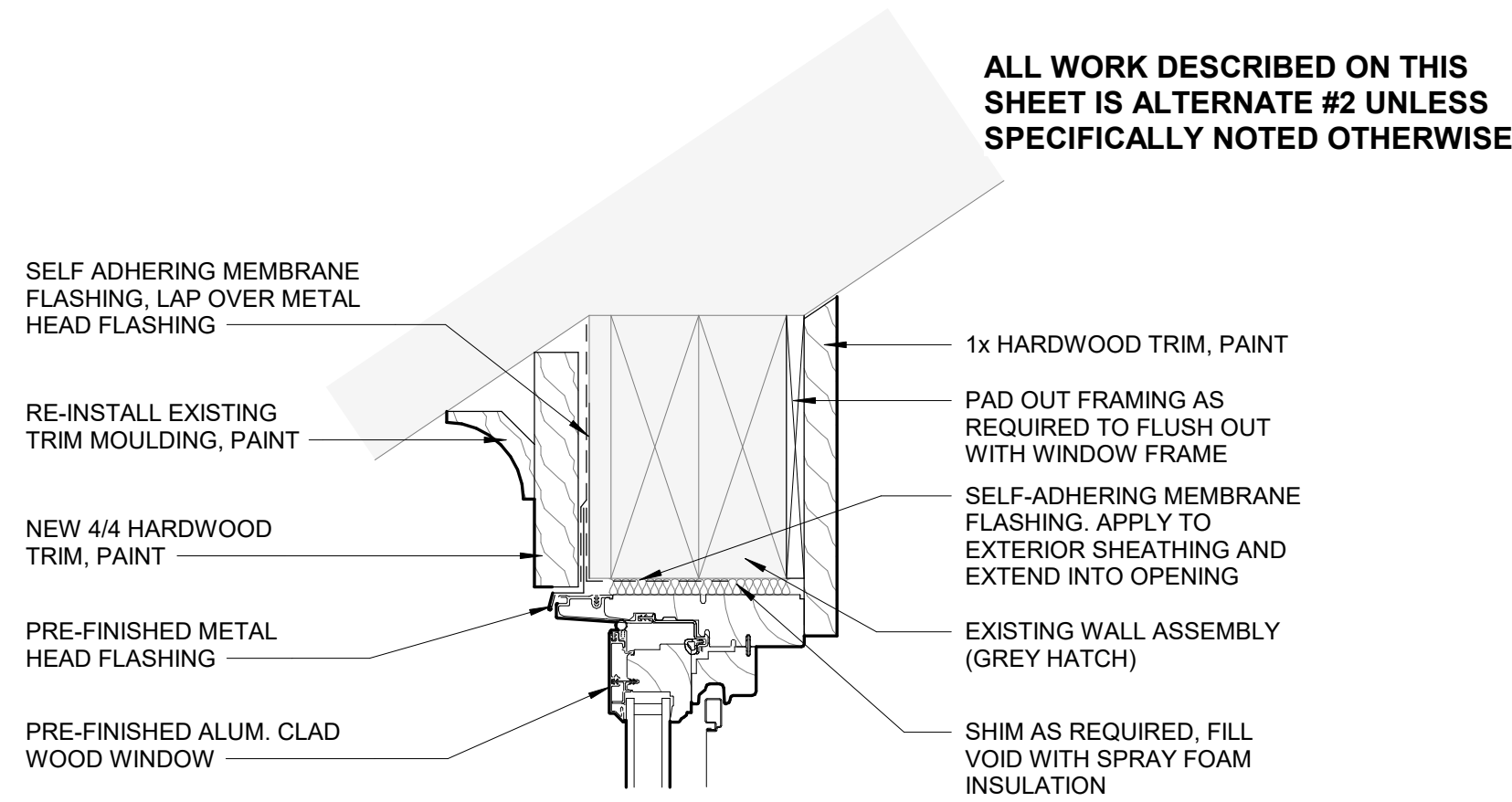
4
A5.3
WND 'K' HEAD DETAIL
3" = 1'-0"



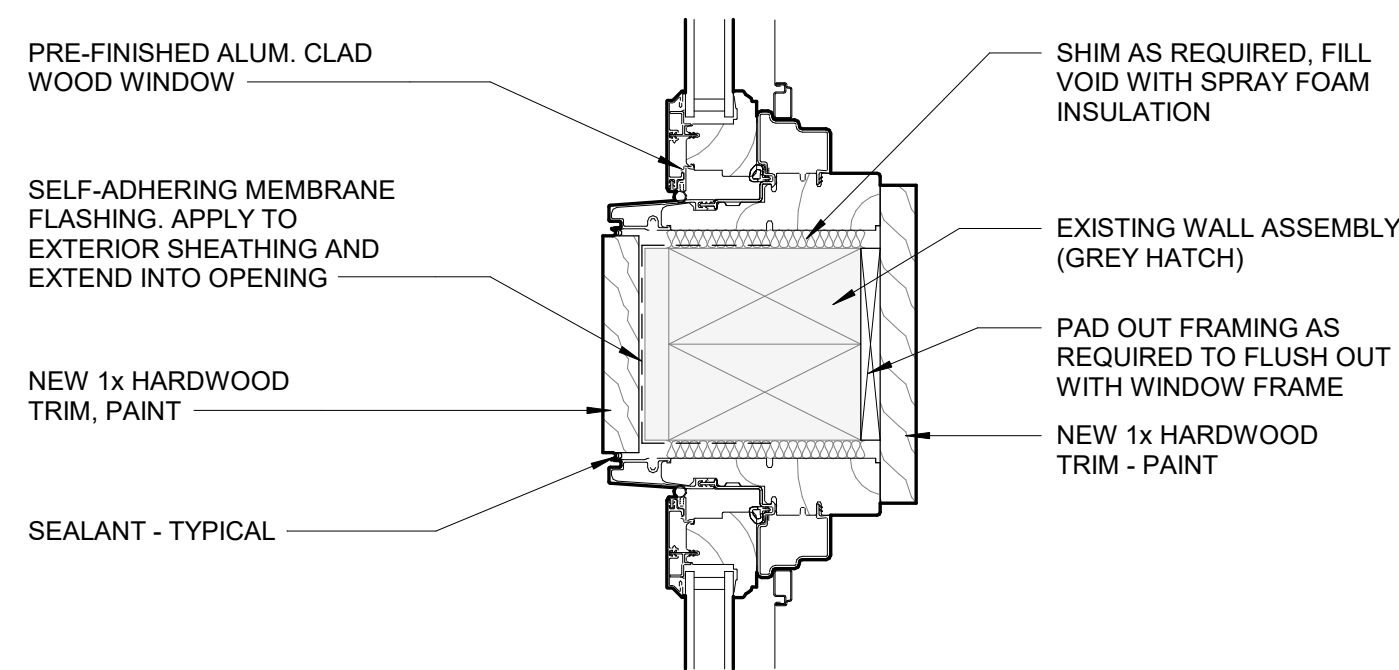
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WND 'K' JAMB DETAIL
3" = 1'-0"



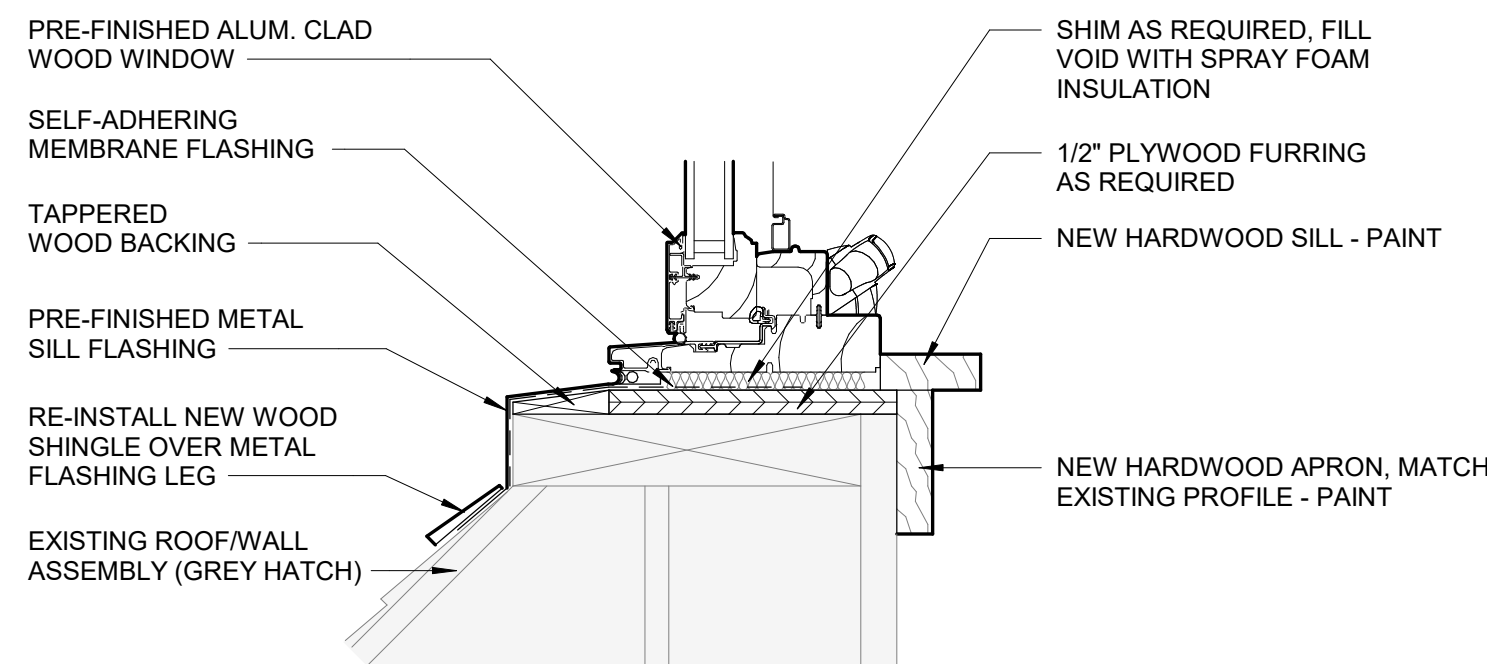
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A5.3
WND 'K' SILL DETAIL
3" = 1'-0"



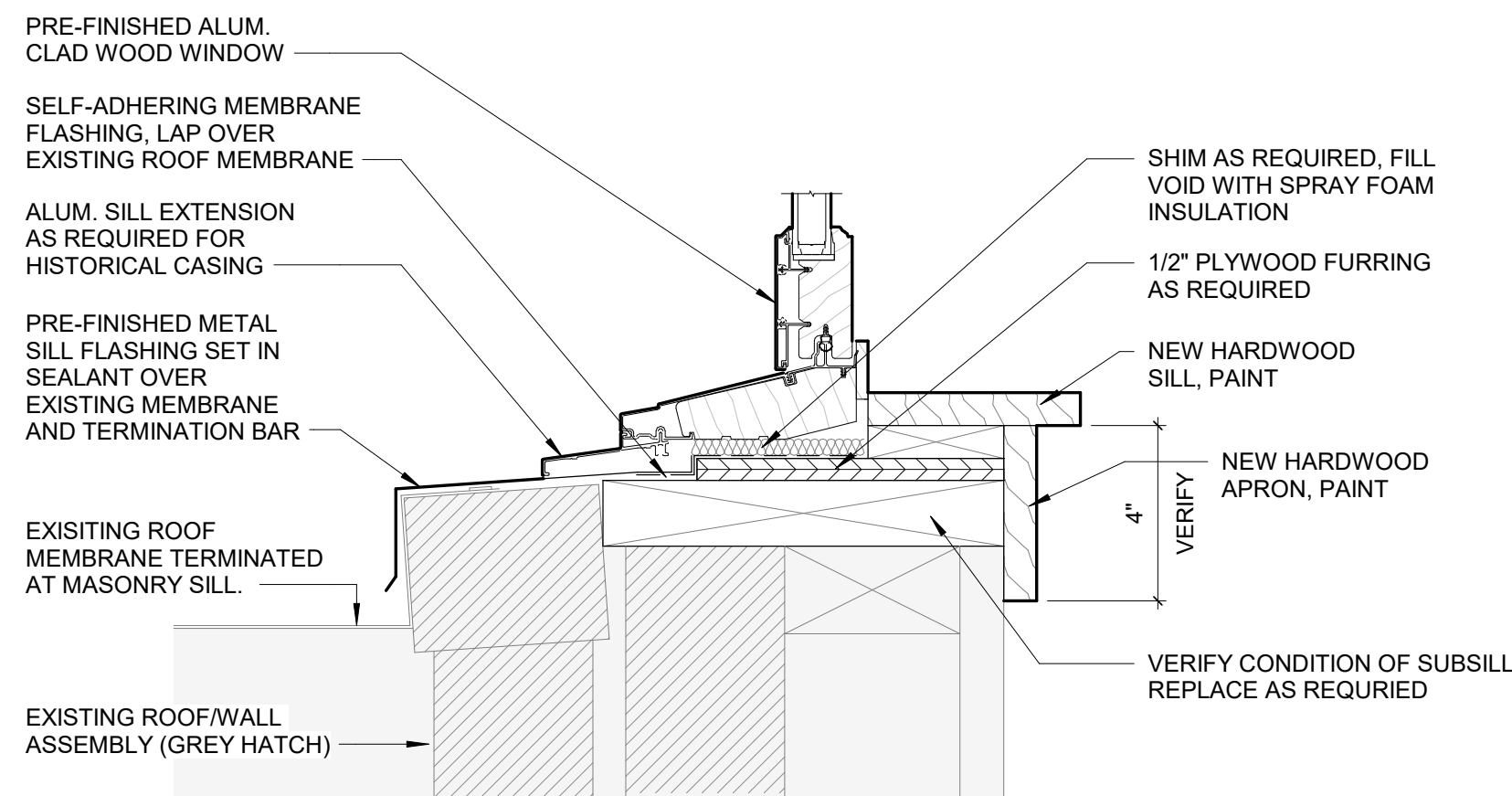
1
A5.3
WND 'L' HEAD DETAIL
3" = 1'-0"



2
A5.3
WND 'L' JAMB DETAIL
3" = 1'-0"

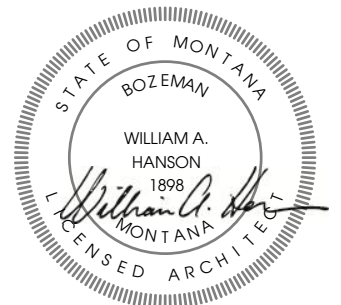


3
A5.3
WND 'L' SILL DETAIL
3" = 1'-0"



7
A5.3
WND 'I' & 'J' SILL DETAIL
3" = 1'-0"

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SHEET TITLE
**WINDOW
DETAILS**

SHEET
A5.3

DATE
03/10/25